STORMWATER MANAGEMENT REPORT

Posey Solar Project

Posey County, Indiana

APRIL 2022



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Stormwater Management Report

Posey Solar Project

Posey County, Indiana

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Project Number: R0027286.00 Date: April 15, 2022

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Introduction

The purpose of this report is to summarize the proposed stormwater management for the Posey Solar Project. This report was prepared to meet local and state requirements and is intended for submittal to these agencies for permitting review and approval.

The project site is proposed on approximately 1,435 acres and is located approximately 4 miles east of the city of Mt. Vernon in Posey County, Indiana. The site's current use is agricultural row crops with limited areas of forested cover.

The proposed use of the site will be a solar facility consisting of approximately 1,404 acres of meadow grass with solar modules mounted above grade and 31 acres of new impervious surface including gravel access roads and associated solar infrastructure. The proposed site will be converted to meadow conditions below the solar modules within the fenced boundary. Panels are not considered impervious due to improvements made to the site to meet Posey County Post Construction Stormwater Management Guidelines.

Minimal grading will be proposed on site and existing drainage patterns will be maintained to the extent practical. A detention basin is proposed at the substation and additional stormwater management measures are proposed in areas with final slopes greater than 5% and localized areas where existing drainage patterns have been altered to meet the requirements of the Posey County Soil and Water Conservation District.

Data Sources

TABLE 1: DATA SOURCES

Task	Format	Source	Use
Elevation	1-foot Contours	Westwood flown on 12/03/2020	Drainage Plans
Crop Data	Shapefile	USDA 2013 Crop Data Layer	Landcover
Soils	Shapefile	USGS SSURGO Dataset	Curve Numbers
Precipitation	PDF File	NOAA Atlas 14	Design Storms
Site Boundary	KMZ	Arevon	Define Model Extents
Site Plan and Solar Layout	Shapefile	Westwood Professional Services	Drainage Plans and Model values
2014 Aerial Photography	ArcGIS Map Service	USDA FSA	Reference
FEMA Flood Zones	PDF; Shapefile	FEMA	Reference
Indiana DNR Flood Zone	Shapefile	Indiana DNR	Reference
Indiana Oil and Gas Well Record Viewer	ArcGIS Layer	Indiana DNR	Orphaned/Abandoned Wells
Karst Sinkhole Inventory	Shapefile	Indiana Geological Society	Sinkhole Locations
Legal Drains	GIS Layer	Posey County	Legal Drain Locations

Site Conditions

Site Location

The project area is located approximately 4 miles east of the city of Mt. Vernon in Posey County, Indiana.

Historical Use

A review of aerial photographs shows that the site is currently used and has historically been used for agricultural row crops with limited areas of forested cover.

Topography Description

The project acquired a ground-calibrated (LiDAR) topographic survey (1-foot contours), which is used to show the existing ground surface on the proposed plans. This dataset was used for the HydroCAD modeling, the slope analysis, to identify existing drainage infrastructure, and to prepare maps and figures. The site is generally flat with slopes around 1%-5%. A detailed review of the land slope of all drainage areas under the array was performed and includes average and maximum land slopes under the array. This review is included as Exhibit 7.

Drainage Patterns

Onsite runoff is split into 28 drainage areas based on discharge locations and existing low areas. Several drainage ditches flow through the site that generally flow south. Drainage areas are shown in Exhibits 5 and 6.

Project Watershed

The project is located north of the Ohio River, with a large majority of the project draining south via smaller ditches and creeks, ultimately discharging into the Ohio River. The site was broken up into 6 sub watersheds. Overall sub watershed discharge locations are shown in Exhibit 2C.

Streams & Floodplains

Both FEMA flood hazards and IDNR best available floodplain hazards are present within and around the project. These areas are shown on Exhibits 5 and 6. Dimensions of existing channels are shown on the existing drainage map (Exhibit 5). All existing channels will be maintained under proposed conditions and no new channels are proposed.

Wetlands

The wetlands and waters mapping is based on a field delineation. The field delineation was performed using the level two on-site routine determination method set forth in the Corps of Engineers Wetlands Delineation Manual (USACE 1987) and the Regional Supplement to the Corps of Engineers Wetland Delineation Manual: Midwest Region, Version 2.0 (USACE 2010). The Newburgh field office of the U.S. Army Corps of Engineers is reviewing the Project's Wetlands and Waters Delineation Report. Wetlands within and around the project are shown on Exhibits 5 and 6.

Soils

Soils data was downloaded from the Websoil Survey Data published on September 9, 2021, by the Natural Resources Conservation Service (NRCS) and can be found in Exhibit 3. The soils data was reviewed and incorporated into the analysis.

The site consists primarily of Hydrologic Soil Group (HSG) B/D and C/D soils, which behave like and have been modeled as D soils. Type B soils are also present on site with a few areas of

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Type C as well. Type B soils have moderate runoff potential and infiltration rates. Type C soils have higher runoff potential and low infiltration rates. Type D soils have the highest runoff potential and very low infiltration rates.

Abandoned Wells & Sinkholes

According to the IDNR Oil and Gas Well Records Viewer, there are no orphaned/abandoned wells within the project. A 2011 sinkhole inventory created by the Indiana Geological Survey for southern Indiana and northern Kentucky shows no sinkholes located within the project area.

Legal Drains

Project infrastructure will maintain a 75-foot buffer from all legal drains located near the site. Legal drains and their 75-foot buffer are labeled and shown on Exhibit 5.

Floodplain Development and IDNR Floodway Permitting

The Project has not submitted an application for a Flood Plain Development Permit for any facilities proposed within Special Flood Hazard Areas as identified by FEMA. No facilities are proposed within a FEMA floodway. In accordance with the Posey County Solar and Wind Ordinance, prior to issuance of an Improvement Location Permit or commencement of construction, the Project will submit all federal, state, and local approvals to the Area Plan Commission, which would include a Floodplain Development Permit, if applicable.

Prior to any construction within an IDNR mapped or unmapped floodway, the Project will secure a permit for construction in compliance with the Flood Control Act (IC 14-28-1). In coordination with the IDNR, it has been determined that the construction limits of the Project will encroach on three floodways regulated by the IDNR - Dixon and Lewis Ditch, Castleberry Creek and Cypress Creek. The Project has submitted separate applications for excavation, fill, or non-residential construction within the IDNR regulated floodways associated with the Dixon and Lewis Ditch and Castleberry Creek. A non-modeling hydraulic assessment was performed for each application to demonstrate that the proposed change in Effective Cross Sectional Flow Area at the most restrictive segment of the encroachment was less than a threshold of five percent, a threshold which has been tested by the IDNR to ensure that the surcharge due to the Project would be significantly less than 0.14 feet. The Base Flood Elevations were determined using the Indiana Floodplain Information Portal. A separate application will be prepared and submitted for excavation, fill, or non-residential construction within the IDNR regulated floodways associated with Cypress Creek.

At the request of the IDNR, breakaway fencing is proposed as an option in certain locations to mitigate any potential impacts to the Base Flood Elevation in the unanticipated event that the fence is to become blocked by debris during a flood. The operation of the breakaway fence is not anticipated because the Project's perimeter security fencing is generally installed parallel to the flow of the IDNR regulated floodways, flood velocities are such that minimal debris flow is expected (< 1 foot per second) and there is limited woody vegetation or debris in these areas that could accumulate in the fencing. A detail of the proposed breakaway fencing is provided as Appendix J.

The Project performed a site-specific hydrology study to assess the maximum flooding depths (ft), peak velocities (fps) and scour impacts (ft) of the overall Project area and watershed drainage. The hydrologic modeling used FLO-2D modeling software. It was performed based on the 100-year, 24-hour storm event (NOAA Atlas 14 - 7.17 inches), considering the latest elevation data, runoff coefficients based on hydrologic soil groups and land cover and inflows

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from the Ohio River and other major streams. The model was validated with a HECRAS 1-D model for the Ohio River. The results have aligned well with FEMA's mapped floodplain extents and the IDNR's mapped and unmapped floodways and associated fringe areas. The Preliminary Hydrology Study is included in this report as Appendix G.

Stormwater Management Requirements

Stormwater management for the project falls under the jurisdiction of the Posey County Soil and Water Conservation District. The following requirements need to be met for the project:

Water Quantity/Runoff Analysis

Stormwater quantity control must be provided so that proposed conditions peak runoff rates must be equal to or less than existing conditions. The 2-year, 10-year, and 100-year 24-hour stormwater events must meet these requirements.

Posey County requires the following post-construction criteria to be met in order to manage runoff from solar panels: an ungraded, uncompacted soil profile, dense and healthy vegetation over the entire surface, and management of runoff from the panels as non-erosive sheet flow.

Methodology

Existing and proposed conditions are modeled in HydroCAD software. HydroCAD is a widely accepted hydrologic and hydraulic modeling package based on TR-20 unit hydrograph equations. It models stormwater runoff discharge rates and velocities from ponds, culverts, outlet control structures, and stream reaches.

Hydrology

Curve Number Methodology, based on the NRCS-TR 55 method, was used in the modeling for predicting direct runoff. Curve numbers were assigned by reviewing the soil and land cover for each drainage area.

Time of concentrations were calculated for each drainage area in HydroCAD using the lag method. The lag method uses the hydraulic length (distance traveled by a drop of water from the most distant part of the subcatchment to the outlet point) and the average land slope (average slope of entire watershed). The overall curve number for the site along with the lag information is used to get the time of concentration for the site.

Atlas 14 precipitation and distribution data for the 2-year, 10-year, and 100-year 24-hour storm events were used as input for the analysis (Appendix A).

Existing Conditions

The existing site consists of row crops with limited areas of forested cover. Cover for the analysis was determined using the USDA 2013 Crop Data Layer and aerial photos. Curve numbers were assigned based on the land cover and soil types, see table below for summary.

TABLE 2: EXISTING CONDITIONS COVER

Cover	CN	Area [ac]
Cultivated, HSG B	78	155.90
Cultivated, HSG C	85	64.30
Cultivated, HSG D	89	1,195.88
Forest, HSG B	55	2.06
Forest, HSG D	77	17.03
Total		1,435.17

Existing Drainage Feature Review and Maintenance

During the development phase, the Project has worked directly with landowners to acquire any available maps of the existing drainage tile infrastructure. Those maps have been digitized and supplemented with a desktop analysis, which identified suspected tile locations based on signatures observed on aerial imagery from multiple years and a review of topography, former wetland areas and hydric soils. The suspected drain tile locations are shown in Exhibit 5 with corresponding confidence levels. An avoidance and minimization approach is taken during the design phase, to the extent practical.

Prior to construction, the contractor will field locate existing surface drainage outlet structures. The contractor will re-route public or mainline drain tile that services neighboring properties in accordance with the Indiana Drainage Code. Damage to any private tile that is identified during trenching activities will be repaired while it is exposed. If shallow ponding or pooling of water resulting from a damaged drain tile system impedes access or operation of the solar facility during or after construction, the system will be repaired and/or a new drainage system consisting of tile or swales will be designed and constructed to reduce the ponding. If the damaged system doesn't impede access for operations or maintenance, the repairs may be deferred until the post-Project reclamation in accordance with the terms of the lease agreement with the landowner.

This Stormwater Management Report does not consider the benefit of any of the existing drainage tiles. Drain tile has very little effect on the runoff analysis because it is typically sized to keep the fields dry after the very frequent storms (0.5" or less), while this Stormwater Management Report addresses the 2-year, 10-year and 100-year storm events, which are 3.3", 4.7" and 7.17", respectively. Vegetation established within solar array areas and any additional BMPs as needed will serve to prevent concentrated flow if the existing drainage structures are not able to be utilized.

Proposed Conditions

The use of the site will be a solar plant. The site will consist of approximately 1,404 acres of meadow grass with solar modules mounted above grade on a racking system and 31 acres of gravel access roads, electrical equipment, and a substation. The solar modules will be located above grade with meadow grass below the proposed array.

The proposed substation will be a raised pad and runoff from this area will sheet flow to a proposed swale along the border of the raised pad. This swale will route water to the proposed detention basin.

Minimal grading is proposed to meet the tolerances of the proposed solar array. See Appendix F for a conceptual array grading diagram. Drainage patterns are expected to remain materially the

same with the addition of the detention basin that outlets similar to existing conditions. The detention basin is proposed to provide rate control for the added impervious of the proposed substation.

TABLE 5: PROPOSED CONDITIONS COVER

Cover	CN	Area [ac]
Solar with Low Maintenance Grass, HSG B	58	155.82
Solar with Low Maintenance Grass, HSG C	71	64.30
Solar with Low Maintenance Grass, HSG D	78	1,183.89
Roads/Inverters/Substation	96	31.16
Total	-	1,435.17

Table 6 shows the percentage of impervious area under the proposed conditions for each of the drainage areas. See Exhibits 5 and 6 for the existing and proposed drainage maps.

Drainage Area	Percent Impervious	Drainage Area	Percent Impervious
1	2.18	16	1.28
2	2.04	17	2.72
3	2.07	18	3.15
4	1.04	19	1.31
5	3.14	20	16.78
6	2.20	21	4.51
7	2.82	22	1.06
8	2.92	23	2.63
9	1.37	24	1.80
10	1.67	25	0.69
11	1.09	26	2.41
12	1.83	27	2.75
13	1.9	14	2.88
14	2.88	28	0.39
15	0.84	Whole Site	2.17

TABLE 6: PROPOSED CONDITIONS IMPERVIOUS PERCENTAGE

Proposed Stormwater Management

A solar project differs greatly from other commercial or residential developments. When constructed, a solar project will include solar panels, at-grade gravel access roads, and other electrical equipment. The panels will be mounted a minimum of 18" above the ground with a low maintenance perennial meadow grass growing under the panels. Due to the area between and beneath the panels being vegetated and compliance with the minimum impervious area disconnection length, panels are not considered an impervious surface. While solar projects may require grading, the existing terrain is smoothed to accommodate array installation, rather than significant changes to grades or slopes, and the grading is designed to maintain existing drainage patterns to the extent practical. Access roads are installed at grade and allow for runoff to sheet flow through the proposed meadow cover which provides reduction in runoff. Runoff rates are reduced from pre- to post-development conditions due to the land cover's conversion from a higher runoff rate row-crop field to a lower runoff rate meadow grass.

A permanent detention basin is proposed near the substation to provide rate control from this added impervious area. The basin will have an outlet culvert at the bottom of the basin to allow water to slowly release to meet requirements. The proposed basin will not have any permanent standing water.

In addition to typical stormwater management BMPs, the recommended approach for solar projects should include the following: limit the amount of impervious surfaces to reduce runoff, minimize the amount of grading to promote sheet flow, and the planting of the meadow grass on the majority of the site to provide both runoff reduction and treatment.

Soil Decompaction

Many of the existing soils onsite belong to hydrologic soil group (HSG) D, which innately allow minimal infiltration. Due to this, compaction would have little to no impact on the runoff analysis that was performed to estimate pre-development and post-development stormwater runoff quantities.

After clearing and grubbing, in areas that require grading, the contractor(s) should strip and stockpile topsoil material for reapplication on all future permanent pervious surface areas. During development, grading and utility construction: subsoils will be compacted as necessary for construction using typical backfill techniques. During final grade, reapplication of the preserved topsoil should be completed by a wide-pad dozer and other equipment to minimize compaction of the topsoil material.

The contractor(s) should restrict vehicle and equipment use to avoid soil compaction where feasible; or techniques such as ripping the soil for decompaction should be completed prior to reseeding or other restoration activity. Heavy construction traffic will be limited to the proposed access roads, so compaction within the solar array areas is not anticipated.

Best Management Practices

During Construction

The exhibit titled Best Management Practices (Exhibit 8) describes several best management practices (BMPs) that are to be implemented in varying slope and array scenarios, as outlined on the exhibit. It also includes typical construction details for the described BMPs.

Information on tracker spacing and height and drip line widths is as follows:

- Tracker Spacing (measured pile to pile or equivalently dripline to dripline): Range of 18' 8" to 24' 10" (at 40% to 30% ground coverage ratio, respectively)
- Tracker Height: max height for the dripline of a standard tracker will be approximately 5'11" in the horizontal position (5' pile reveal); a tracker located within a floodplain would be elevated based on the expected inundation level, increasing the max height up to approximately 9' in the horizontal position.
- Drip Line Width: The drip line will move as the tracker's orientation changes throughout the day. The effective drip line on both the east and west sides of the tracker will be approximately 22" based on the tracker's +/- 60-degree range of motion.

Site Stabilization

Temporary or permanent erosion prevention practices should be initiated within seven (7) days of inactivity and completed by the fourteenth (14) day after construction activity soil disturbance is anticipated to be temporarily or permanently ceased for a period of at least 15 days. Although

a stormwater pollution prevention plan (SWPPP) has not yet been prepared for this project, this information will be cited in Section 8.3.1 of the SWPPP once completed.

A seed mix will be coordinated with the Posey County Extension Office to ensure it complies with the Renewable Energy Generations Systems Ordinance for Unincorporated Posey County prior to construction. The seed mix will be a combination of native forbs and grasses.

According to the Posey County Post Construction Stormwater Management Guidelines, in order to submit a Notice of Termination (NOT), the site must achieve 80% native vegetative ground cover with a clear plan for achieving 90% establishment, or other provisions must be employed until the ground cover meets the 90% threshold. Once completed, this specification will be cited in SWPPP Section 12.0.

Highly erodible soils are typically associated with steeper slopes. The SWPPP will include guidance in section 8.3.2 for steep slope stabilization timing. Timing for stabilization is recommended to be within 7 days to limit duration of exposure and potential for large rain events. The base assumption for the solar project is that areas with slopes in excess of 15% will be avoided for solar arrays. Areas of steep slopes that cannot be avoided during construction must have temporary or permanent erosion prevention practices initiated immediately (end of the same working day) after construction activity disturbing soil in an area is temporarily or permanently suspended for a period exceeding seven calendar days. The application of temporary or permanent erosion control management practices should be completed prior to the seventh day of temporarily or permanently suspending construction activity in an area of steep slopes. BMPs recommended for steeper slopes would include seed and erosion control blanket and runoff controls which would contribute to temporary sediment basins during construction. Temporary basins will include riser outlet structures and skimmer structures to allow drawdown of water below the riser structure elevation.

It is common practice for the construction contractor to pre-seed the site for stabilization prior to initiating site grading and other preparation activities, such that the scope of the restoration is limited to those areas where more significant disturbance occurs. Details of the plan will be included within the SWPPP and may vary based on the specific timing of construction.

Post Construction

The included exhibit titled Best Management Practices (Exhibit 8) identifies locations where existing slopes are 5% or greater for areas under the proposed array. In locations where the final grade is 5% or greater, additional BMPs are proposed which may include one or a combination of the following:

- Stormwater Ponds These would be proposed at locations downstream of the slopes that are 5% or greater and designed to meet runoff rate requirements assuming that the areas directly under the panels are impervious with the areas between the panels as meadow grass.
- Level Spreaders or Slope Interrupters These would be proposed at 100 ft intervals where slopes are 5% or greater to disperse runoff and promote sheet flow. See Exhibit 8B for the proposed level spreader design.

Micrograding

Recent innovations in solar panels and their structural racking systems have increased the grades and tolerances that are acceptable to build on, including steeper slopes and greater undulations. However, there are still instances where some grading is required to meet the

specifications of the racking system. These "micrograding" areas are determined on a pile-bypile basis based on the structural tolerances of the equipment. In most cases, the controlling tolerances are due to undulations in the topography, which are locally smoothed, but not leveled. Also due to the small, localized nature of the micrograding it does not typically alter drainage patterns and typically reduces erosion potential and stormwater runoff by flattening or smoothing some of the steeper areas and increasing the time of concentration. Maintaining existing drainage patterns and limiting mass grading is an objective of the detailed design. The practice is imperative to the success of the construction efforts and long-term profitability of the Project.

Most grading will occur in areas with slopes greater than 10-15% and will not materially alter existing drainage patterns. In the localized areas where the cut/fill is in excess of 1 foot, the Project will separate, stockpile and respread topsoil prior to seeding and stabilization. The minimum depth of topsoil spreading will be based on the depth of the existing topsoil and the requirements for the proposed vegetation. The minimum depth would be 4 inches, but the re-application is typically 6 to 8 inches. The intent of the project is to maintain existing drainage patterns but if patterns are altered during the final grading design appropriate permanent stormwater basins or other BMPs will be utilized to meet the runoff control requirements.

Water Quantity/Runoff Analysis

Stormwater quantity calculations for the site were prepared using HydroCAD. The proposed site meets the rate control requirements of the Posey County Soil and Water Conservation District. Table 8 shows a summary of the runoff rates for each event at the site discharge locations. Calculations are included in Appendices B and C.

Location 2-year Runoff (cfs)		10-year Ru	10-year Runoff (cfs)		100-year Runoff (cfs)	
	Existing	Proposed	Existing	Proposed	Existing	Proposed
1	98.41	44.58	158.23	84.61	253.24	154.90
2	191.76	81.89	307.90	162.74	486.76	303.52
3	289.79	128.57	467.46	248.07	747.51	457.80
4	541.89	211.59	894.76	443.15	1,446.73	860.01
5	115.43	52.63	185.30	101.88	293.79	187.26
6	48.20	21.12	77.25	41.38	122.16	76.44

TABLE 8: RUNOFF RATE SUMMARY

Stormwater Management Practices

Permanent Detention Basin

A permanent detention basin is provided near the substation to capture runoff to slow release rates. Table 9 summarizes the proposed substation basin on site. Calculations can be found in Appendix D.

TABLE 9: PROPOSED BASIN SUMMARY

Basin	Bottom Elevation	Outlet Elevation	Emergency Overflow Elev.	100-year High Water Elevation	Top Elevation
Substation	388.00	388.00	389.00	389.43	390.00

Swale Sizing

A swale is proposed to route runoff from the substation to the basin and safely outlet basin runoff from the site. The swale will be sized during the detailed design prior to the application for the Improvement Location Permit.

Proposed Culverts

Culverts are proposed below new access roads at entrances and at areas of concentrated flow to maintain existing drainage patterns through the proposed site. Culverts are sized for the 10-year 24-hour storm event with a 1-foot allowable head. Table 10 summaries the proposed culverts on site, see civil plans for culvert locations. Peak runoff rates were calculated using HydroCAD and these flow rates were then input into CulvertMaster along with the inlet and outlet elevations and approximate culvert lengths to determine the required culvert diameter.

Crossing ID	Culvert Diameter	Length (ft)	Inlet Elevation (ft)	Outlet Elevation (ft)	Material
DC-29	2-24"	44	371.7	371.5	CMP
DC-30	1-30"	44	370.4	370.3	CMP
DC-31	3-24"	48	372.1	372	CMP
DC-34	1-18"	36	381.4	381.3	CMP
DC-51	1-24"	48	395.9	394.8	CMP
DC-52	1-30"	50	370.4	369.5	CMP
EC-01	1-18"*	92	387.7	387.1	СМР
EC-02	1-24"	94	389.5	389	CMP
EC-03	1-18"*	86	390.4	389.8	CMP
EC-04	1-18"*	102	384.6	382.7	CMP
EC-05	1-18"*	67	372.3	371.9	СМР
EC-06	1-48"	92	386	385.6	CMP
EC-07	1-18"*	83	380.7	380.6	CMP
EC-08	1-18"*	70	379.8	379.6	CMP
EC-09	1-18"*	92	379.4	377.9	CMP
EC-10	1-18"*	76	381.5	380.9	CMP
EC-11	1-24"	92	379.1	378.7	СМР
EC-12	1-18"*	95	379.5	379.3	CMP
EC-14	1-18"*	84	383.2	382.4	CMP
EC-15	1-18"*	87	379.1	379	CMP
EC-16	1-18"*	92	378.9	378.6	CMP
EC-17	2-30"	82	379.2	378.7	CMP
EC-19	1-18"*	84	380.9	380.6	CMP
EC-20	1-18"*	55	383.4	383.3	CMP
EC-21	2-42"	84	388.5	388.3	СМР
EC-29	1-18"*	84	378.4	377.7	CMP
EC-30	1-24"	90	377.2	376.7	CMP

TABLE 10: PROPOSED CULVERT SUMMARY

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Crossing ID	Culvert Diameter	Length (ft)	Inlet Elevation (ft)	Outlet Elevation (ft)	Material
EC-31	1-18"	86	377	376.7	CMP
EC-32	1-18"*	90	384.7	384.4	CMP
EC-33	1-18"*	90	380.3	379.4	CMP
EC-35	1-18"*	48	418.8	417.7	CMP
EC-36	1-18"*	65	431	428.6	CMP
EC-37	1-18"*	65	406	405.4	CMP
EC-39	1-18"*	44	476.5	474.5	CMP
EC-40	1-18"*	44	407.6	406.4	CMP
EC-41	1-18"*	44	372	371.9	CMP
EC-42	1-18"*	100	374.7	374.5	CMP
EC-43	1-18"*	83	369.9	359.8	CMP
EC-44	1-18"*	55	380.3	379.6	CMP
EC-45	1-18"*	58	392	391.4	CMP
EC-46	1-18"*	50	409.4	409.1	CMP
EC-47	1-18"*	58	391.9	391.4	CMP
EC-48	1-18"*	83	406.1	403.3	CMP
EC-49	1-18"*	52	435.3	434.2	CMP

*Minimal drainage to crossing: 1-18" culvert assumed

Proposed Low Water Crossings

Low water crossings are proposed across access roads at areas of concentrated flow where lack of cover renders culverts unsuitable. Crossings were sized for the 10-year 24-hour storm event and crossing types were determined by calculating the average shear stress at these areas of concentrated flow. Table 11 provides a summary of the proposed low water crossings, see civil plan for crossing locations. See Appendix G for a standard duty low water crossing detail.

Crossing ID	Ditch Slope (%)	Average 10-year Shear Stress (lb/ft^2)	LWC Type	Standard Duty LWC Allowable Shear Stress (lb/ft^2)
DC-32	1.15	0.52	STANDARD DUTY	
DC-33	0.31	0.31	STANDARD DUTY	
DC-35	0.33	0.46	STANDARD DUTY	2.4
DC-36	1.30	0.65	STANDARD DUTY	
DC-46	0.38	0.15	STANDARD DUTY	

TABLE 11: PROPOSED LOW WATER CROSSING SUMMARY

Construction Conditions

During construction conditions, higher runoff rates and volumes can be expected when compared to the fully vegetated final condition. To account for this, dewatering should be anticipated as needed until vegetation has fully established on the site. This may include pumping of temporary swales and diversions. Once the site has been stabilized, sediment will

Stormwater Management Report | Posey Solar Project

need to be removed from any permanent basins on site. Using temporary seed/mulch at the onset of construction can greatly reduce the amount of erosion and rework on solar sites. See the Site Stabilization section of the report for additional information on temporary and permanent stabilization of the site.

Dewatering

Dewatering of turbid water (water that is visibly cloudy or brown in color) should be discharged via pump and hose or overland flow (via temporary ditch or grade cuts) to a temporary sediment basin for pretreatment. The use of riprap apron (energy dissipation) should be used for the discharge location. If riprap is not used, an alternative form of energy dissipation should be used to prevent scour and re-suspension of soil at the discharge point of the hose. If discharge to a temporary sediment basin is not feasible, the use of dewatering dumpsters, dewatering bags or other prefabricated product should be used. Appendix J includes the detail and specifications for use of dewatering bags. The use of rock checks, erosion control blanket and sumps or traps shall be considered for overland flow dewatering. After the use of BMPs, the water could be discharge through a vegetated buffer and energy dissipation. The discharge of water from the site should be visibly clear in appearance.

The discharge of accumulated water should not:

- Contain oil, grease, a sheen, odor, or concrete washout (use an oil-water separator or suitable filtration device if material is found);
- Adversely impact adjacent properties with water or sediment;
- Adversely impact waters of the state;
- Cause erosion of slopes and channels;
- Cause nuisance conditions; and
- Contribute to inundation of wetlands which negatively impact the wetlands.

Additional BMPs

The final Stormwater Pollution Prevention Plan (SWPPP) will be prepared for the submittal of the project's general construction stormwater permit. Potential BMPs include silt fence, grade breaks, erosion control blankets, fiber rolls, bonded fiber matrix (BFM), and others as deemed necessary.

Post-Construction Stormwater O&M Manual

A post-construction stormwater operations and maintenance (O&M) manual outlining the inspection methods necessary to meet the stormwater management guidelines set forth by the Posey County Soil and Water Conservation District will be prepared as part of the final construction plans.

Conclusion

The proposed site was designed to meet the requirements of the Posey County Soil and Water Conservation District for stormwater management. The proposed site consists of proposed basins, swales, and crossings, as well as additional BMPs in areas with final slopes greater than 5% and as necessary in locations where existing drainage patterns may have been altered in order to maintain existing drainage patterns and reduce runoff rates.

References Cited

National Engineering Handbook, Part 630 Hydrology. Chapter 9 Hydrologic Soil-Cover Complexes. USDA. NRCS. 210-VI-NEH, July 2004

USDA Geospatial Data Gateway, 2-meter DEM, Elevation data, Accessed March 2021, https://datagateway.nrcs.usda.gov/

Web soil survey. Retrieved November 2021, from https://websoilsurvey.sc.egov.usda.gov/App/WebSoilSurvey.aspx

NOAA, & Service, N. W. AHPS Precipitation analysis. Retrieved March 2021, from http://water.weather.gov/precip/download.php

USGS. USGS water resources: About USGS water resources. Retrieved March 2021, from https://water.usgs.gov/GIS/huc.html

USDA 2013 Crop Data Layer, Landcover data. Retrieved March 2021, from https://www.nass.usda.gov/Research_and_Science/Cropland/SARS1a.php

Posey County, IN. Final Drainage Plan Requirements for Solar Projects Version 1.2. Adopted 11/15/2021

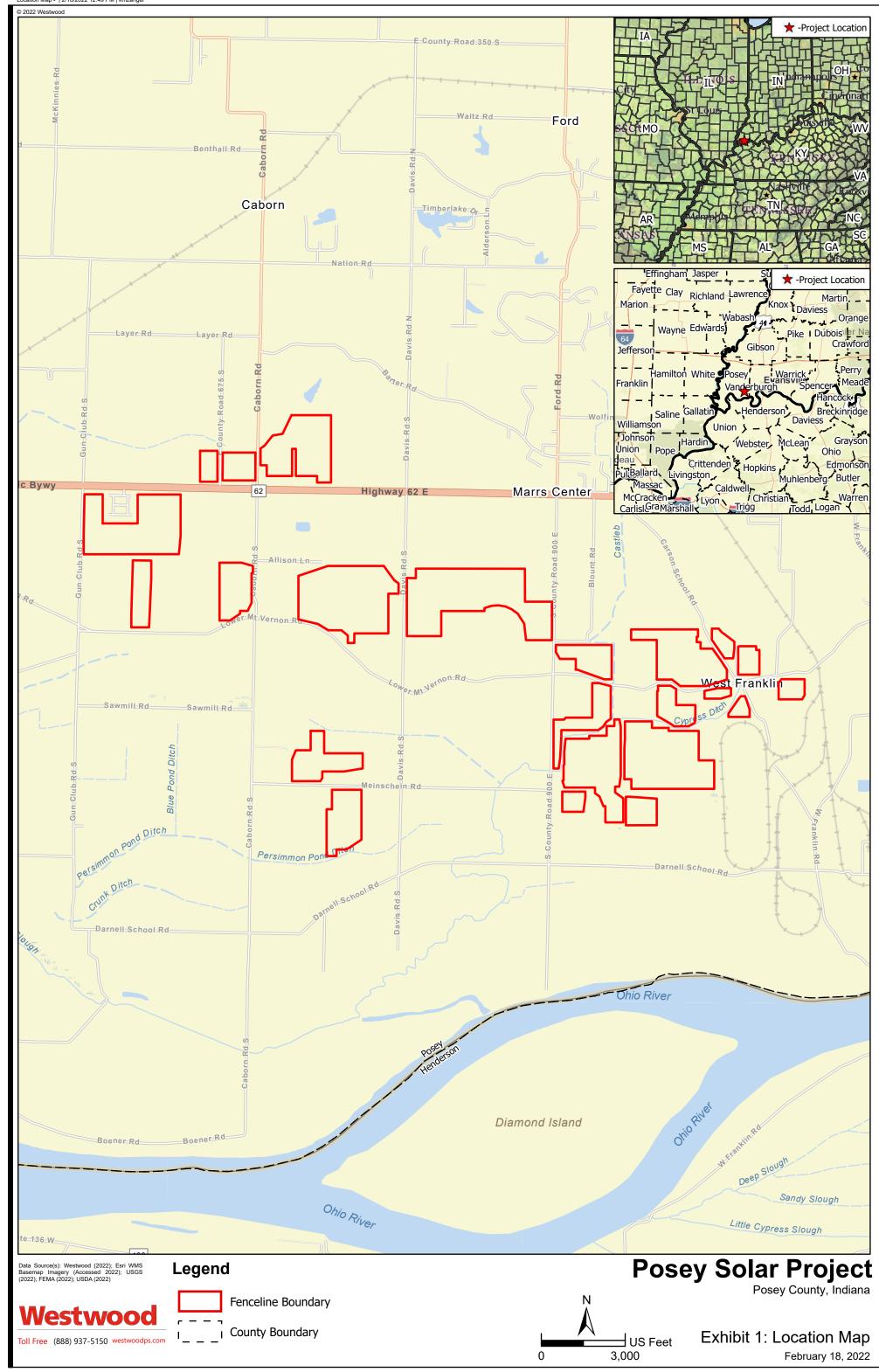
Indiana Geological Society. Karst Sinkhole Inventory (2011). Accessed November 2021, from https://maps.indiana.edu/previewMaps/Hydrology/Karst_Sinkhole_Inventory_2011.html

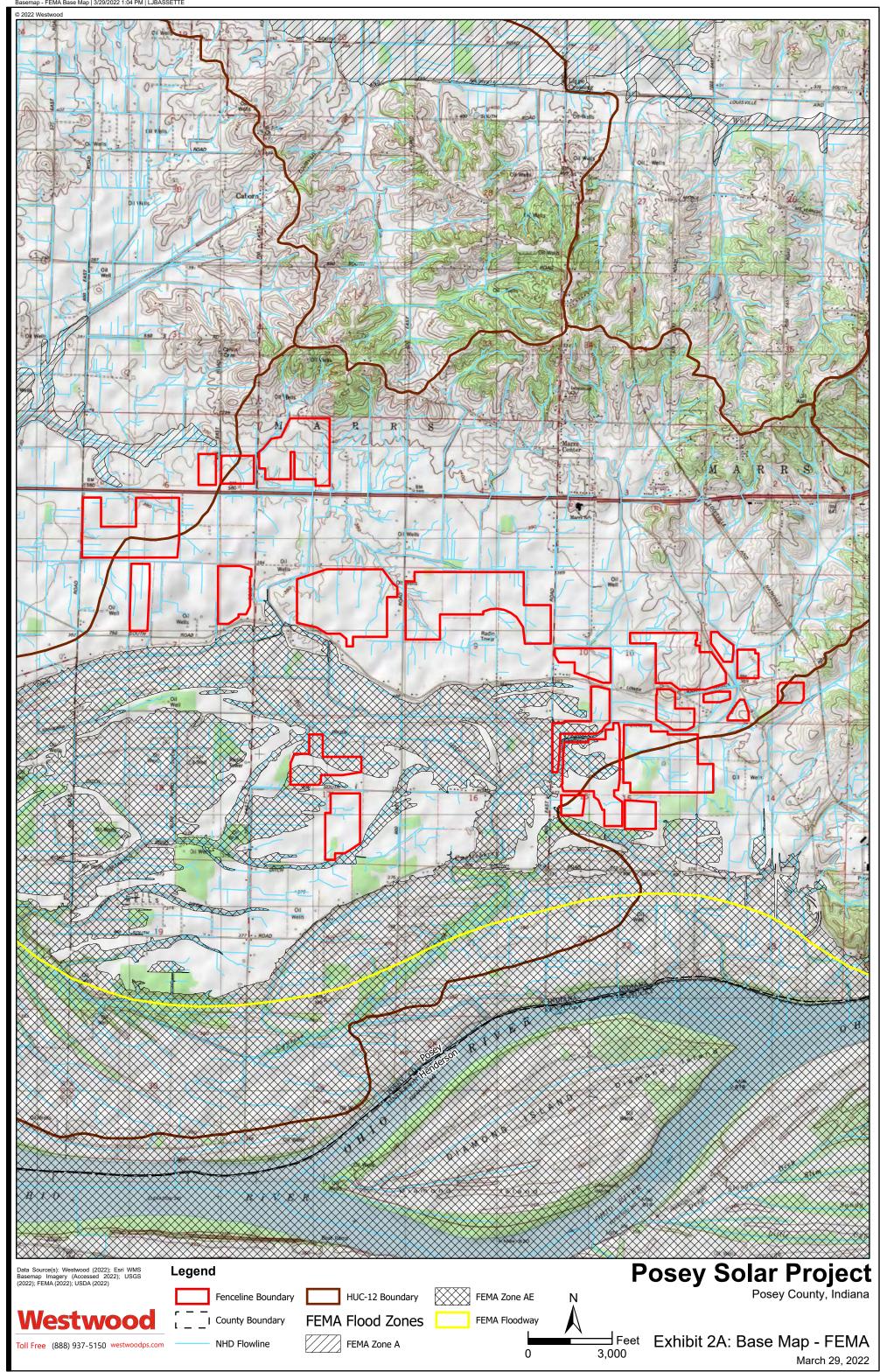
Indiana DNR. Indiana Oil and Gas Well Records Viewer. Accessed November 2021, from https://www.arcgis.com/apps/webappviewer/index.html?id=2a50bd93503e48299531dd776f21 1d84

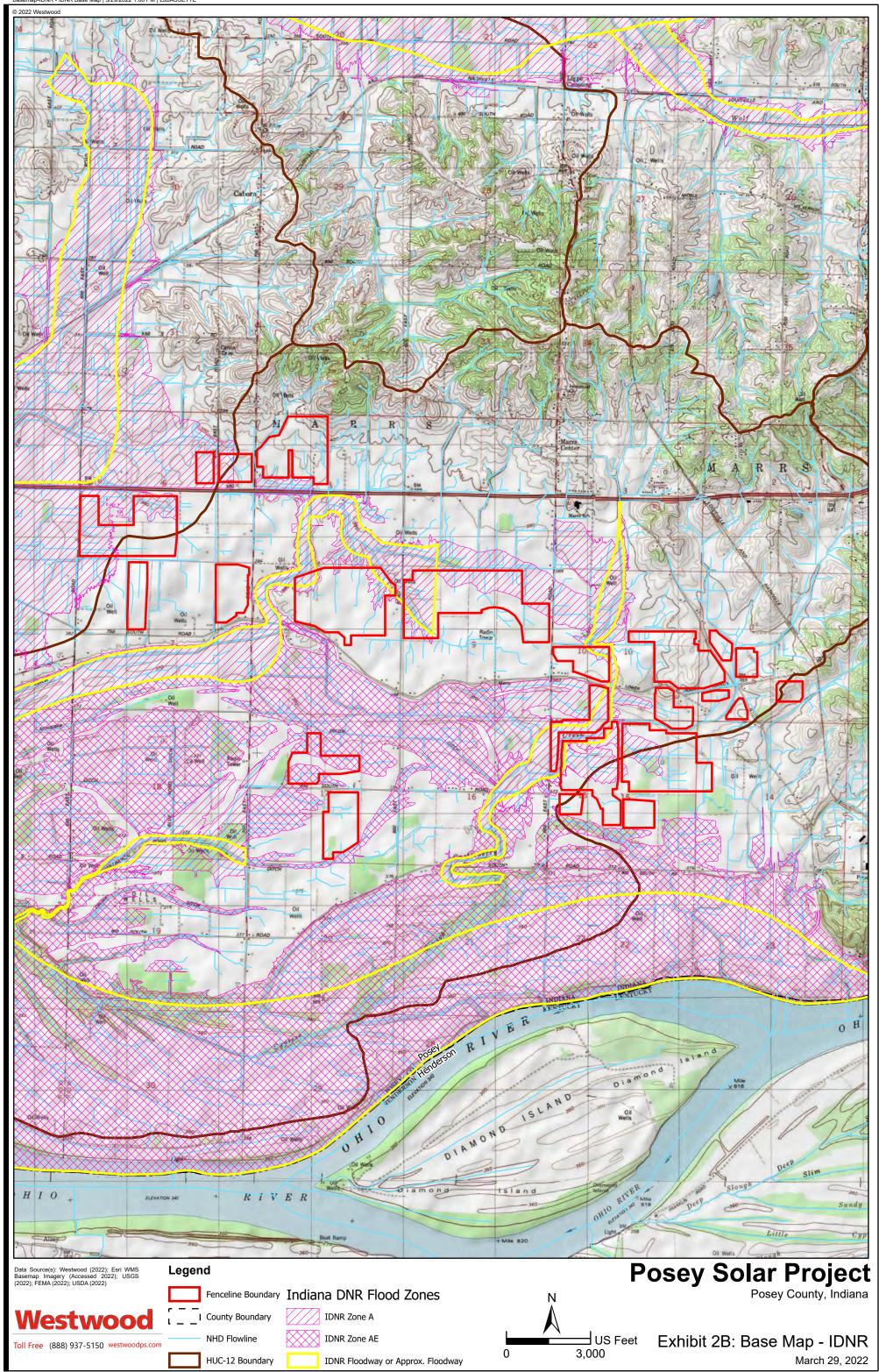
FEMA Flood Insurance Rate Maps. Retrieved November 2021 from https://msc.fema.gov/portal/advanceSearch#searchresultsanchor

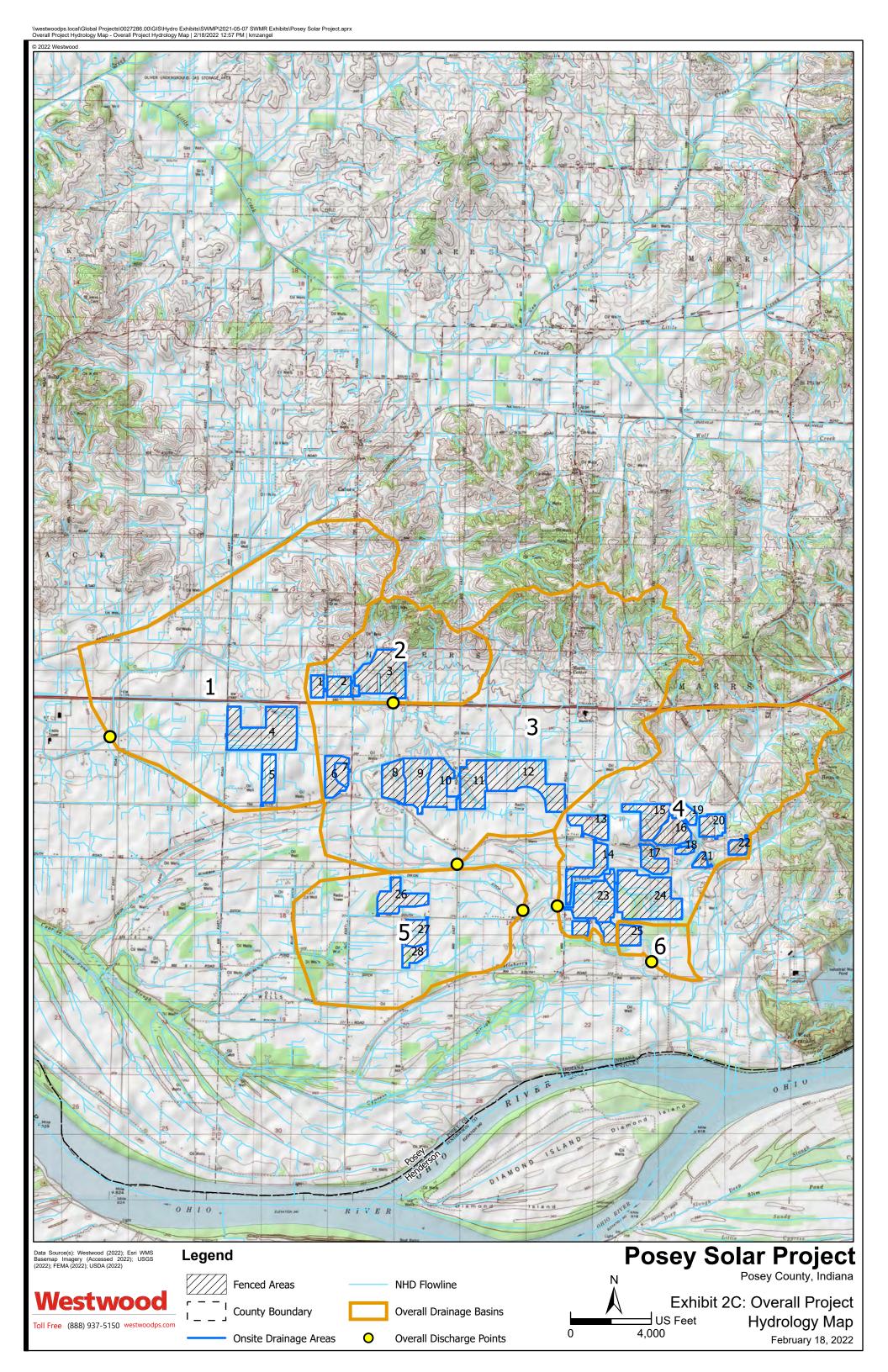
Indiana Department of Natural Resources. Indiana Best Available Floodplain Mapping. Retrieved November 2021 from https://www.in.gov/dnr/water/9846.htm

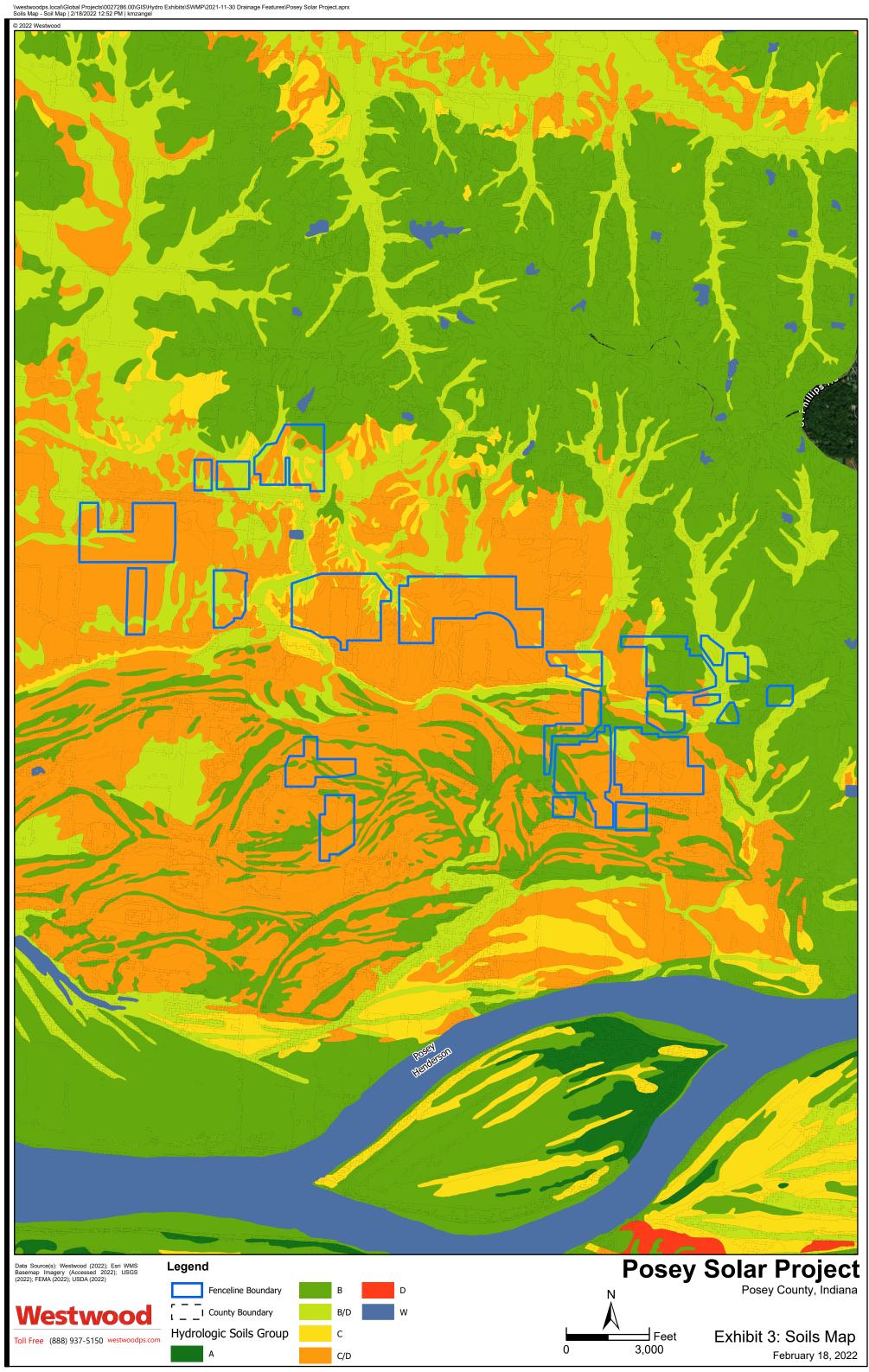
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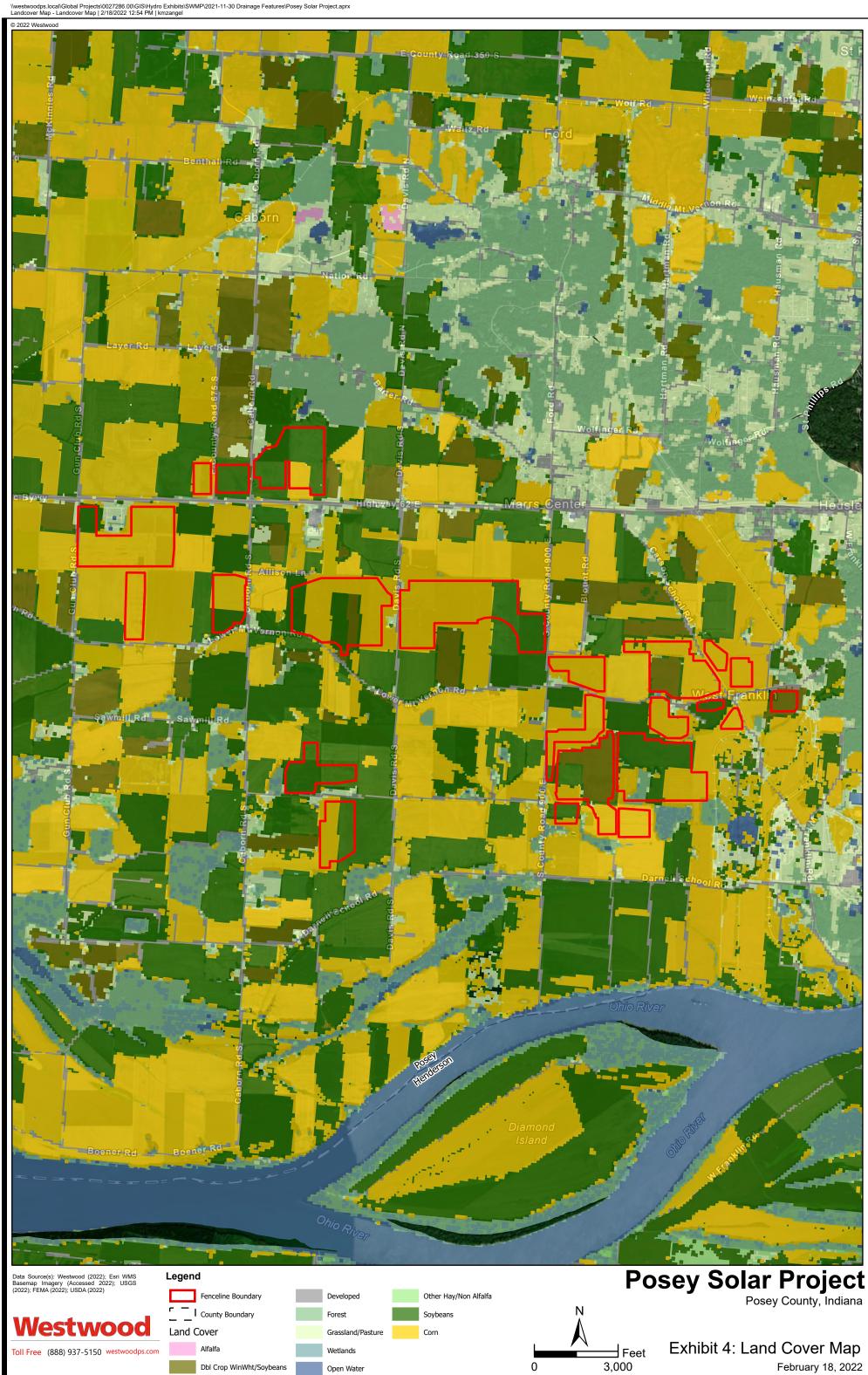


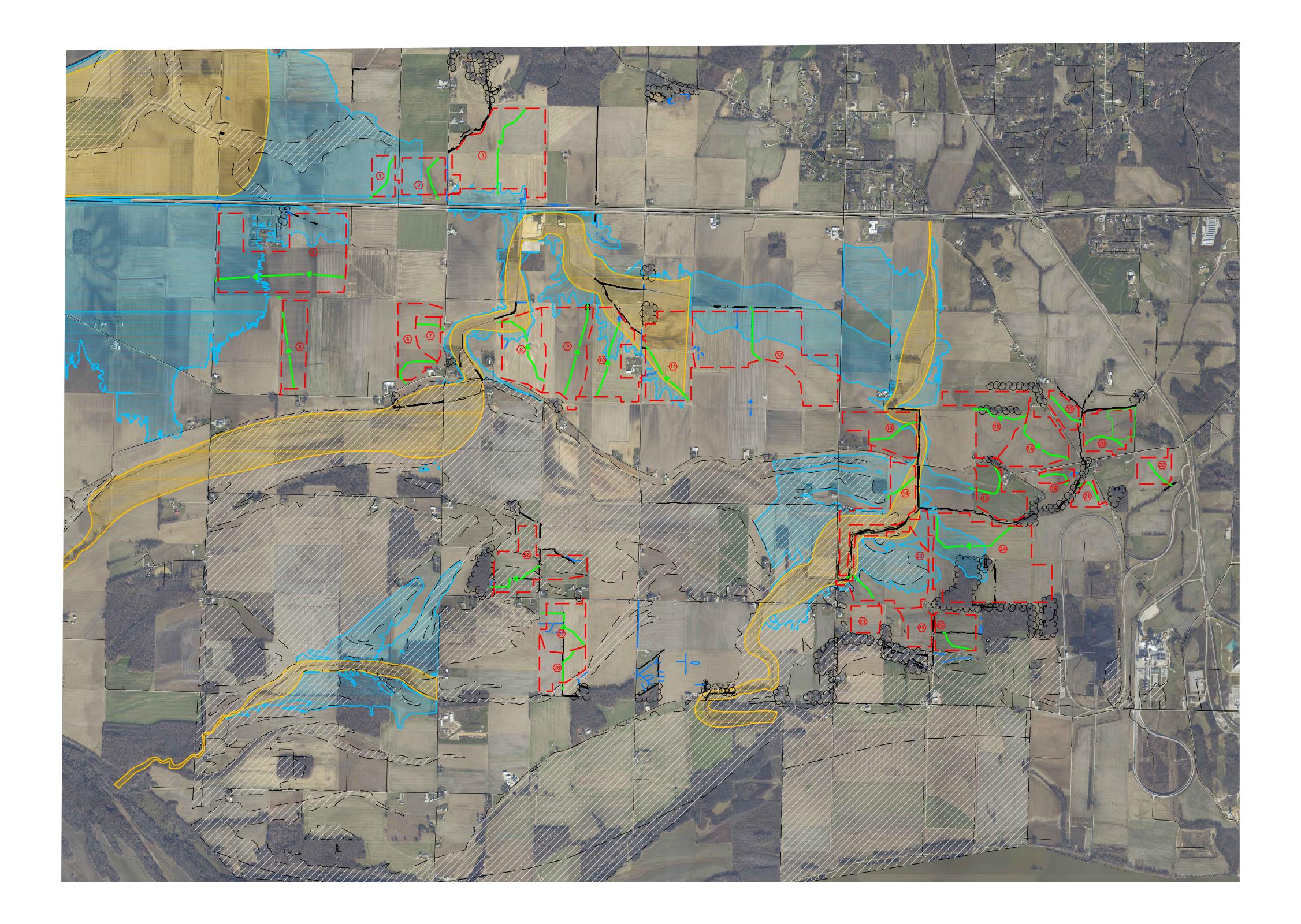


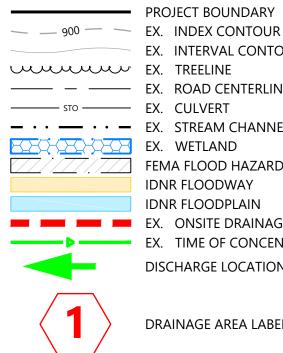












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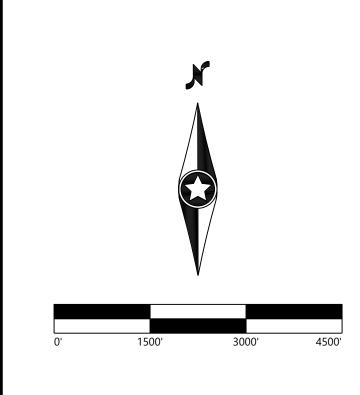
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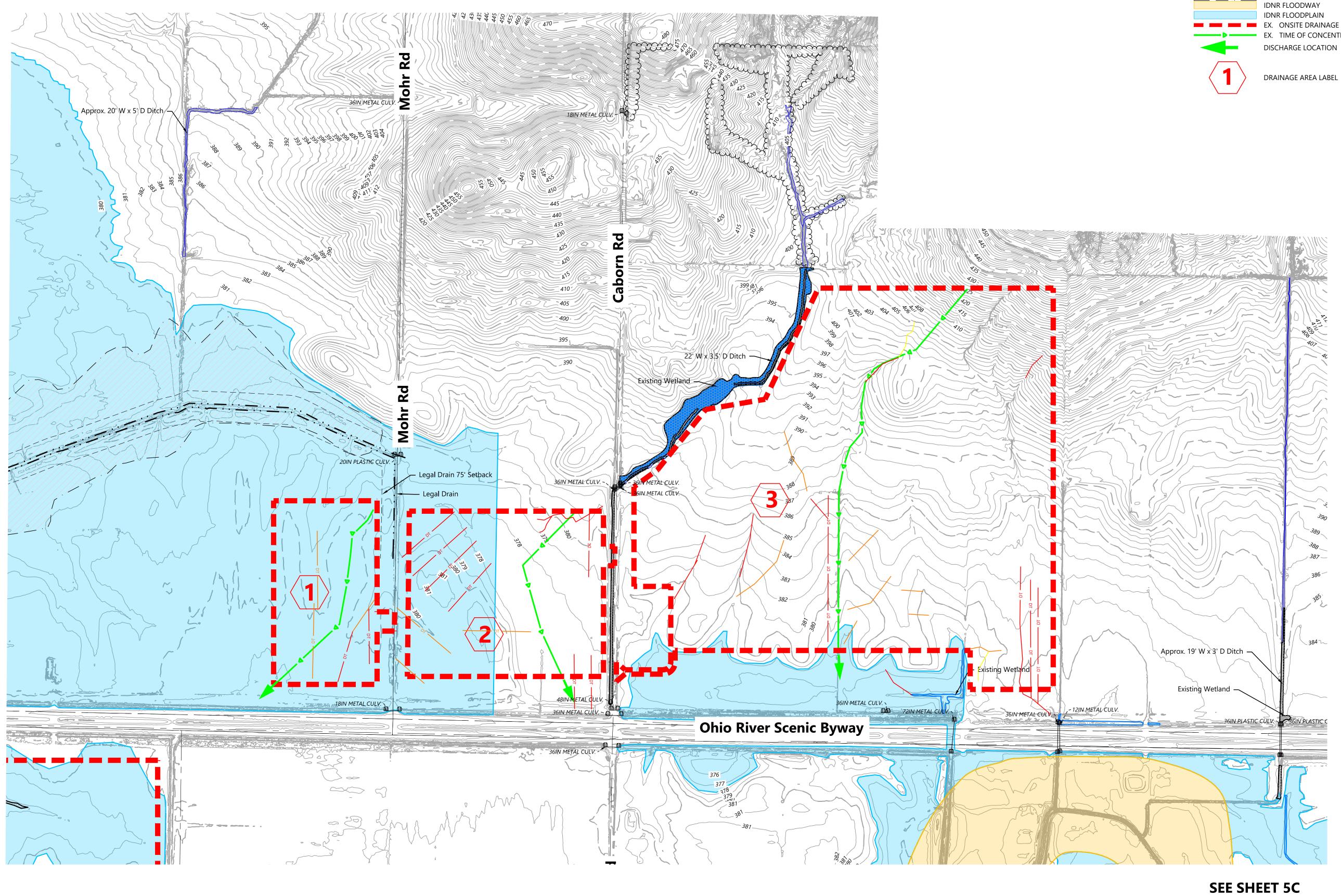
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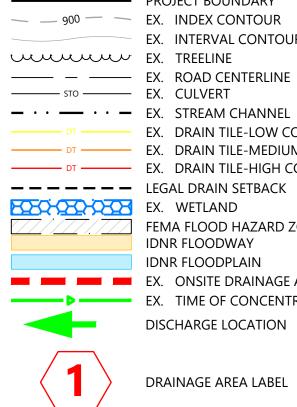
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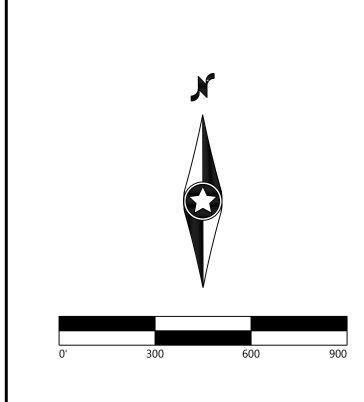
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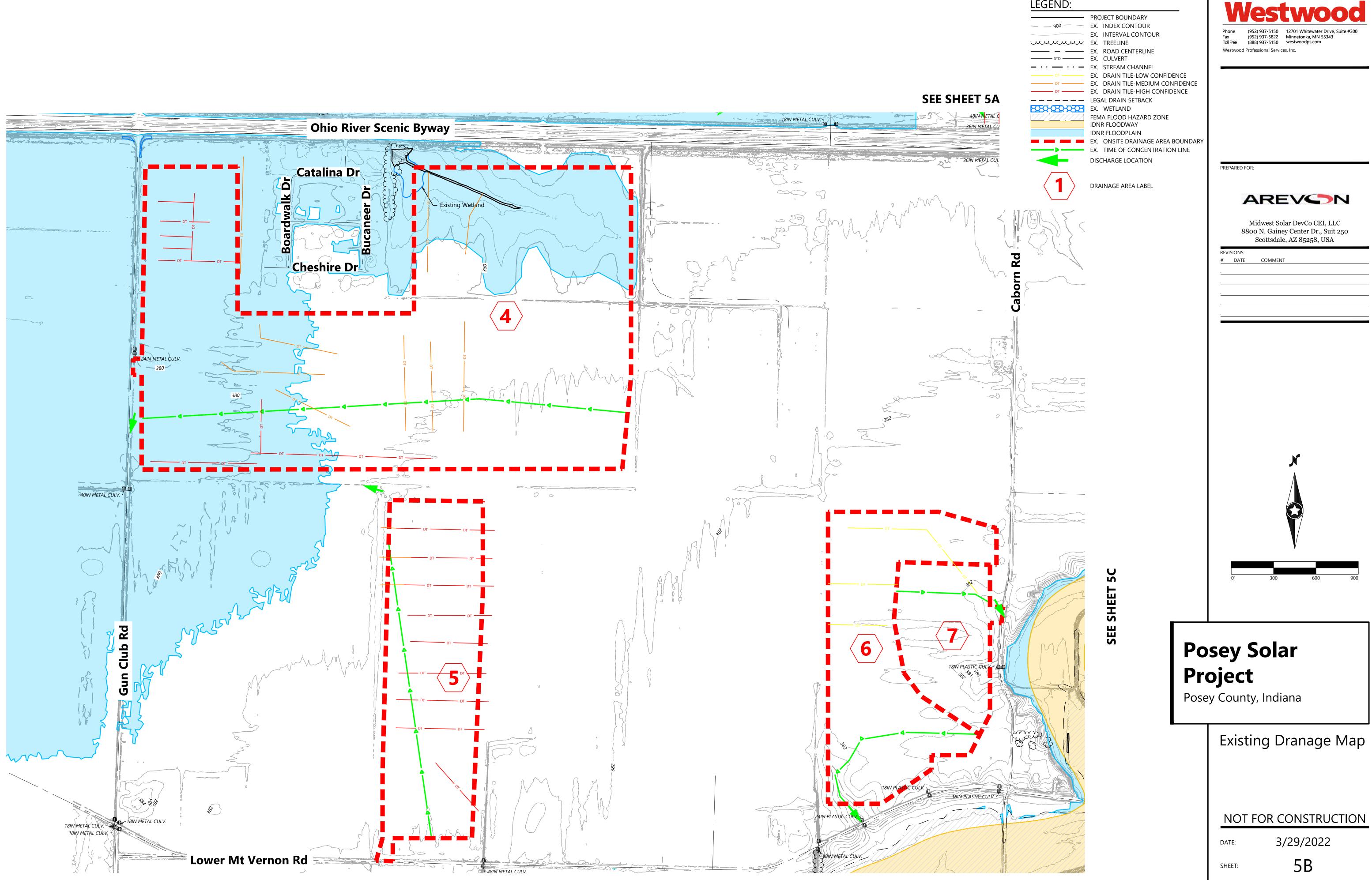
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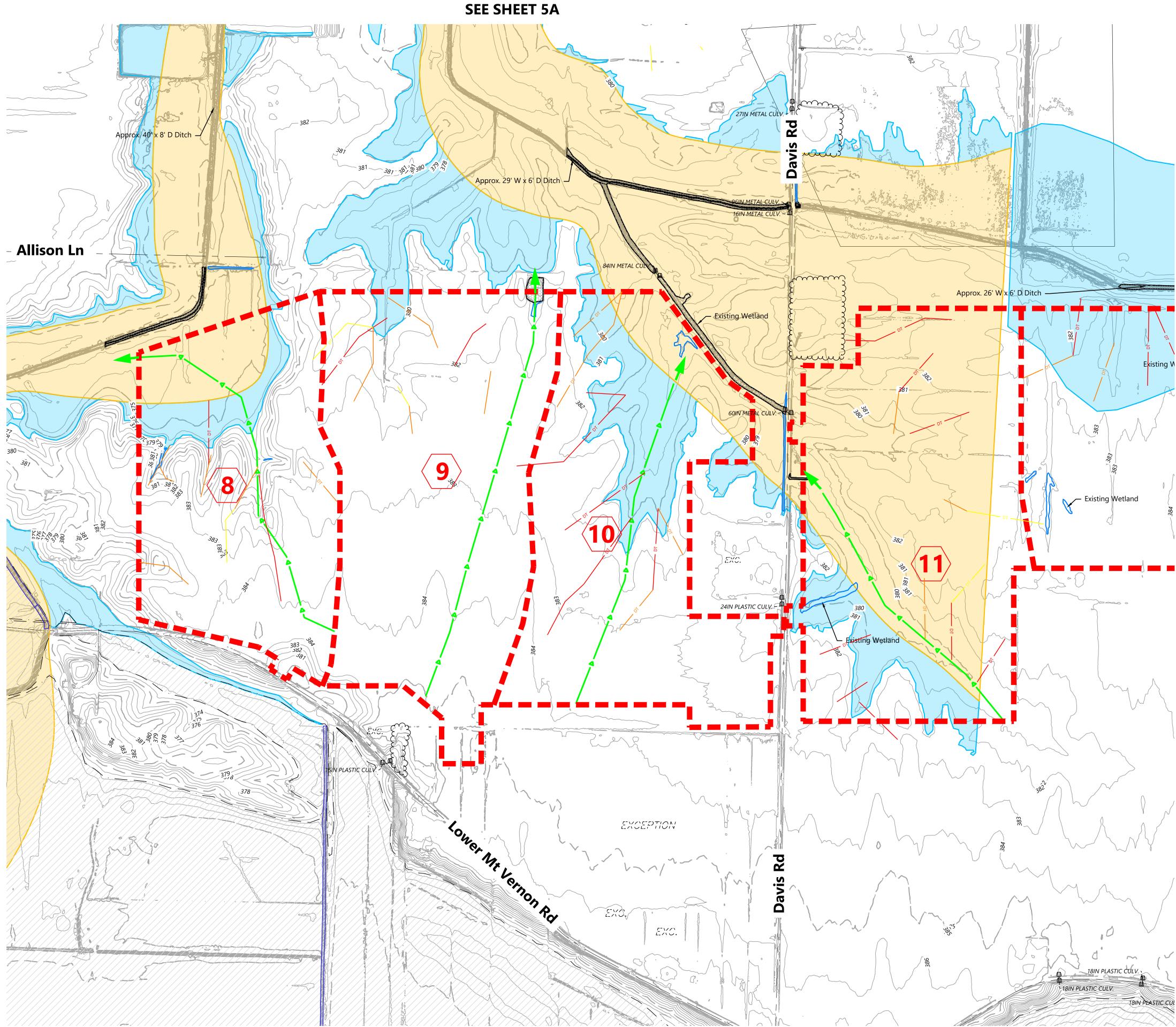
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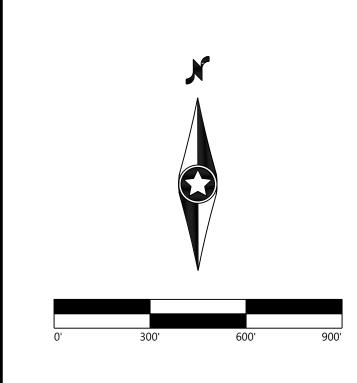
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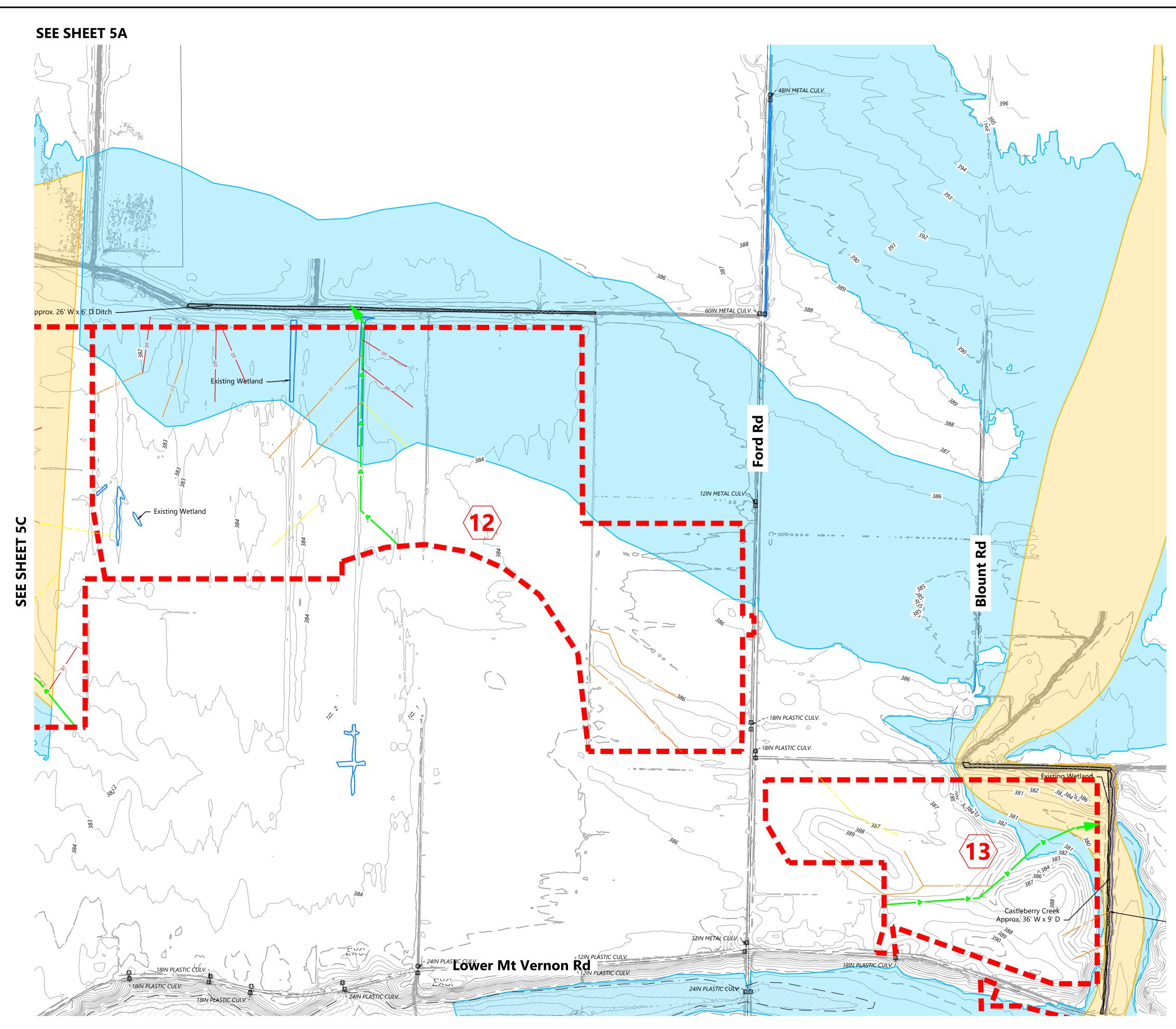
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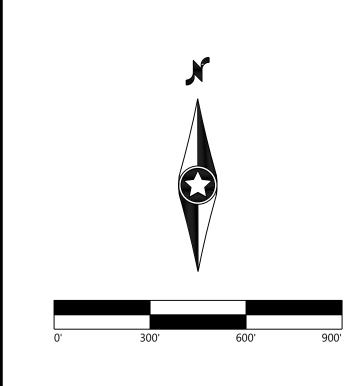
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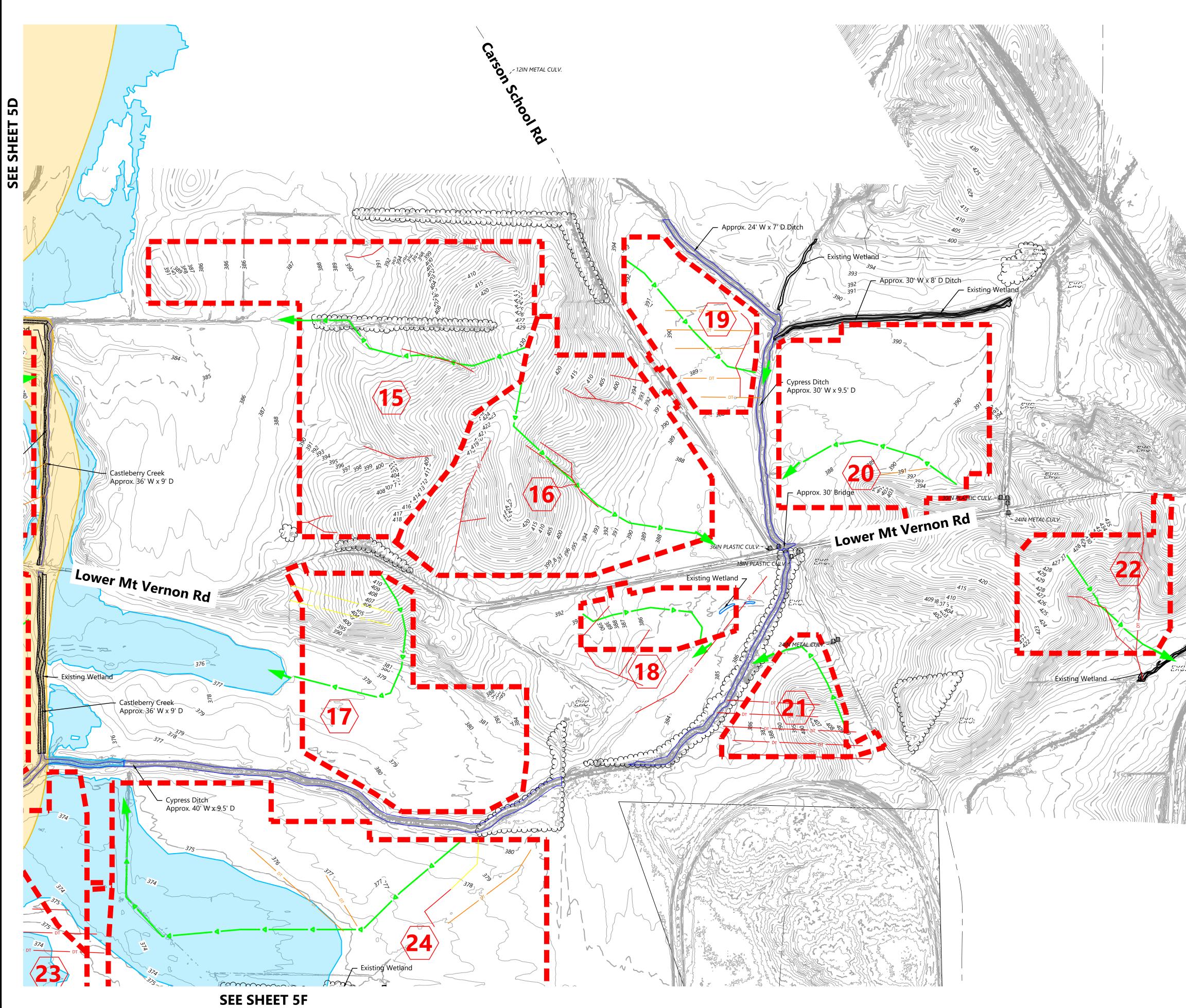
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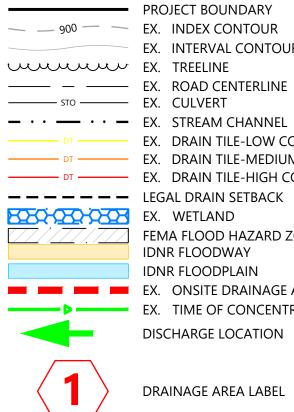
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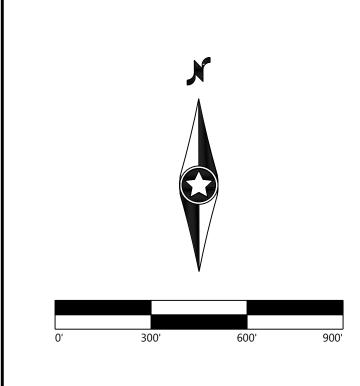
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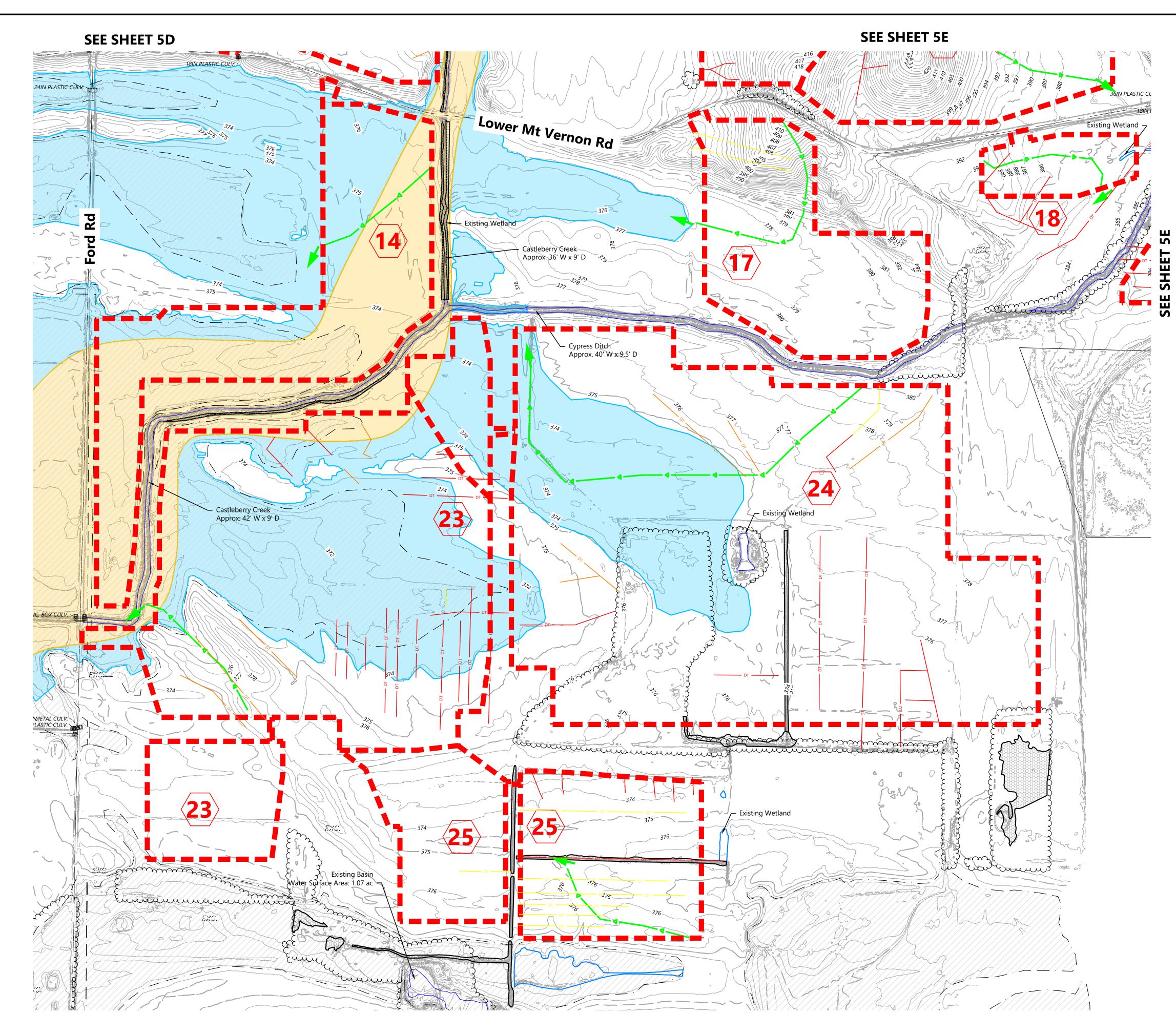
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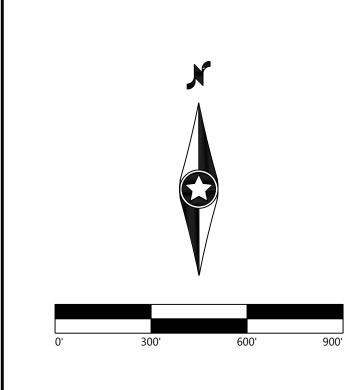
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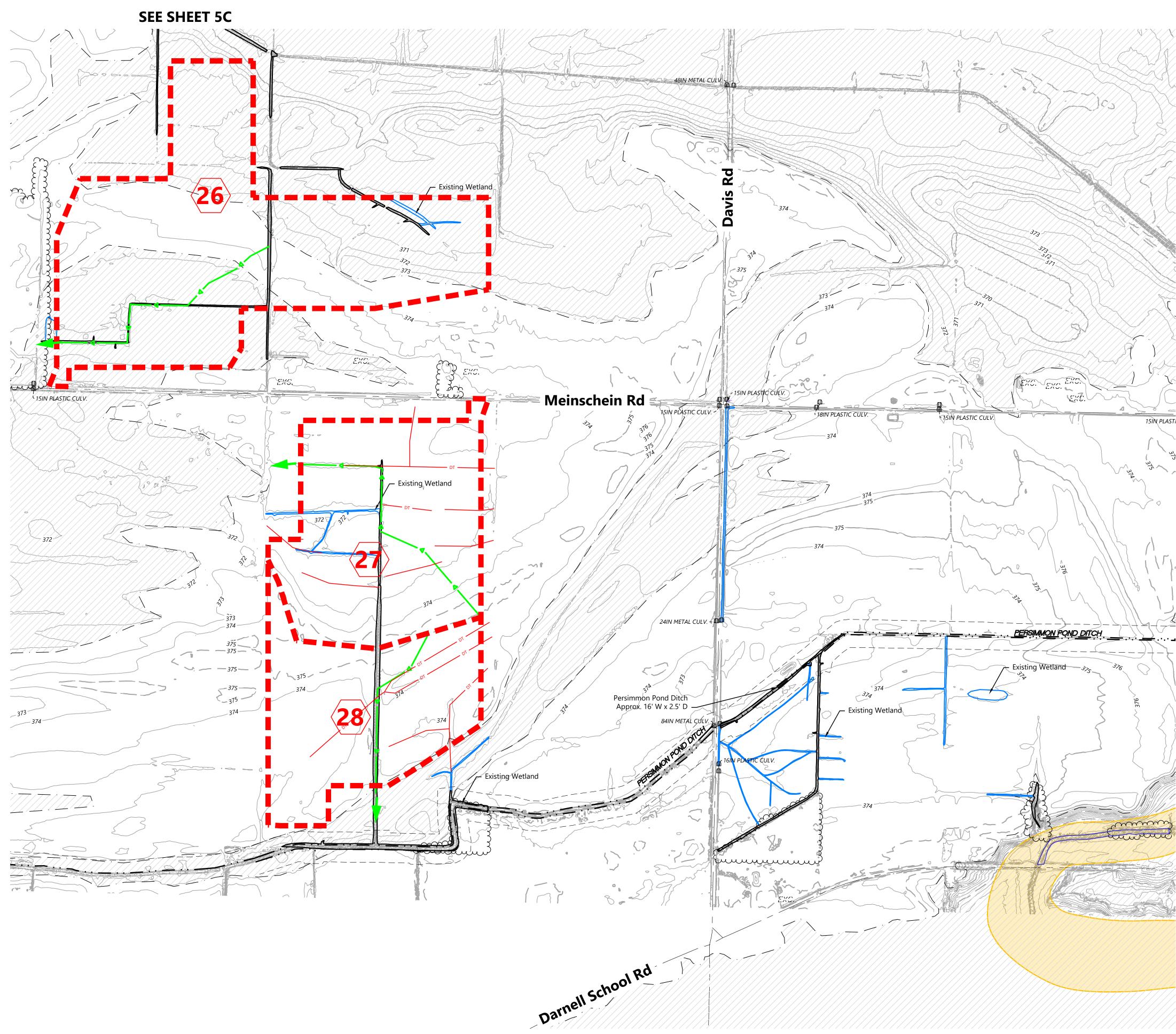
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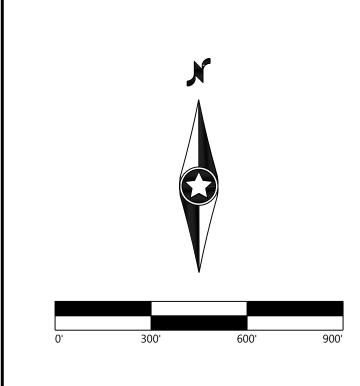
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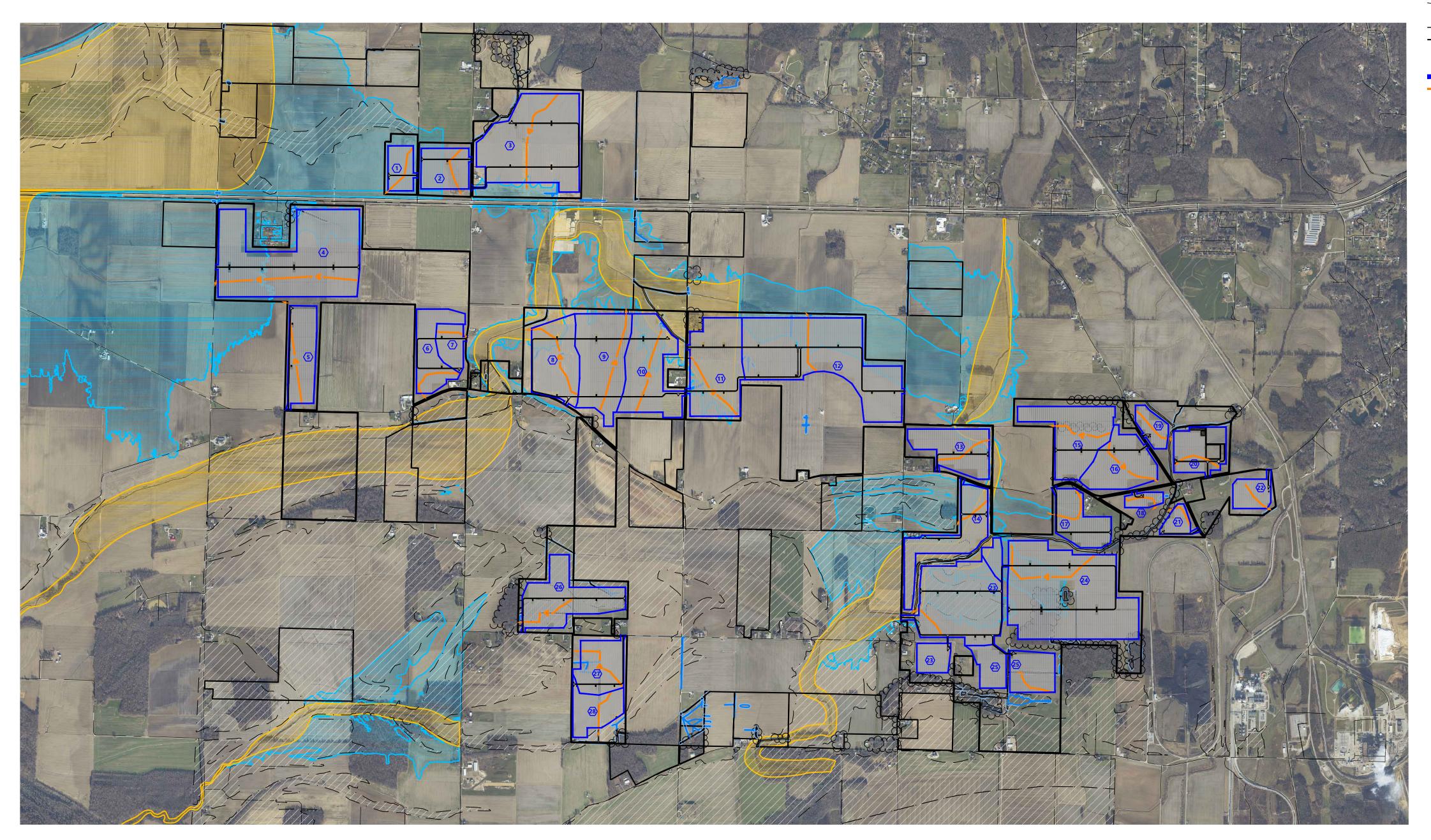
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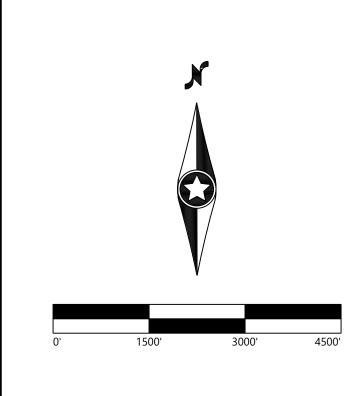
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Proposed Drainage Map - Overall

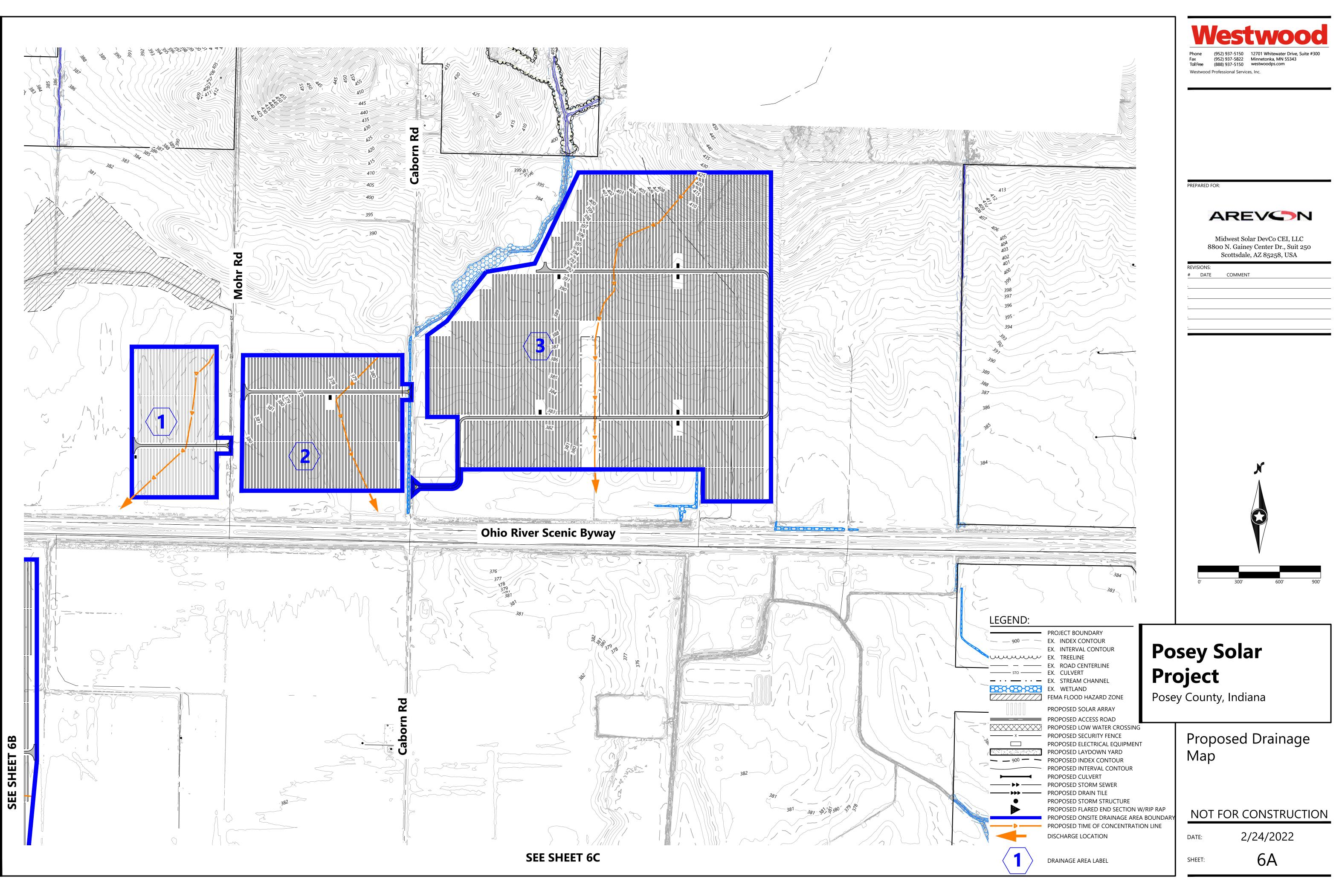
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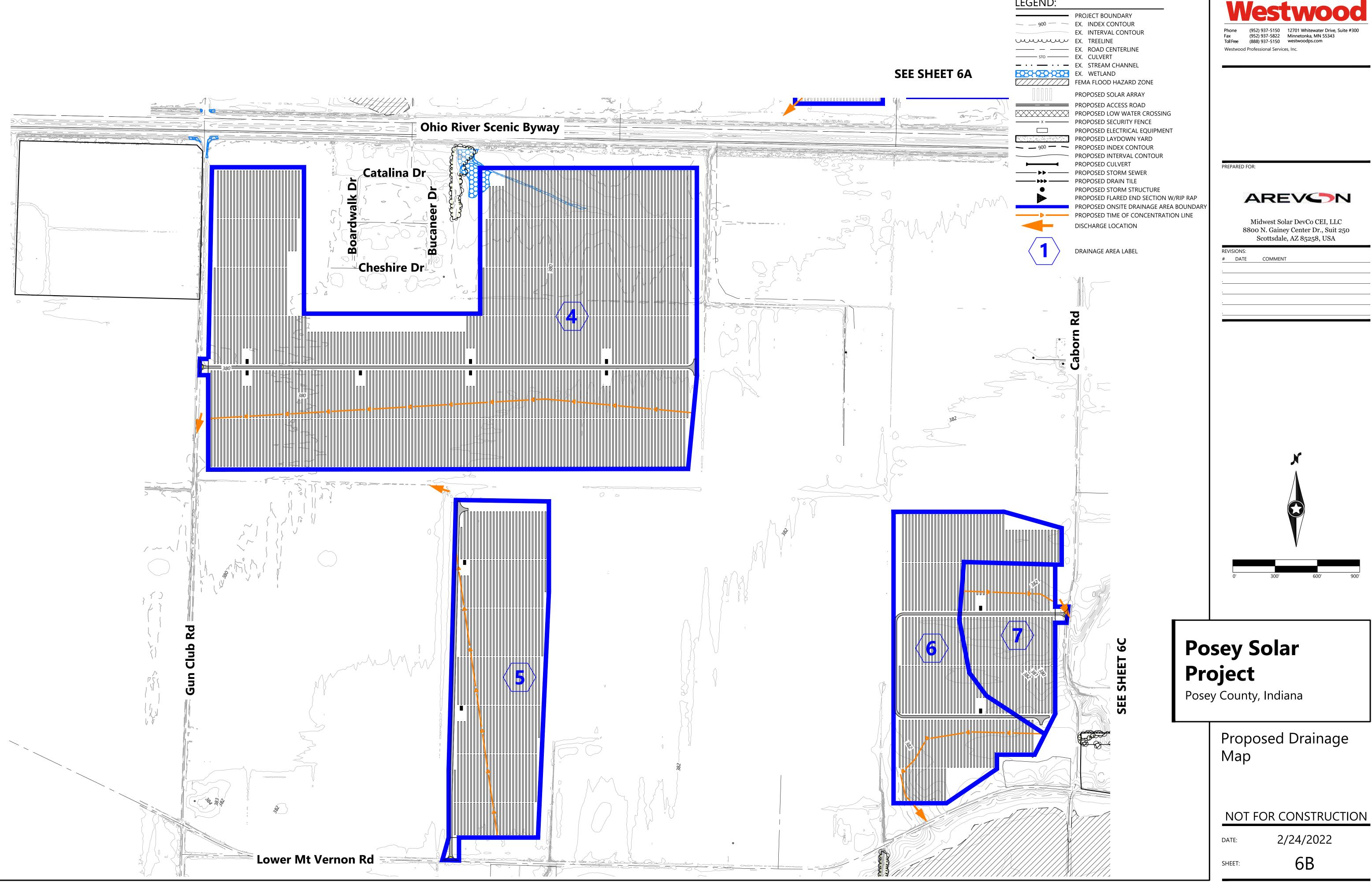
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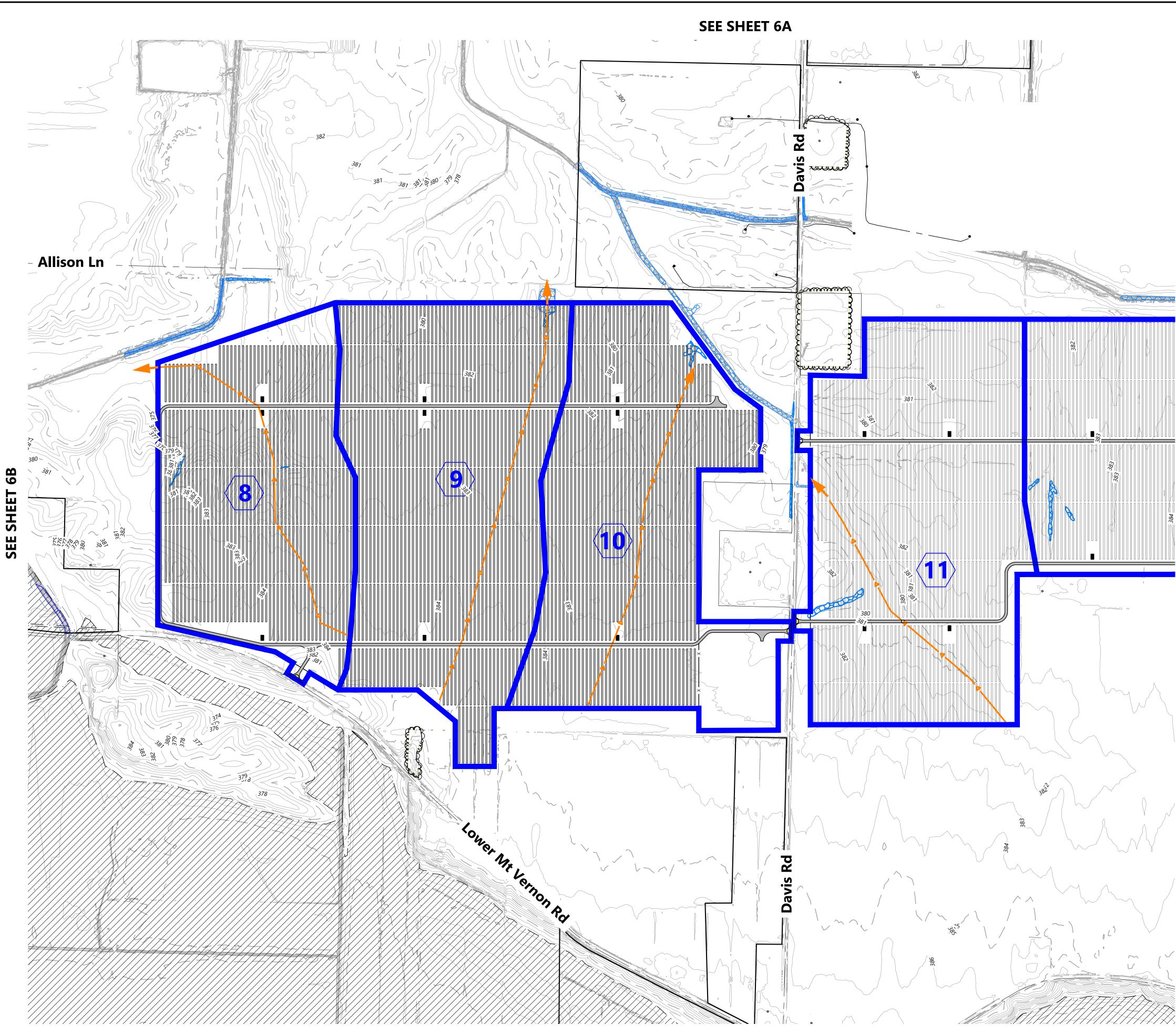
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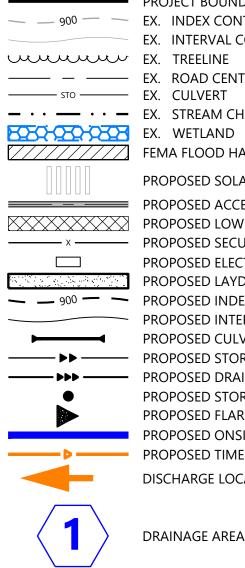
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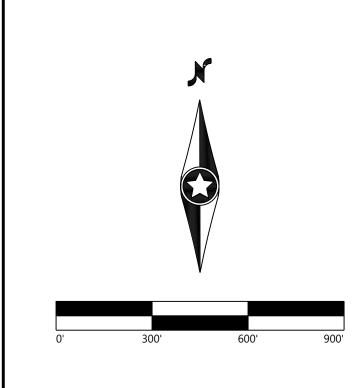
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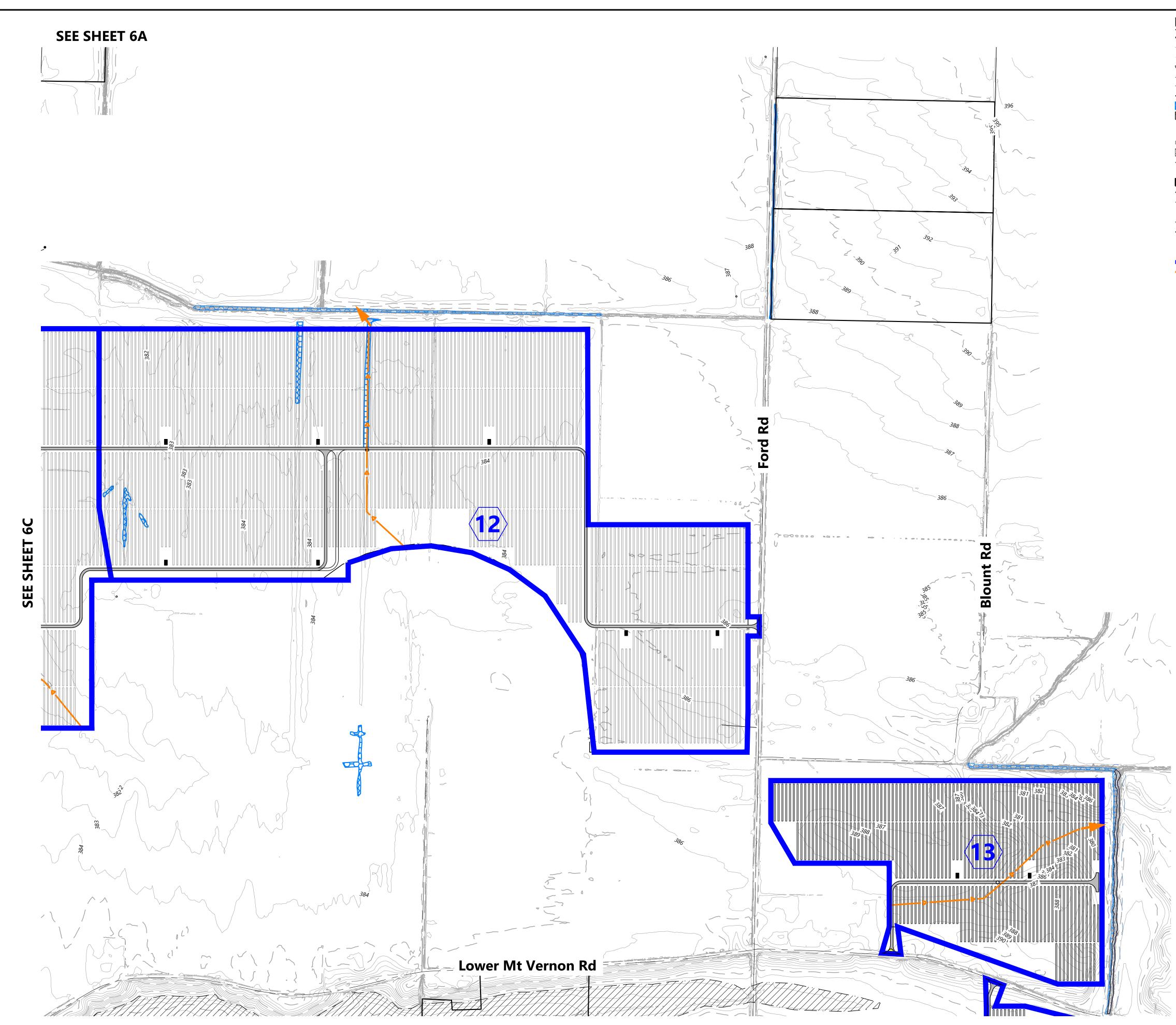
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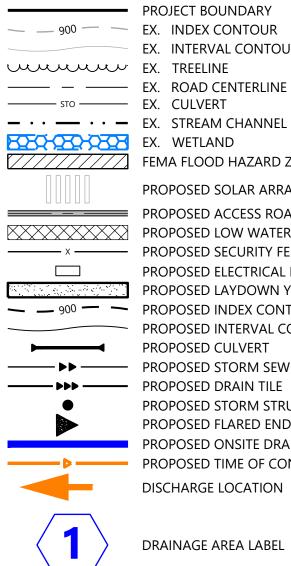
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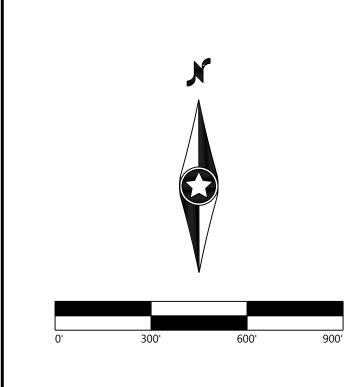
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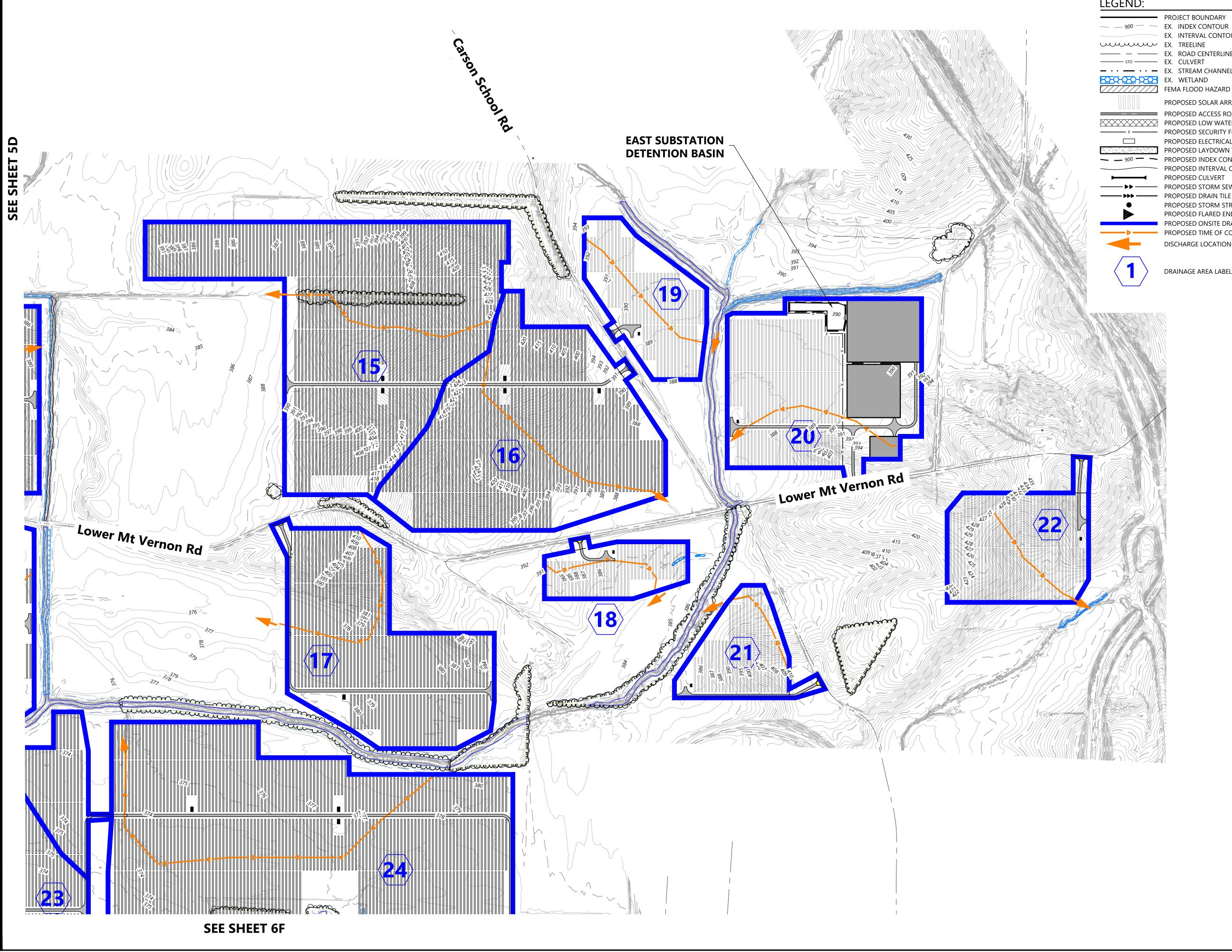
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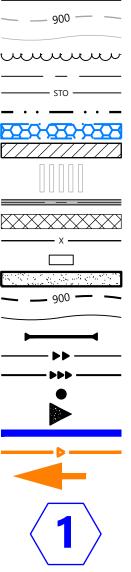
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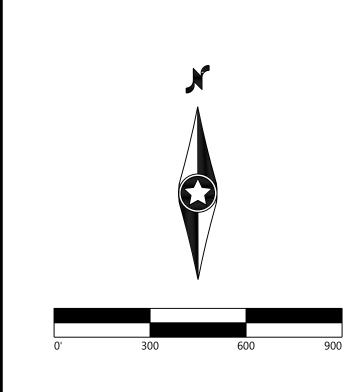
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Midwest Solar DevCo CEI, LLC 8800 N. Gainey Center Dr., Suit 250 Scottsdale, AZ 85258, USA

REVISIONS: # DATE COMMENT



Posey Solar Project

Posey County, Indiana

Proposed Drainage Мар

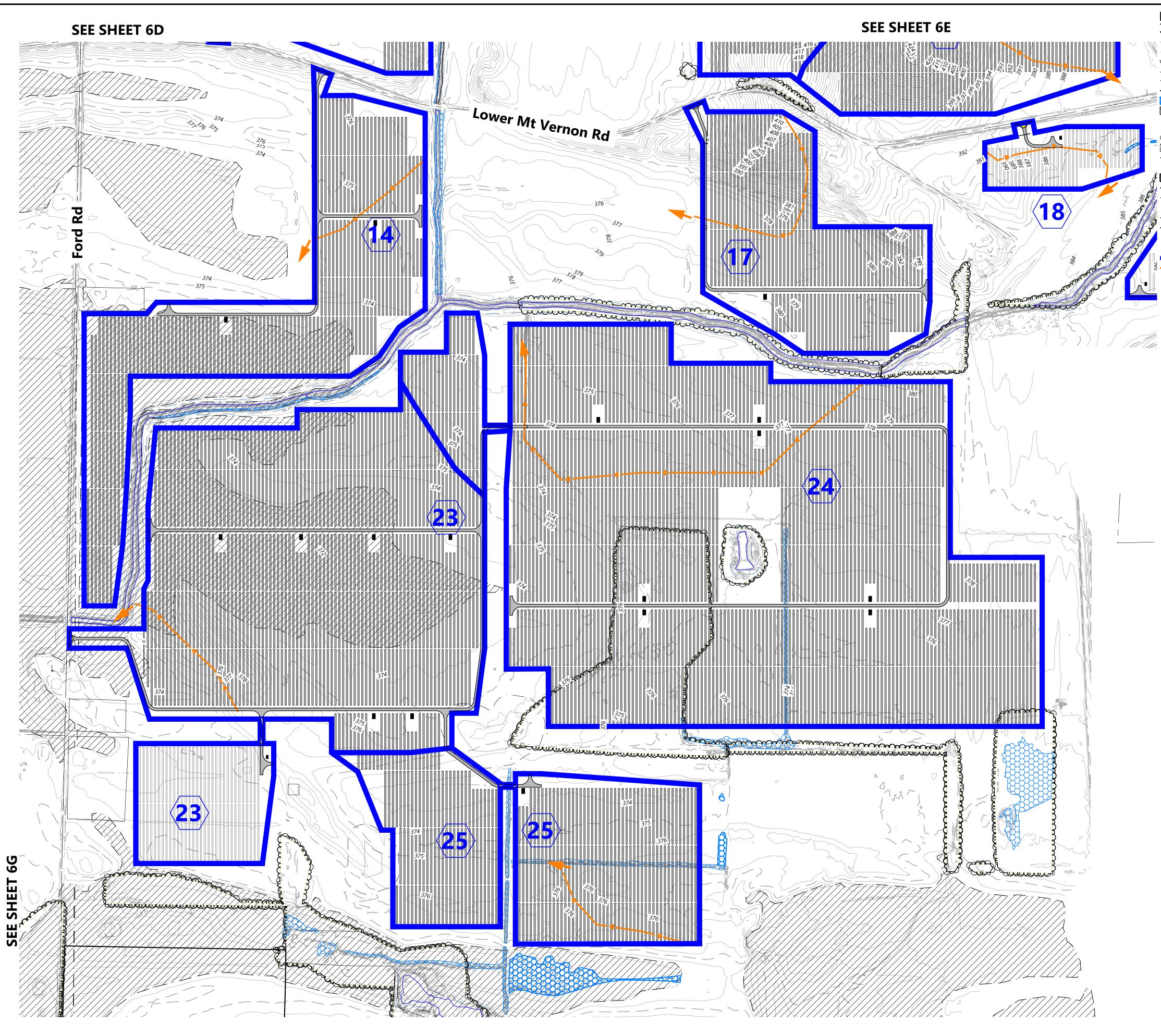
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PROJECT BOUNDARY EX. INDEX CONTOUR EX. INTERVAL CONTOUR EX. TREELINE EX. ROAD CENTERLINE EX. CULVERT EX. STREAM CHANNEL ex. Wetland FEMA FLOOD HAZARD ZONE PROPOSED SOLAR ARRAY PROPOSED ACCESS ROAD PROPOSED LOW WATER CROSSING PROPOSED SECURITY FENCE PROPOSED ELECTRICAL EQUIPMENT PROPOSED LAYDOWN YARD PROPOSED INDEX CONTOUR PROPOSED INTERVAL CONTOUR PROPOSED CULVERT PROPOSED STORM SEWER PROPOSED DRAIN TILE PROPOSED STORM STRUCTURE PROPOSED FLARED END SECTION W/RIP RAP PROPOSED ONSITE DRAINAGE AREA BOUNDARY PROPOSED TIME OF CONCENTRATION LINE DISCHARGE LOCATION

DRAINAGE AREA LABEL

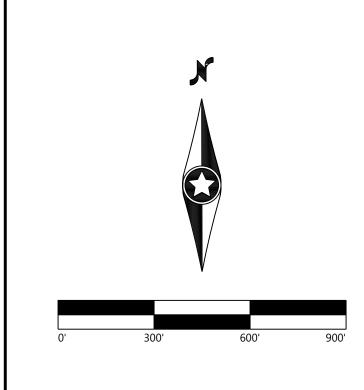
None(952) 937-515012701 Whitewater Drive, Suite #300Tax(952) 937-5822Minnetonka, MN 55343Toll Free(888) 937-5150Westwoodps.comWestwood Professional Services, Inc.

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Posey Solar Project

Posey County, Indiana

Proposed Drainage Map

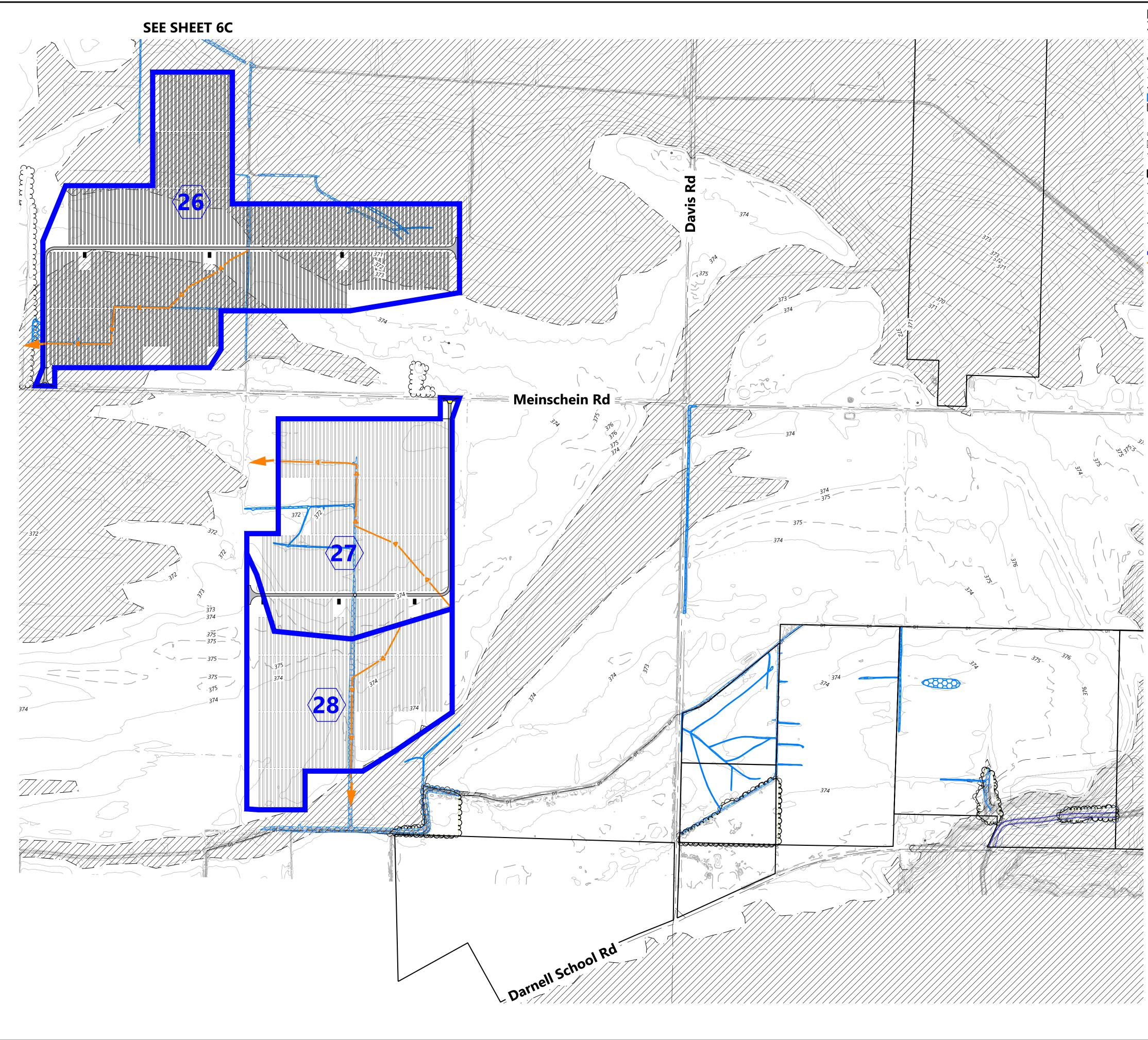
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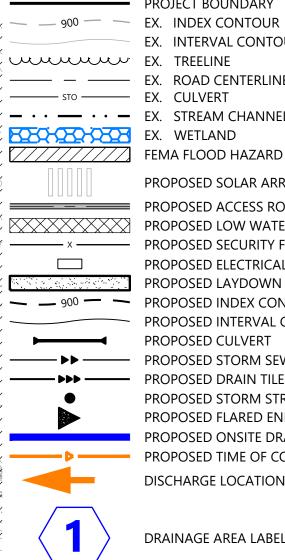
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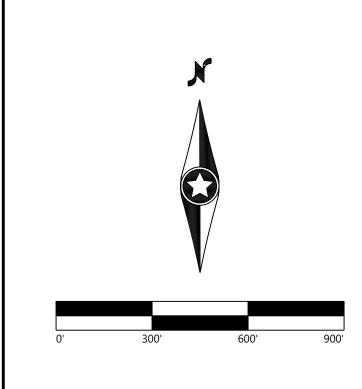
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Posey Solar Project

Posey County, Indiana

Proposed Drainage Мар

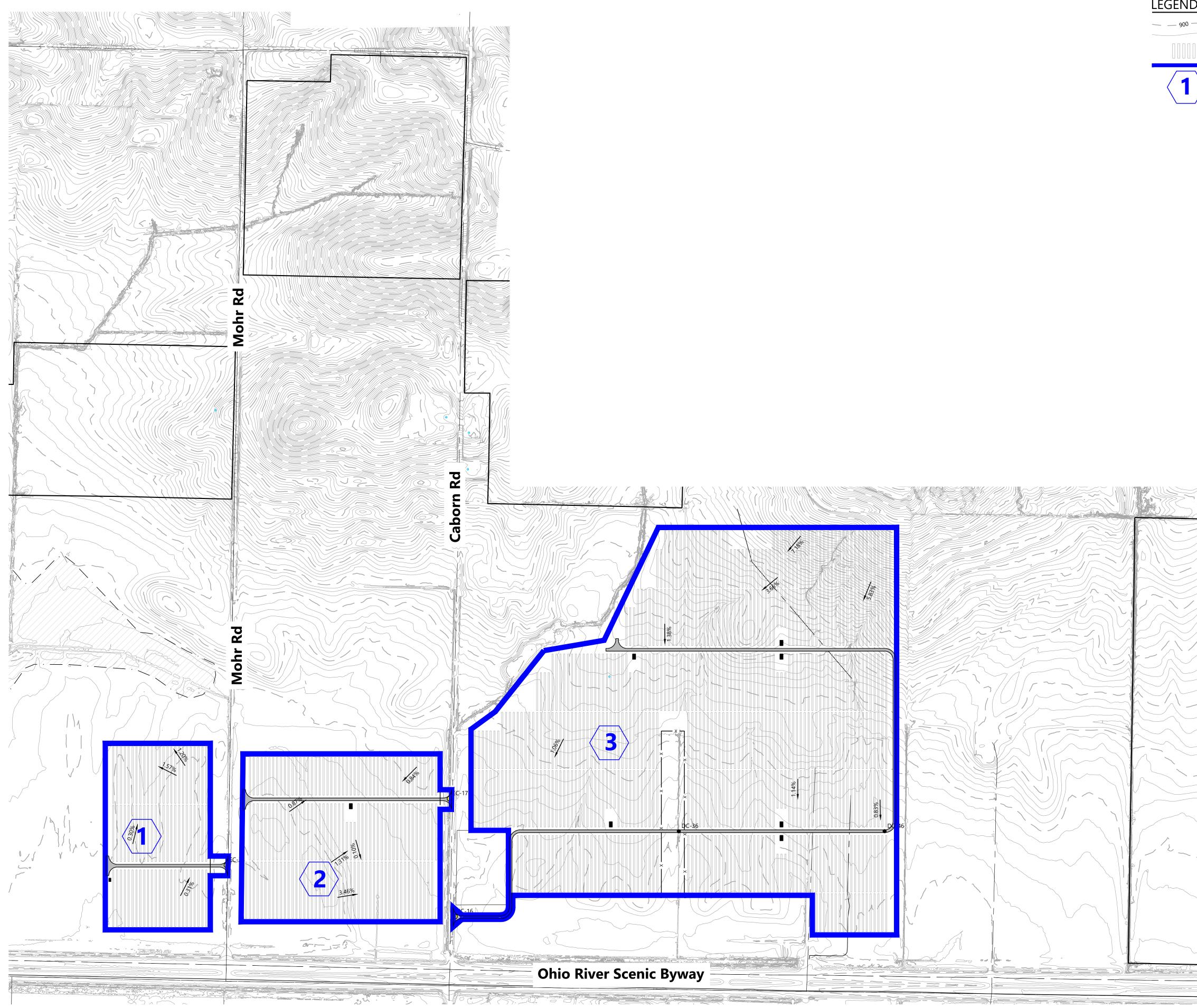
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*SEE SHEET 8H FOR AVERAGE AND MAXIMUM SLOPE PER DRAINAGE AREA SUMMARY TABLE



— _____ 900 — ____ EX. INDEX CONTOUR EX. INTERVAL CONTOUR PROPOSED SOLAR ARRAY

PROPOSED ONSITE DRAINAGE AREA BOUNDARY

DRAINAGE AREA LABEL



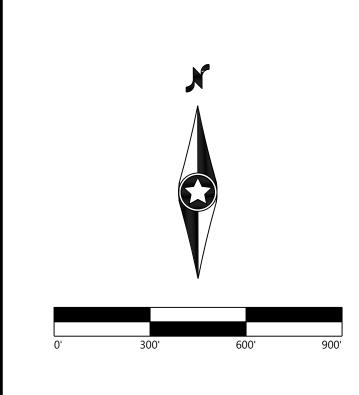
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Posey Solar Project Posey County, Indiana

Slope Analysis Map

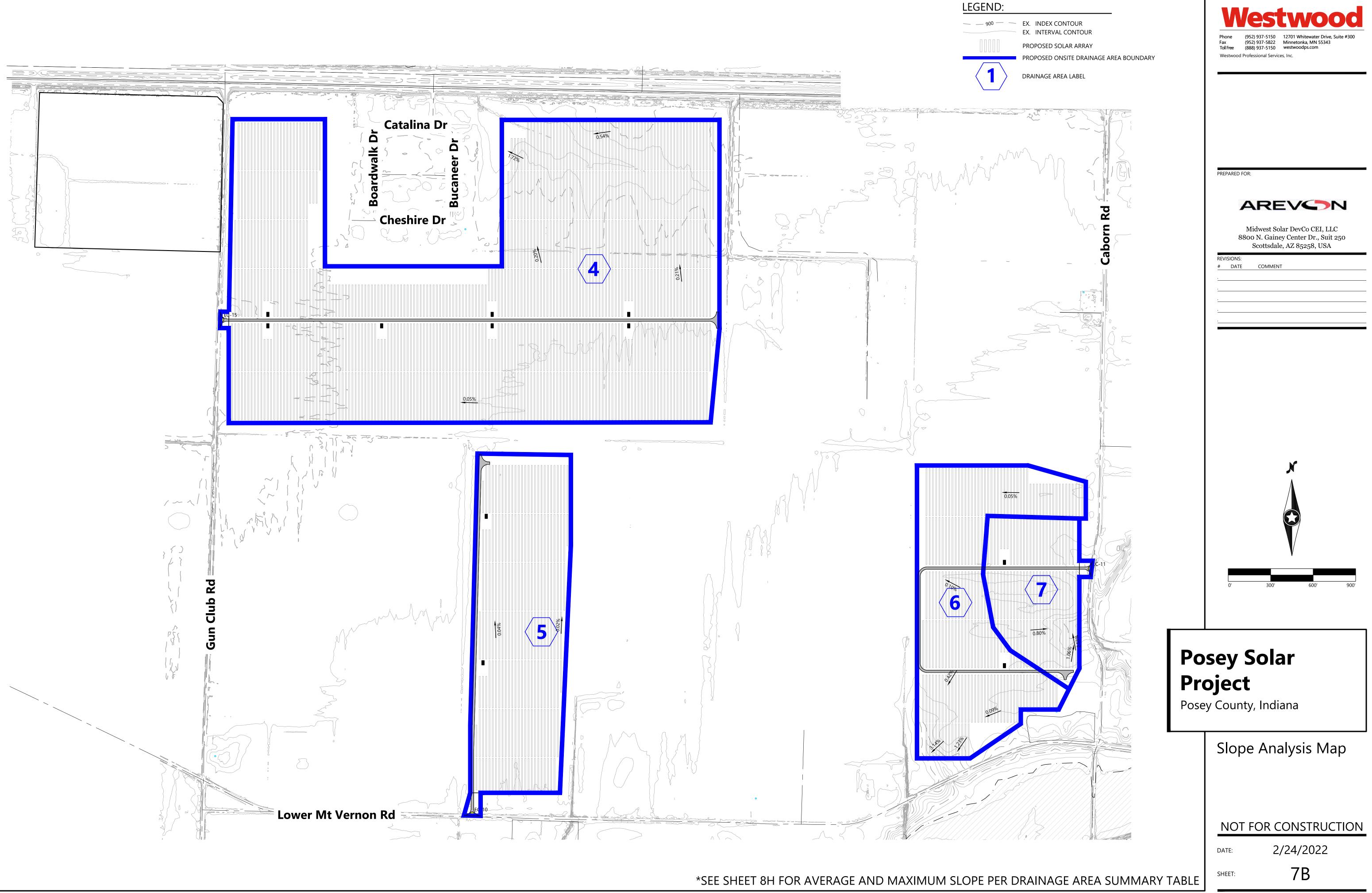
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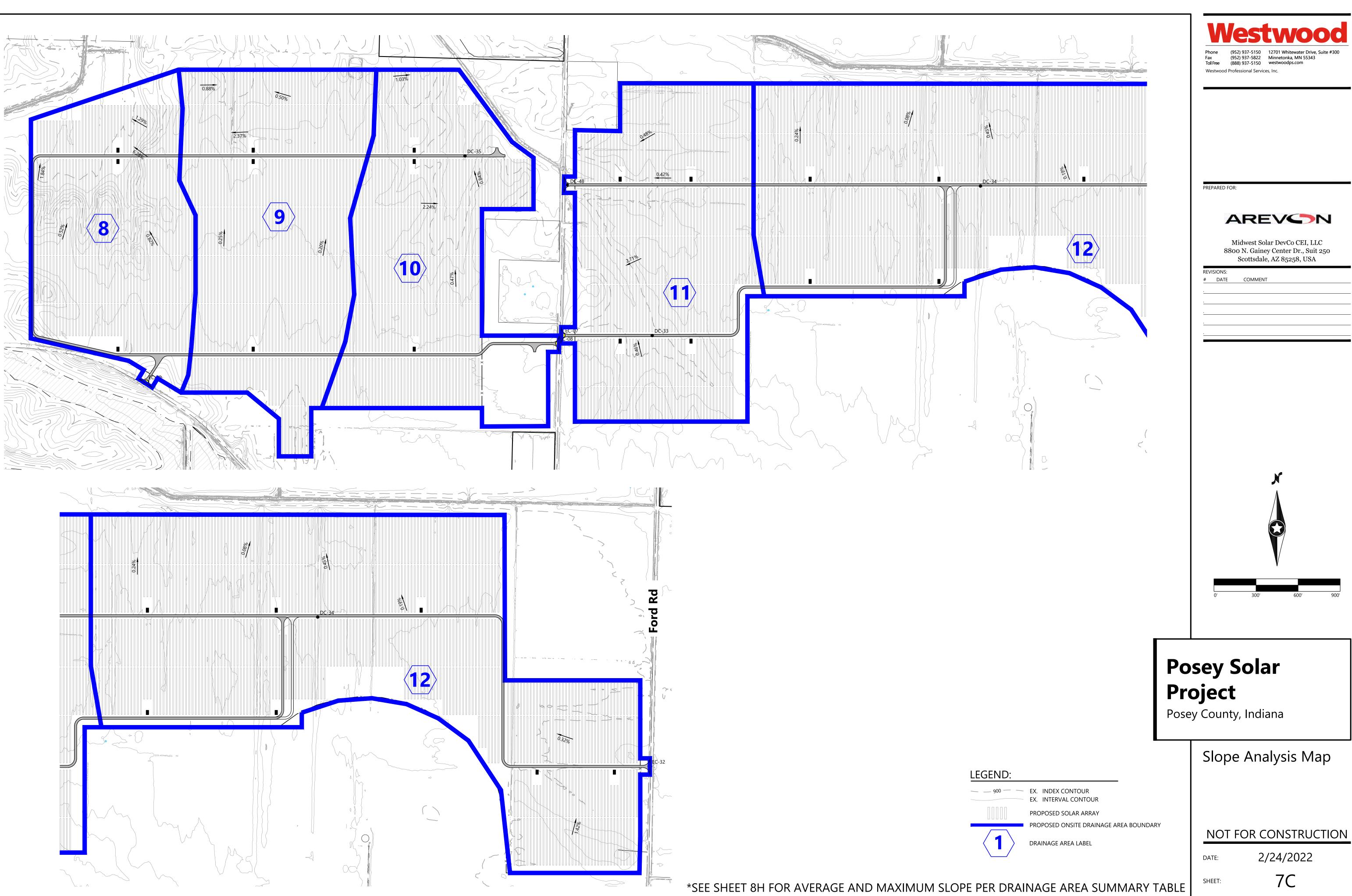
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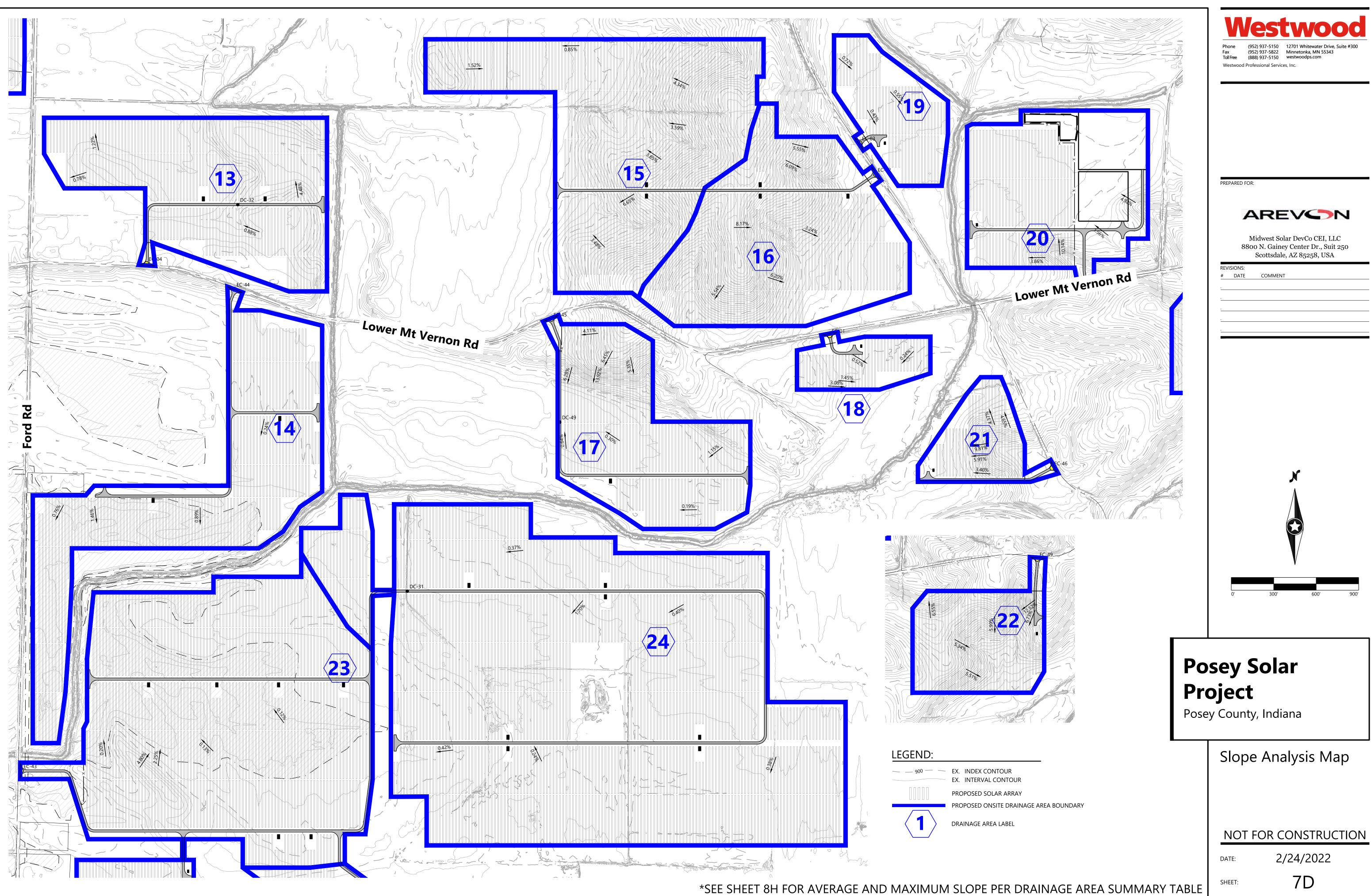
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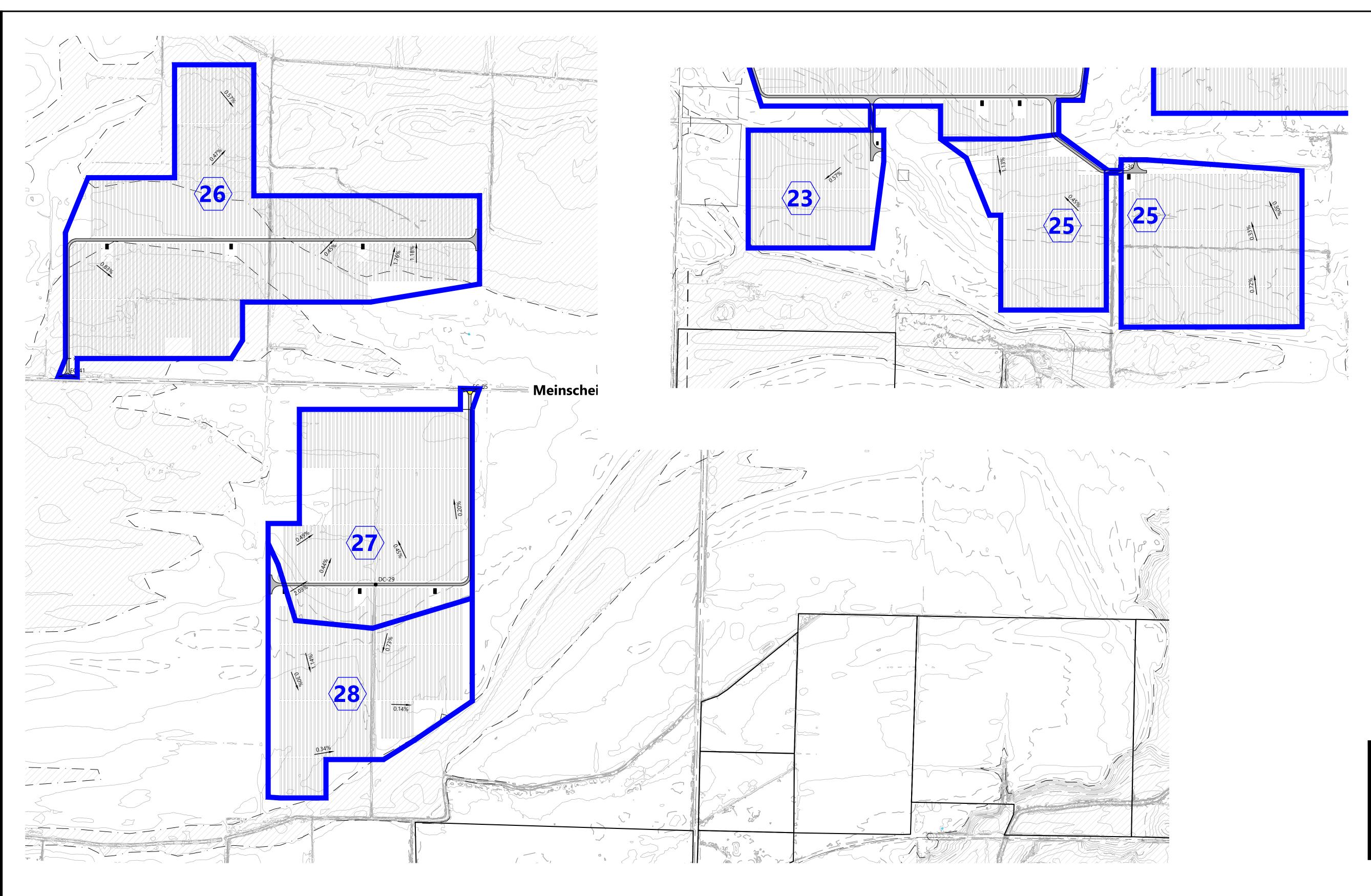


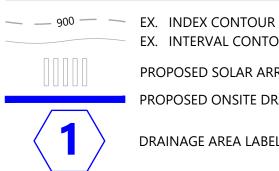


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EX. INTERVAL CONTOUR PROPOSED SOLAR ARRAY PROPOSED ONSITE DRAINAGE AREA BOUNDARY

DRAINAGE AREA LABEL

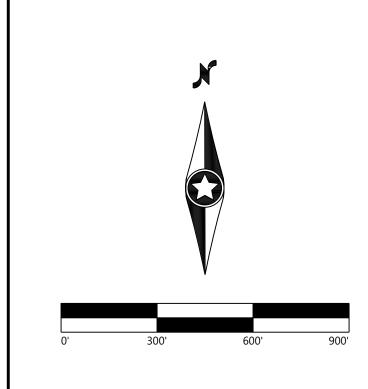


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Posey Solar Project

Posey County, Indiana

Slope Analysis Map

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2/24/2022

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Area	Average Slope	Max Slope	Sheet
1	0.60%	1.57%	8A
2	0.75%	3.46%	8A
3	2.27%	7.18%	8A
4	0.25%	1.72%	8B
5	0.03%	0.04%	8B
6	0.38%	3.14%	8B
7	0.77%	2.53%	8B
8	1.12%	6.28%	8C
9	0.46%	2.37%	8C
10	0.66%	2.24%	8C
11	0.44%	2.71%	8C
12	0.19%	1.42%	8C
13	1.58%	4.48%	8D
14	0.63%	1.46%	8D
15	3.10%	6.65%	8D
16	4.99%	8.17%	8D
17	3.06%	13.92%	8D
18	0.74%	3.08%	8D
19	0.49%	0.72%	8D
20	2.69%	10.14%	8D
21	4.58%	9.81%	8D
22	6.02%	12.92%	8D
23	0.65%	4.80%	8D/8E
24	0.36%	1.70%	8D
25	0.69%	1.30%	8E
26	0.70%	1.76%	8E
27	0.40%	2.03%	8E
28	0.46%	1.14%	8E

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Posey Solar Project Posey County, Indiana

Slope Analysis Map

NOT FOR CONSTRUCTION

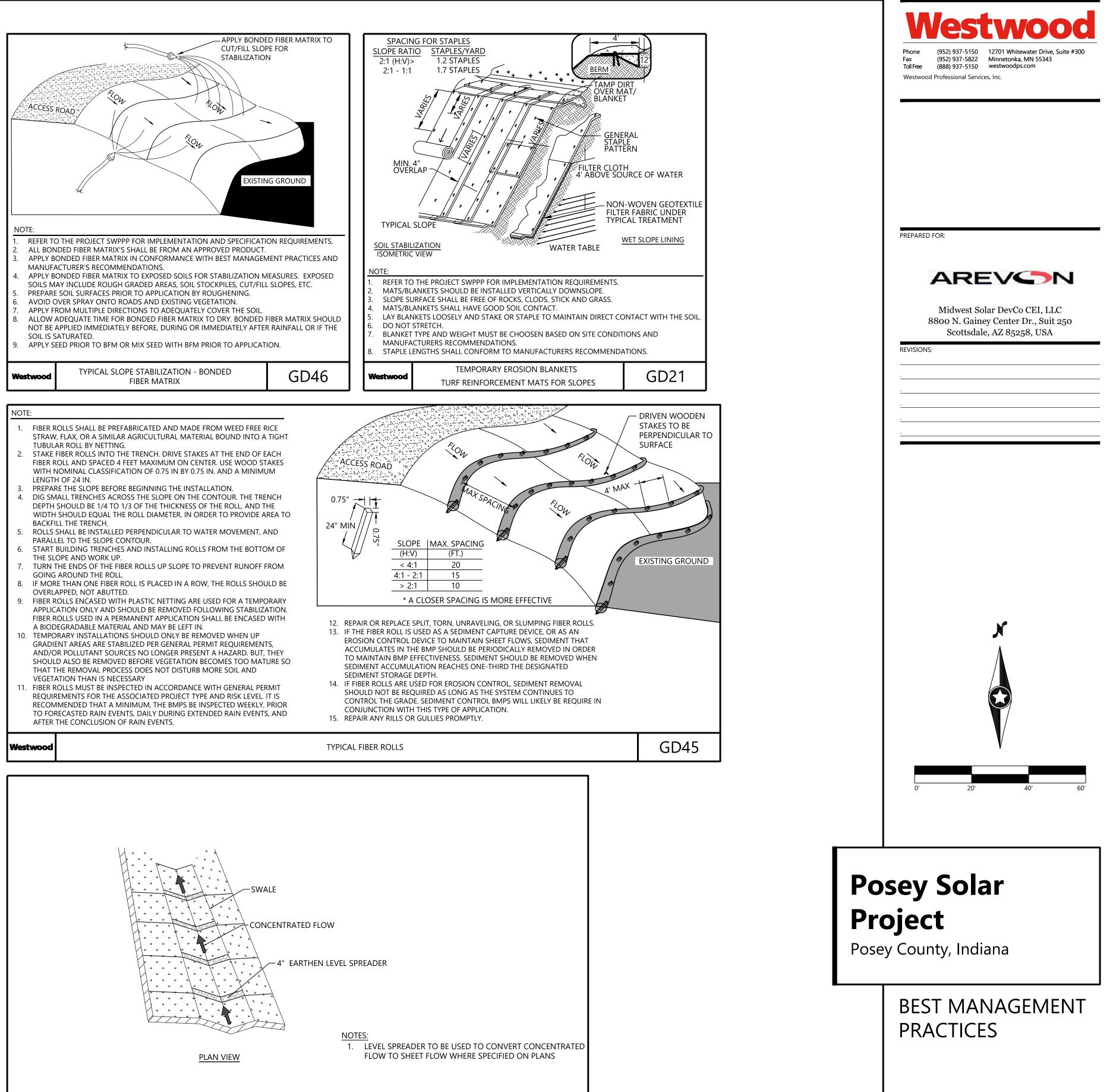
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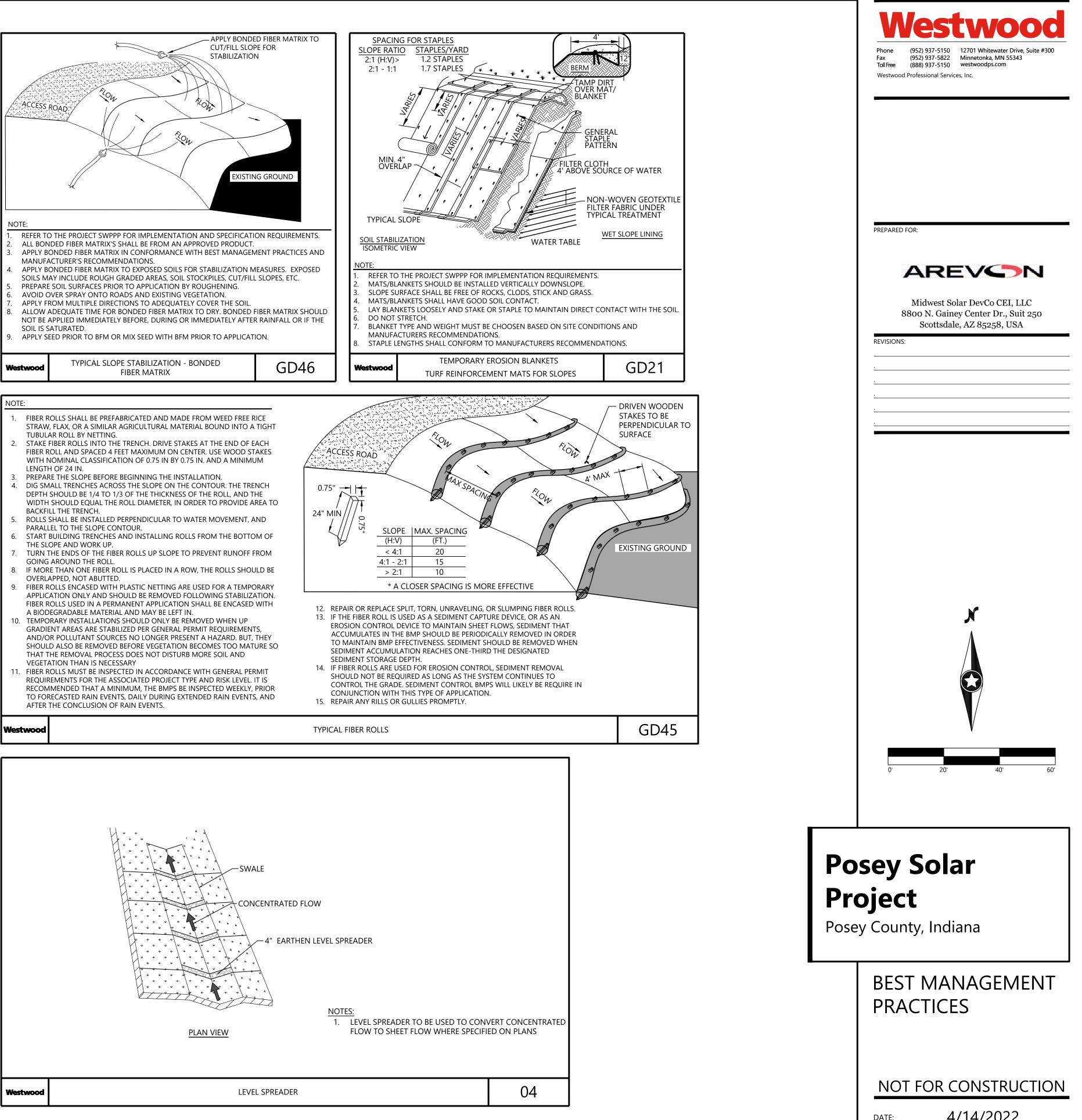
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LEGEND: EX. INDEX CONTOUR ____ 900 -EX. INTERVAL CONTOUR PROPOSED SOLAR ARRAY BMP TREATMENT AREA * * * TYPICAL DRIP EDGE BMP APPROACH: 1. PRESEED AREA BEFORE PILE CONSTRUCTION, INSTALL PILES AND RESEED WITH PERMANENT COVER. IF FAILURE OCCURS PROCEED TO STEP 2. RESEED AREAS OF FAILURE WITH HYDROMULCH OF BONDED FIBER MATRIX. IF FAILURE OCCURS PROCEED TO STEP 3. CORRECTLY INSTALL EROSION CONTROL BLANKET WITH FIBER ROLLS AT AREAS OF FAILURE. BLANKET PRODUCT TO BE ANALYZED FOR SLOPE AND SHEAR STRESS AND FIBER ROLLS TO BE PLACED AT CORRECT INTERVALS BASED ON SLOPE. IF FAILURE OCCURS PROCEED TO STEP 4. RESEED AND INSTALL 4" LEVEL SPREADER AT APPROPRIATE FAILURE LOCATIONS TO ENCOURAGE SHEET FLOW. INTERVALS TO BE REVIEWED AND RECOMMENDED BASED ON SLOPE AND CONTRIBUTING WATERSHED.





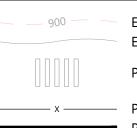


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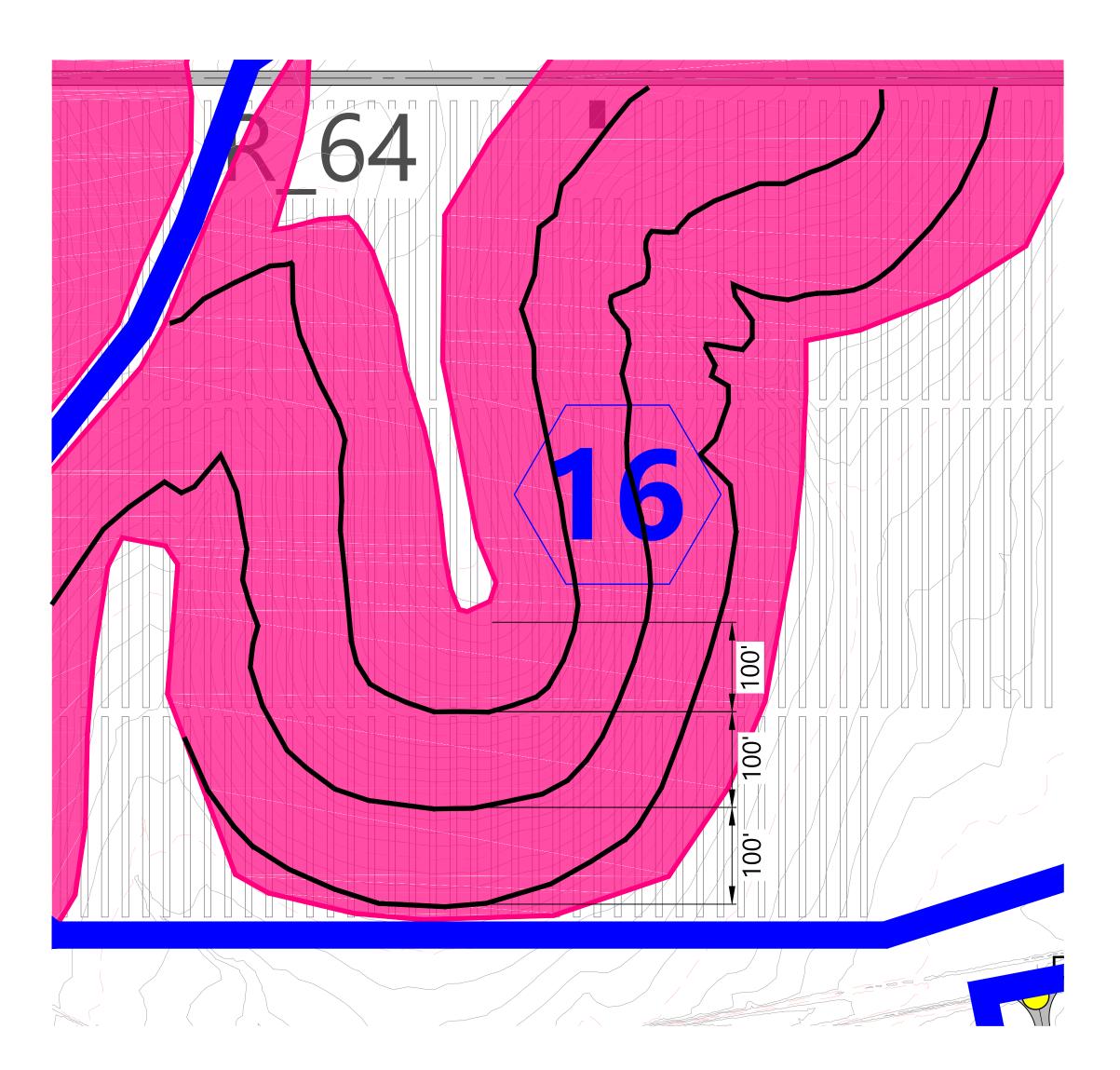
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EX. INDEX CONTOUR EX. INTERVAL CONTOUR PROPOSED SOLAR ARRAY

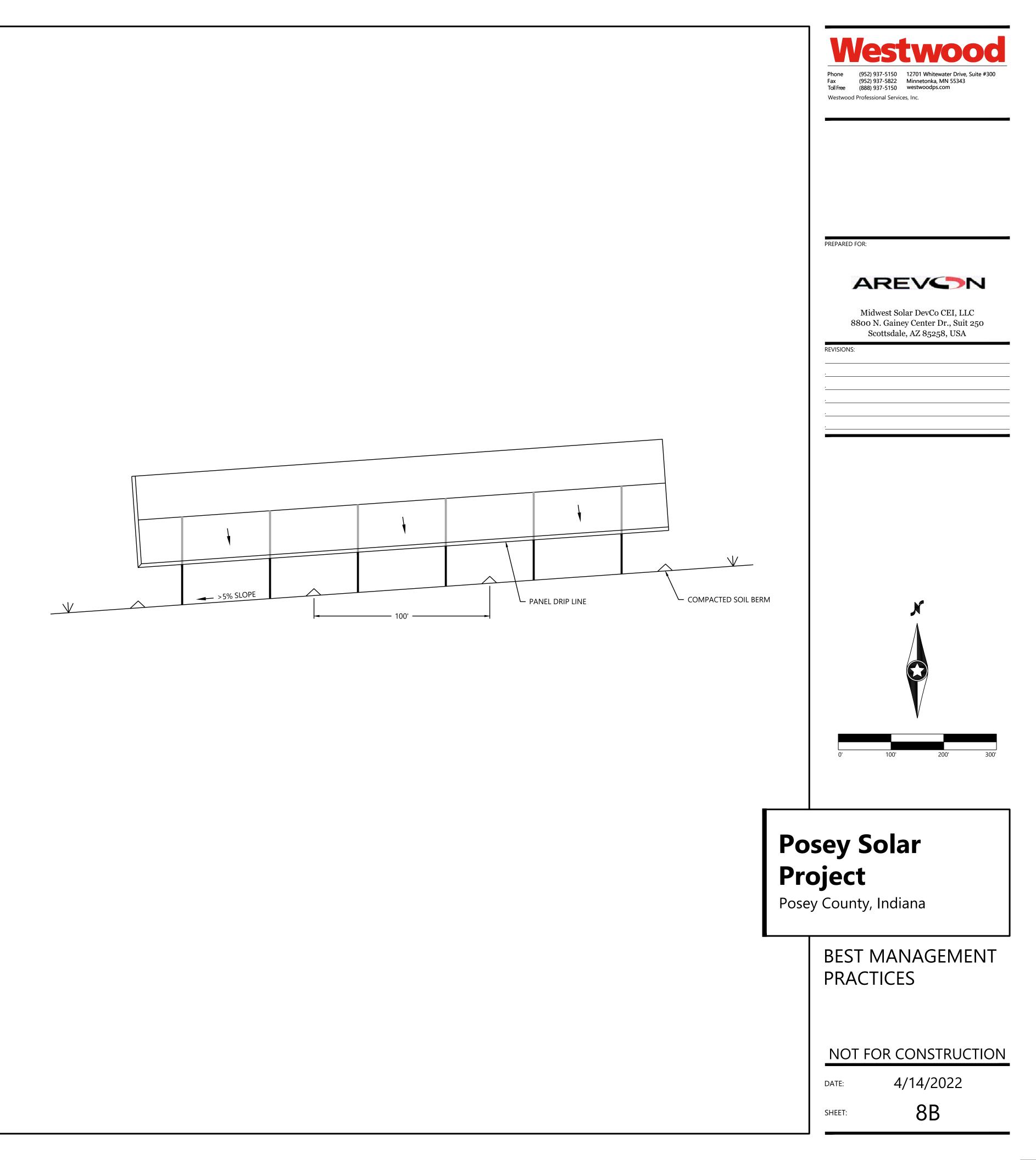


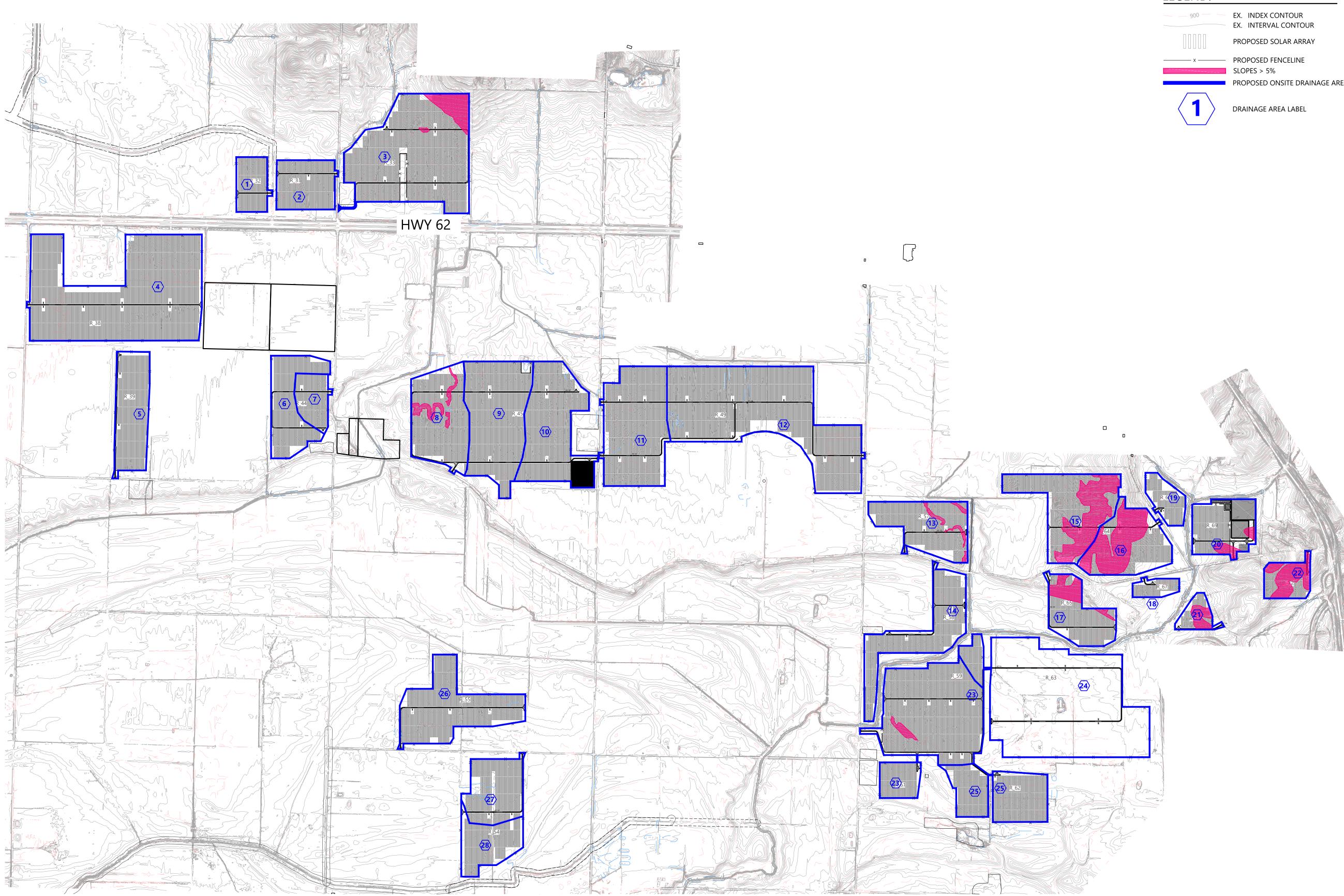
DRAINAGE AREA LABEL



LEVEL SPREADER DESIGN - PROPOSED FOR ALL AREAS WITH UNINTERRUPTED SLOPES >5% THAT ARE >100 FEET IN LENGTH

ALTERNATIVELY STORM WATER PONDS DESIGNED TO MEET RUNOFF RATE REQUIREMENTS MAY BE UTILIZED IN AREAS >5% ASSUMING AREAS UNDER THE PANELS ARE IMPERVIOUS





*AREAS SHOWN ON EXHIBIT ARE EXISTING SLOPES. FINAL DETERMINIATION WILL BE MADE BASED OFF PROPOSED GRADING.

LEGEND:



PROPOSED ONSITE DRAINAGE AREA BOUNDARY

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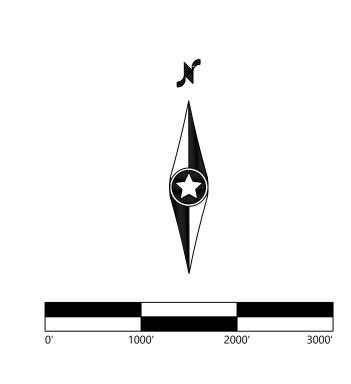
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REVISIONS:



Posey Solar Project

Posey County, Indiana

BEST MANAGEMENT PRACTICES

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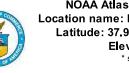
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Appendix A NOAA Atlas 14 Precipitation Data



NOAA Atlas 14, Volume 2, Version 3 Location name: Mount Vernon, Indiana, USA* Latitude: 37.9198°, Longitude: -87.7712° Elevation: 369.98 ft** source: ESRI Maps ** source: USGS



POINT PRECIPITATION FREQUENCY ESTIMATES

G.M. Bonnin, D. Martin, B. Lin, T. Parzybok, M.Yekta, and D. Riley

NOAA, National Weather Service, Silver Spring, Maryland

PF_tabular | PF_graphical | Maps_&_aerials

PF tabular

PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches) ¹										
Duration				Average	e recurrence	e interval (y	ears)			
Duration	1	2	5	10	25	50	100	200	500	1000
5-min	0.395	0.466	0.552	0.622	0.710	0.779	0.847	0.918	1.01	1.08
	(0.361-0.434)	(0.427-0.513)	(0.504-0.606)	(0.566-0.680)	(0.643-0.775)	(0.702-0.850)	(0.759-0.922)	(0.818-1.00)	(0.896-1.11)	(0.954-1.18)
10-min	0.614	0.728	0.858	0.960	1.09	1.18	1.27	1.37	1.49	1.58
	(0.561-0.674)	(0.666-0.801)	(0.784-0.941)	(0.873-1.05)	(0.983-1.19)	(1.06-1.29)	(1.14-1.39)	(1.22-1.49)	(1.32-1.63)	(1.39-1.72)
15-min	0.752	0.890	1.05	1.18	1.34	1.46	1.58	1.70	1.86	1.97
	(0.688-0.826)	(0.814-0.979)	(0.963-1.16)	(1.07-1.29)	(1.21-1.47)	(1.32-1.59)	(1.42-1.73)	(1.52-1.86)	(1.65-2.03)	(1.74-2.15)
30-min	0.995	1.19	1.44	1.64	1.89	2.09	2.29	2.49	2.75	2.96
	(0.910-1.09)	(1.09-1.31)	(1.32-1.58)	(1.49-1.79)	(1.72-2.07)	(1.88-2.28)	(2.05-2.49)	(2.22-2.71)	(2.44-3.01)	(2.60-3.23)
60-min	1.22	1.46	1.81	2.09	2.46	2.75	3.06	3.37	3.81	4.15
	(1.11-1.34)	(1.34-1.61)	(1.65-1.99)	(1.90-2.28)	(2.22-2.68)	(2.48-3.00)	(2.74-3.33)	(3.01-3.68)	(3.37-4.16)	(3.65-4.54)
2-hr	1.47	1.78	2.22	2.57	3.06	3.45	3.86	4.29	4.87	5.34
	(1.35-1.61)	(1.63-1.95)	(2.04-2.43)	(2.35-2.81)	(2.78-3.33)	(3.13-3.75)	(3.48-4.19)	(3.84-4.66)	(4.34-5.30)	(4.72-5.81)
3-hr	1.58	1.91	2.39	2.78	3.32	3.76	4.23	4.72	5.42	5.98
	(1.45-1.73)	(1.75-2.09)	(2.18-2.61)	(2.53-3.03)	(3.01-3.62)	(3.40-4.10)	(3.80-4.60)	(4.22-5.13)	(4.79-5.89)	(5.25-6.50)
6-hr	1.94	2.33	2.91	3.38	4.05	4.60	5.18	5.80	6.68	7.39
	(1.78-2.12)	(2.14-2.56)	(2.66-3.19)	(3.09-3.70)	(3.68-4.42)	(4.15-5.01)	(4.66-5.64)	(5.18-6.31)	(5.90-7.27)	(6.47-8.06)
12-hr	2.29	2.76	3.42	3.97	4.73	5.36	6.02	6.71	7.70	8.49
	(2.10-2.49)	(2.53-3.01)	(3.14-3.73)	(3.62-4.32)	(4.31-5.14)	(4.86-5.82)	(5.43-6.53)	(6.01-7.29)	(6.83-8.36)	(7.47-9.23)
24-hr	2.74	3.30	4.10	4.75	5.66	6.40	7.17	7.98	9.11	10.0
	(2.57-2.93)	(3.09-3.53)	(3.84-4.39)	(4.45-5.08)	(5.28-6.04)	(5.94-6.82)	(6.62-7.64)	(7.33-8.51)	(8.30-9.74)	(9.06-10.7)
2-day	3.25	3.90	4.86	5.65	6.77	7.70	8.69	9.76	11.3	12.5
	(3.03-3.49)	(3.64-4.20)	(4.53-5.22)	(5.25-6.06)	(6.27-7.27)	(7.10-8.27)	(7.96-9.34)	(8.88-10.5)	(10.2-12.2)	(11.2-13.6)
3-day	3.46 (3.23-3.72)	4.15 (3.87-4.46)	5.15 (4.80-5.54)	5.98 (5.56-6.43)	7.18 (6.65-7.72)	8.19 (7.55-8.80)	9.26 (8.49-9.97)	10.4 (9.49-11.2)	12.1 (10.9-13.1)	13.5 (12.0-14.6)
4-day	3.67	4.39	5.44	6.32	7.60	8.67	9.83	11.1	12.9	14.4
	(3.43-3.95)	(4.10-4.73)	(5.07-5.86)	(5.88-6.80)	(7.04-8.18)	(8.00-9.34)	(9.02-10.6)	(10.1-12.0)	(11.6-14.0)	(12.9-15.7)
7-day	4.26	5.10	6.31	7.31	8.77	9.99	11.3	12.7	14.7	16.4
	(3.97-4.59)	(4.75-5.51)	(5.87-6.81)	(6.79-7.89)	(8.10-9.46)	(9.19-10.8)	(10.3-12.2)	(11.5-13.7)	(13.2-16.0)	(14.6-17.9)
10-day	4.80	5.75	7.10	8.22	9.84	11.2	12.6	14.1	16.3	18.2
	(4.46-5.21)	(5.35-6.24)	(6.60-7.71)	(7.62-8.92)	(9.07-10.7)	(10.3-12.1)	(11.5-13.7)	(12.8-15.4)	(14.6-17.8)	(16.1-19.8)
20-day	6.62	7.86	9.42	10.7	12.4	13.8	15.2	16.7	18.7	20.2
	(6.24-7.05)	(7.40-8.37)	(8.86-10.0)	(10.0-11.4)	(11.6-13.2)	(12.9-14.7)	(14.1-16.2)	(15.4-17.8)	(17.1-20.0)	(18.4-21.7)
30-day	8.17	9.64	11.4	12.8	14.7	16.2	17.7	19.2	21.3	22.9
	(7.72-8.65)	(9.12-10.2)	(10.8-12.1)	(12.1-13.5)	(13.8-15.6)	(15.2-17.2)	(16.5-18.8)	(17.9-20.5)	(19.7-22.7)	(21.0-24.5)
45-day	10.2	12.0	14.0	15.6	17.8	19.5	21.2	22.8	25.1	26.8
	(9.72-10.8)	(11.4-12.7)	(13.3-14.8)	(14.8-16.5)	(16.8-18.8)	(18.4-20.5)	(19.9-22.3)	(21.4-24.2)	(23.3-26.6)	(24.8-28.5)
60-day	12.2 (11.6-12.8)	14.3 (13.6-15.1)	16.6 (15.8-17.5)	18.4 (17.5-19.4)	20.7 (19.7-21.9)	22.5 (21.3-23.8)	24.3 (22.9-25.6)	26.0 (24.4-27.5)	28.2 (26.4-29.9)	29.9 (27.8-31.8)

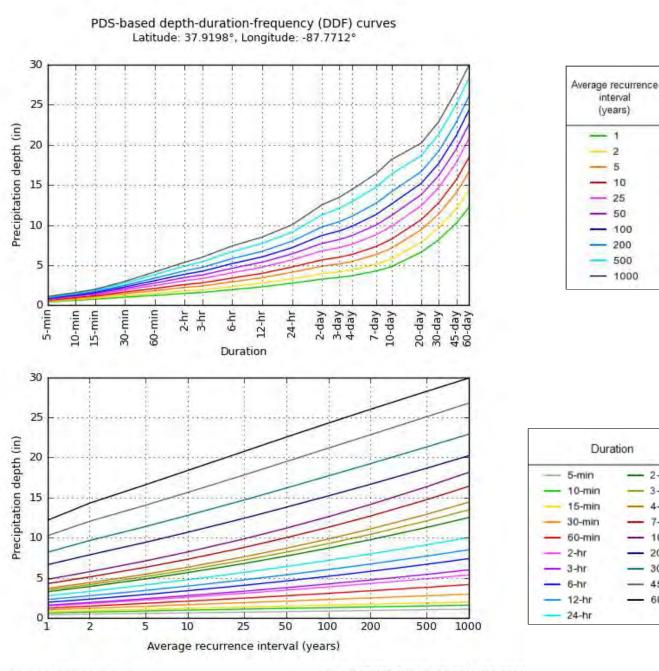
¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

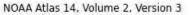
Numbers in parenthesis are PF estimates at low er and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the low er bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

Please refer to NOAA Atlas 14 document for more information.

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PF graphical





Created (GMT): Tue Jun 23 20:44:11 2020

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Maps & aerials

Small scale terrain

interval

(years)

1

2 5 10

25 50

100 200 500

- 1000

Duration

2-day

3-day

4-day

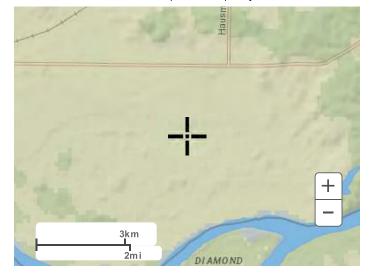
7-day

10-day

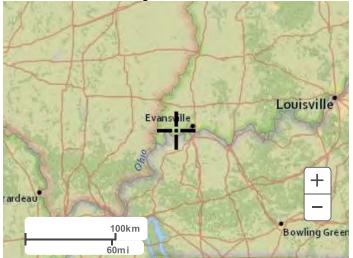
20-day 30-day

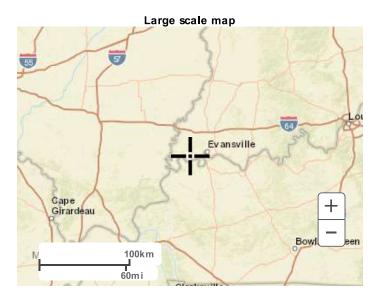
45-day

60-day



Large scale terrain





Large scale aerial

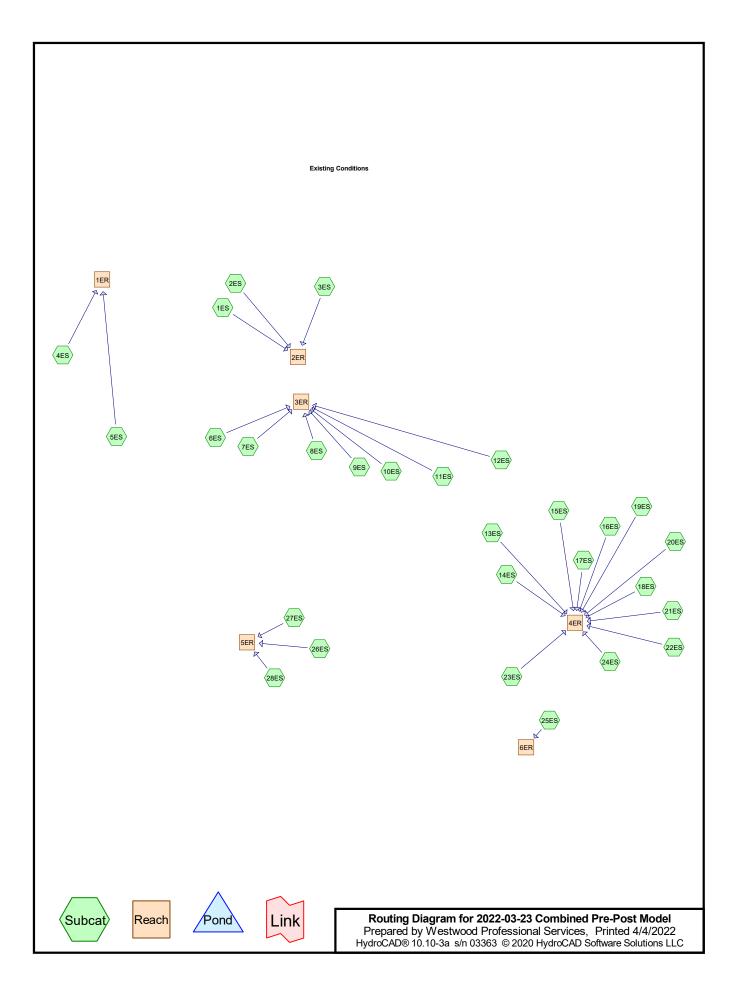


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US Department of Commerce National Oceanic and Atmospheric Administration National Weather Service National Water Center 1325 East West Highway Silver Spring, MD 20910 Questions?: <u>HDSC.Questions@noaa.gov</u>

Disclaimer

Appendix B Existing HydroCAD Results



Prepared by Westwood Professional Services HydroCAD® 10.10-3a s/n 03363 © 2020 HydroCAD Software Solutions LLC

Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
1	1-yr	IN-Posey 24-hr S1	1-yr	Default	24.00	1	2.74	2
2	2-yr	IN-Posey 24-hr S1	2-yr	Default	24.00	1	3.30	2
3	5-yr	IN-Posey 24-hr S1	5-yr	Default	24.00	1	4.10	2
4	10-yr	IN-Posey 24-hr S1	10-yr	Default	24.00	1	4.75	2
5	25-yr	IN-Posey 24-hr S1	25-yr	Default	24.00	1	5.66	2
6	50-yr	IN-Posey 24-hr S1	50-yr	Default	24.00	1	6.40	2
7	100-yr	IN-Posey 24-hr S1	100-yr	Default	24.00	1	7.17	2

Rainfall Events Listing

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Area Listing (selected nodes)

Area	CN	Description
(acres)		(subcatchment-numbers)
155.900	78	Row crops, straight row, Good, HSG B (3ES, 14ES, 15ES, 16ES, 17ES, 20ES, 21ES, 23ES, 24ES, 25ES, 27ES, 28ES)
64.300	85	Row crops, straight row, Good, HSG C (3ES, 7ES, 8ES, 10ES, 11ES, 13ES, 17ES)
1,195.880	89	Row crops, straight row, Good, HSG D (1ES, 2ES, 3ES, 4ES, 5ES, 6ES, 7ES, 8ES, 9ES, 10ES, 11ES, 12ES, 13ES, 14ES, 15ES, 16ES, 17ES, 18ES, 19ES, 20ES, 21ES, 22ES, 23ES, 24ES, 25ES, 26ES, 27ES, 28ES)
2.060	55	Woods, Good, HSG B (15ES)
17.030	77	Woods, Good, HSG D (24ES)
1,435.170	87	TOTAL AREA

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Soil Listing (selected nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
0.000	HSG A	
157.960	HSG B	3ES, 14ES, 15ES, 16ES, 17ES, 20ES, 21ES, 23ES, 24ES, 25ES, 27ES, 28ES
64.300	HSG C	3ES, 7ES, 8ES, 10ES, 11ES, 13ES, 17ES
1,212.910	HSG D	1ES, 2ES, 3ES, 4ES, 5ES, 6ES, 7ES, 8ES, 9ES, 10ES, 11ES, 12ES, 13ES, 14ES, 15ES, 16ES, 17ES, 18ES, 19ES, 20ES, 21ES, 22ES, 23ES, 24ES, 25ES, 26ES, 27ES, 28ES
0.000	Other	
1,435.170		TOTAL AREA

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HSG-A	HSG-B	HSG-C	HSG-D	Other	Total	Ground	Subcatchment
(acres)	(acres)	(acres)	(acres)	(acres)	(acres)	Cover	Numbers
 0.000	155.900	64.300	1,195.880	0.000	1,416.080	Row crops, straight row, Good	
			.,		.,	·····, ····, ····	2ES,
							3ES,
							4ES,
							5ES,
							6ES,
							7ES,
							8ES,
							9ES,
							10ES,
							11ES,
							12ES,
							13ES,
							14ES,
							15ES,
							16ES,
							17ES,
							18ES,
							19ES,
							20ES,
							21ES,
							22ES,
							23ES,
							24ES,
							25ES,
							26ES,
							27ES,
0.000	0.000	0.000	47.000	0.000	10.000		28ES
0.000	2.060	0.000	17.030	0.000	19.090	Woods, Good	15ES,
			4 040 040				24ES
0.000	157.960	64.300	1,212.910	0.000	1,435.170	TOTAL AREA	

Ground Covers (selected nodes)

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Summary for Subcatchment 1ES:

Runoff = 19.19 cfs @ 12.65 hrs, Volume= 2.901 af, Depth= 2.17"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 2-yr Rainfall=3.30"

Area (ac) CN Description										
16.020 89 Row crops, straight row, Good, HSG D										
16.020 100.00% Pervious Area										
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)										
51.0 1,260 0.0030 0.41 Lag/CN Method,										
Summary for Subcatchment 2ES:										
Runoff = 33.30 cfs @ 12.61 hrs, Volume= 4.875 af, Depth= 2.17"										
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 2-yr Rainfall=3.30"										
Area (ac) CN Description										
26.920 89 Row crops, straight row, Good, HSG D										
26.920 100.00% Pervious Area										
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)										
47.6 1,155 0.0030 0.40 Lag/CN Method,										

Summary for Subcatchment 3ES:

Runoff = 142.41 cfs @ 12.48 hrs, Volume= 18.201 af, Depth= 2.00"

Area ((ac)	CN	Desc	ription			
17.	940	78	Row	crops, stra	aight row, C	Good, HSG B	
15.	860	85	Row	crops, stra	aight row, C	Good, HSG C	
75.2	210	89	Row	crops, stra	aight row, C	Good, HSG D	
109.	010	87	Weig	hted Aver	age		
109.	010		100.0	00% Pervi	ous Area		
-					0		
Tc	Lengt		Slope	Velocity	Capacity	Description	
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)		
37.9	2,502	20.	.0190	1.10		Lag/CN Method,	

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Summary for Subcatchment 4ES:

Runoff = 76.24 cfs @ 13.97 hrs, Volume= 25.296 af, Depth= 2.17"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 2-yr Rainfall=3.30"

	Area (ac) CN Description								
139.690 89 Row crops, straight row, Good, HSG D									
139.690 100.00% Pervious Area									
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
(min)			, ,	(015)					
158.9	2,625	0.0010	0.28		Lag/CN Method,				
Summary for Subcatchment 5ES:									
Runoff	=	23.01 cfs	s@ 13.74	4 hrs, Volu	ume= 6.637 af, Depth= 2.17"				

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 2-yr Rainfall=3.30"

Area	(ac) C	N Dese	cription								
36.	650 8	9 Row	ow crops, straight row, Good, HSG D								
36.	36.650 100.00% Pervious Area										
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description						
133.2	2,106	0.0010	0.26		Lag/CN Method,						

Summary for Subcatchment 6ES:

Runoff = 42.45 cfs @ 12.60 hrs, Volume= 6.184 af, Depth= 2.17"

Area	(ac)	CN	Desc	ription							
34	.150	89	Row	ow crops, straight row, Good, HSG D							
34	34.150 100.00% Pervious Area										
Tc (min)	Lengtl (feet		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
47.5	1,380	0.0	0040	0.48		Lag/CN Method,					

	-	ombined Pre	IN-Posey 24-hr S1 2-yr Rainfall=3.30"							
Prepare	Prepared by Westwood Professional Services Printed									
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	Summary for Subcatchment 7ES:									
Runoff	=	23.20 cfs @	12.39 hrs, Volume=	2.710 af, Depth= 2.09"						
	Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 2-yr Rainfall=3.30"									
Aroo	(aa)	CN Decorinti	on							

_	Area	(ac) C	<u>CN De</u>	escription					
	2.110 85 Row crops, straight row, Good, HSG C								
	13.	470	89 Ro	ow crops, str	aight row, (Good, HSG D			
	15.580 88 Weighted Average								
	15.580 100.00% Pervious Area								
	Tc	Length	Slop	e Velocity	Capacity	Description			
_	(min)	(feet)	(ft/f	t) (ft/sec)	(cfs)				
	32.2	675	0.003	0 0.35		Lag/CN Method,			
						-			

Summary for Subcatchment 8ES:

56.82 cfs @ 12.62 hrs, Volume= 8.458 af, Depth= 2.09" Runoff =

Area (ac) CN Description							
14.510 85 Row crops, straight row, Good, HSG C							
34.120 89 Row crops, straight row, Good, HSG D							
48.630 88 Weighted Average							
48.630 100.00% Pervious Area							
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)							
49.5 1,783 0.0060 0.60 Lag/CN Method,							
Summary for Subcatchment 9ES:							
Runoff = 51.01 cfs @ 13.26 hrs, Volume= 12.030 af, Depth= 2.17"							
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 2-yr Rainfall=3.30"							
Area (ac) CN Description							
66.430 89 Row crops, straight row, Good, HSG D							
66.430 100.00% Pervious Area							
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)							

100.1	2,271	0.0020	0.38	Lag/CN Method,
100.1	2,211	0.0020	0.00	Lug/on monou,

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Summary for Subcatchment 10ES:	
Runoff = 52.95 cfs @ 12.98 hrs, Volume= 10.336 af, Depth= 2.09"	
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0 N-Posey 24-hr S1 2-yr Rainfall=3.30"	0.05 hrs
Area (ac) CN Description	
10.900 85 Row crops, straight row, Good, HSG C 48.530 89 Row crops, straight row, Good, HSG D	
59.430 88 Weighted Average 59.430 100.00% Pervious Area	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
75.8 1,968 0.0030 0.43 Lag/CN Method,	
Summary for Subcatchment 11ES:	
Summary for Subcatchment TIES.	
Runoff = 59.84 cfs @ 13.04 hrs, Volume= 12.084 af, Depth= 2.17"	
	05 has
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0 N-Posey 24-hr S1 2-yr Rainfall=3.30"	0.05 hrs
Area (ac) CN Description	
5.740 85 Row crops, straight row, Good, HSG C 60.990 89 Row crops, straight row, Good, HSG D	
66.730 89 Weighted Average	
66.730 100.00% Pervious Area	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
80.1 1,720 0.0020 0.36 Lag/CN Method,	
Summary for Subcatchment 12ES:	
-	
-	
۔ Runoff = 77.88 cfs @ 13.65 hrs, Volume= 22.026 af, Depth= 2.17" Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0	0.05 hrs
۔ Runoff = 77.88 cfs @ 13.65 hrs, Volume= 22.026 af, Depth= 2.17" Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0	0.05 hrs
Runoff = 77.88 cfs @ 13.65 hrs, Volume= 22.026 af, Depth= 2.17" Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0 N-Posey 24-hr S1 2-yr Rainfall=3.30"	0.05 hrs
Runoff = 77.88 cfs @ 13.65 hrs, Volume= 22.026 af, Depth= 2.17" Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0 N-Posey 24-hr S1 2-yr Rainfall=3.30" <u>Area (ac) CN Description</u>	1.05 hrs

IN-Posey 24-hr S1 2-yr Rainfall=3.30"

Tc Length Slope Velocity Capacity Description (feet) (min) (ft/ft) (ft/sec) (cfs) 129.2 3,125 0.0020 0.40 Lag/CN Method,

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Summary for Subcatchment 13ES:	
Runoff = 45.56 cfs @ 12.67 hrs, Volume= 7.074 af, Depth= 2.09"	
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 2-yr Rainfall=3.30"	6
Area (ac) CN Description	
12.570 85 Row crops, straight row, Good, HSG C 28.100 89 Row crops, straight row, Good, HSG D	
40.670 88 Weighted Average	
40.670 100.00% Pervious Area	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
52.8 1,500 0.0040 0.47 Lag/CN Method,	
Summary for Subcatchment 14ES:	
Runoff = 64.17 cfs @ 12.41 hrs, Volume= 7.675 af, Depth= 2.09"	
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 2-yr Rainfall=3.30"	3
Area (ac) CN Description 2.920 78 Row crops, straight row, Good, HSG B	
41.210 89 Row crops, straight row, Good, HSG D	
44.13088Weighted Average44.130100.00% Pervious Area	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
33.6 852 0.0040 0.42 Lag/CN Method,	
Summary for Subcatchment 15ES:	
Runoff = 68.50 cfs @ 12.32 hrs, Volume= 7.201 af, Depth= 1.55"	
Runoff = 68.50 cfs @ 12.32 hrs, Volume= 7.201 af, Depth= 1.55" Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 2-yr Rainfall=3.30"	3
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 2-yr Rainfall=3.30" <u>Area (ac) CN Description</u>	5
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 2-yr Rainfall=3.30" <u>Area (ac) CN Description</u> 34.820 78 Row crops, straight row, Good, HSG B	5
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 2-yr Rainfall=3.30" <u>Area (ac) CN Description</u>	3
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 2-yr Rainfall=3.30" <u>Area (ac) CN Description</u> 34.820 78 Row crops, straight row, Good, HSG B 18.940 89 Row crops, straight row, Good, HSG D	5

IN-Posey 24-hr S1 2-yr Rainfall=3.30"

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Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
25.9 1,535 0.0280 0.99 Lag/CN Method,	
Summary for Subcatchment 16ES:	
Runoff = 43.90 cfs @ 12.38 hrs, Volume= 5.005 af, Depth= 1.48"	
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= IN-Posey 24-hr S1 2-yr Rainfall=3.30"	0.05 hrs
Area (ac) CN Description	
34.110 78 Row crops, straight row, Good, HSG B	
6.490 89 Row crops, straight row, Good, HSG D	
40.60080Weighted Average40.600100.00% Pervious Area	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
29.9 1,600 0.0240 0.89 Lag/CN Method,	
Summary for Subcatchment 17ES:	
Runoff = 41.54 cfs @ 12.30 hrs, Volume= 4.235 af, Depth= 1.62"	
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= IN-Posey 24-hr S1 2-yr Rainfall=3.30"	0.05 hrs
Area (ac) CN Description	
19.050 78 Row crops, straight row, Good, HSG B	
2.610 85 Row crops, straight row, Good, HSG C	
9.730 89 Row crops, straight row, Good, HSG D	
31.39082Weighted Average31.390100.00% Pervious Area	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
24.5 1,390 0.0250 0.94 Lag/CN Method,	

IN-Posey 24-hr S1 2-yr Rainfall=3.30"

Summary for Subcatchment 18ES:

Runoff = 13.61 cfs @ 12.26 hrs, Volume= 1.320 af, Depth= 2.17"

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IN-Posey 24-hr S1 2-yr Rainfall=3.30" Printed 4/4/2022 Page 12

Area (ac) CN Description								
7.290 89 Row crops, straight row, Good, HSG D								
7.290 100.00% Pervious Area								
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)								
22.8 850 0.0080 0.62 Lag/CN Method,								
Summary for Subcatchment 19ES:								
Runoff = 20.19 cfs @ 12.39 hrs, Volume= 2.347 af, Depth= 2.17"								
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 2-yr Rainfall=3.30" Area (ac) CN Description								
12.960 89 Row crops, straight row, Good, HSG D								
12.960 100.00% Pervious Area								
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)								
31.8 1,185 0.0070 0.62 Lag/CN Method,								
Summary for Subcatchment 20ES:								
Runoff = 55.00 cfs @ 12.28 hrs, Volume= 5.461 af, Depth= 2.09"								
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 2-yr Rainfall=3.30"								
Area (ac) CN Description								
2.950 78 Row crops, straight row, Good, HSG B 28.450 89 Row crops, straight row, Good, HSG D								
31.40088Weighted Average31.400100.00% Pervious Area								
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)								
23.9 789 0.0070 0.55 Lag/CN Method,								
Summary for Subcatchment 21ES:								
Runoff = 16.60 cfs @ 12.12 hrs, Volume= 1.202 af, Depth= 1.55"								
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 2-yr Rainfall=3.30"								

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IN-Posey 24-hr S1 2-yr Rainfall=3.30" Printed 4/4/2022 HydroCAD® 10.10-3a s/n 03363 © 2020 HydroCAD Software Solutions LLC Page 13

Area (ac) CN Description								
7.130 78 Row crops, straight row, Good, HSG B 2.190 89 Row crops, straight row, Good, HSG D								
9.320 81 Weighted Average								
9.320 100.00% Pervious Area								
Tc Length Slope Velocity Capacity Description								
(min) (feet) (ft/ft) (ft/sec) (cfs) 12.1 690 0.0360 0.95 Lag/CN Method,								
12.1 690 0.0360 0.95 Lag/CN Method ,								
Summary for Subcatchment 22ES:								
Runoff = 35.62 cfs @ 12.16 hrs, Volume= 2.830 af, Depth= 2.17"								
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 2-yr Rainfall=3.30"								
Area (ac) CN Description								
15.630 89 Row crops, straight row, Good, HSG D								
15.630 100.00% Pervious Area								
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)								
15.2 790 0.0160 0.87 Lag/CN Method,								
Summary for Subcatchment 23ES:								
Runoff = 172.13 cfs @ 12.28 hrs, Volume= 17.131 af, Depth= 2.00"								
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 2-yr Rainfall=3.30"								
Area (ac) CN Description								
15.700 78 Row crops, straight row, Good, HSG B								
86.900 89 Row crops, straight row, Good, HSG D								
102.60087Weighted Average102.600100.00% Pervious Area								
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)								
24.0 615 0.0050 0.43 Lag/CN Method,								
Summary for Subcatchment 24ES:								
Runoff = 83.57 cfs @ 13.79 hrs, Volume= 24.524 af, Depth= 2.00"								

IN-Posey 24-hr S1 2-yr Rainfall=3.30" Printed 4/4/2022 HydroCAD® 10.10-3a s/n 03363 © 2020 HydroCAD Software Solutions LLC Page 14

Area ((ac)	CN	Desc	ription		
7.	500	78	Row	crops, stra	aight row, C	Good, HSG B
122.3	350	89	Row	crops, stra	aight row, C	Good, HSG D
17.	030	77	Woo	ds, Good,	HSG D	
146.	880	87	Weig	ghted Aver	age	
146.880 100.00% Pervious Area						
Tc	Lengt		Slope	Velocity	Capacity	Description
<u>(min)</u>	(fee	t)	(ft/ft)	(ft/sec)	(cfs)	
136.5	3,04	2 (0.0020	0.37		Lag/CN Method,
						-

Summary for Subcatchment 25ES:

Runoff 48.20 cfs @ 12.64 hrs, Volume= 7.310 af, Depth= 2.09" =

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 2-yr Rainfall=3.30"

Area	(ac)	CN	Desc	cription		
5	.340	78	Row	crops, str	aight row,	Good, HSG B
36	.690	89	Row	crops, str	aight row,	Good, HSG D
42	.030	88	Weig	ghted Aver	age	
42	.030		100.	00% Pervi	ous Area	
Tc	Lengt	th	Slope	Velocity	Capacity	1
(min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)	
50.9	93	80 0	.0020	0.30		Lag/CN Method,
						.

Summary for Subcatchment 26ES:

Runoff 66.51 cfs @ 12.65 hrs, Volume= 10.056 af, Depth= 2.17" =

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 2-yr Rainfall=3.30"

Area ((ac) C	N Dese	cription					
55.	55.530 89 Row crops, straight row, Good, HSG D							
55.	55.530 100.00% Pervious Area							
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
51.0	1,260	0.0030	0.41		Lag/CN Method,			
	Summany for Subcatchmont 27ES.							

Summary for Subcatchment 27ES:

Runoff 26.32 cfs @ 13.06 hrs, Volume= 5.463 af, Depth= 2.00" =

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 2-yr Rainfall=3.30"

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IN-Posey 24-hr S1 2-yr Rainfall=3.30" Printed 4/4/2022 LC Page 15

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_	Area	(ac)	CN	Desc	cription			
	4.	470	78	Row	crops, stra	aight row, (Good, HSG B	
_	28.	250	89	Row	crops, stra	aight row, (Good, HSG D	
	32.	720	87	Weig	ghted Aver	age		
	32.	720		100.0	00% Pervi	ous Area		
	_			~ .		•		
	TC	Lengt		Slope	Velocity	Capacity	Description	
_	(min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)		
	83.1	1,63	5 0	0.0020	0.33		Lag/CN Method,	
							-	
					-	_		

Summary for Subcatchment 28ES:

Runoff = 31.18 cfs @ 12.54 hrs, Volume= 4.229 af, Depth= 2.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 2-yr Rainfall=3.30"

Area	(ac)	CN	Desc	cription		
3	.970	78	8 Row	crops, stra	aight row, (Good, HSG B
21	.360	89	Row	crops, stra	aight row, (Good, HSG D
25	.330	87	′ Weig	ghted Aver	age	
25	.330		100.	00% Pervi	ous Area	
Tc (min)	Leng (fee		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
42.0	1,07	75	0.0040	0.43		Lag/CN Method,

Summary for Reach 1ER:

Inflow Are	a =	176.340 ac,	0.00% Impervious, Inflow	Depth = 2.17" for 2-yr event
Inflow	=	98.41 cfs @	13.94 hrs, Volume=	31.933 af
Outflow	=	98.41 cfs @	13.94 hrs, Volume=	31.933 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs

Summary for Reach 2ER:

Inflow Are	a =	151.950 ac,	0.00% Impervious, Inflov	v Depth = 2.05"	for 2-yr event
Inflow	=	191.76 cfs @	12.51 hrs, Volume=	25.977 af	
Outflow	=	191.76 cfs @	12.51 hrs, Volume=	25.977 af, Atte	en= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs

Summary for Reach 3ER:

Inflow Are	ea =	412.580 ac,	0.00% Impervious, Int	flow Depth = 2.15"	for 2-yr event
Inflow	=	289.79 cfs @	12.92 hrs, Volume=	73.827 af	
Outflow	=	289.79 cfs @	12.92 hrs, Volume=	73.827 af, Atte	en= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs

Summary for Reach 4ER:

Inflow Are	a =	538.690 ac,	0.00% Impervious, Inflow	Depth = 1.92"	for 2-yr event
Inflow	=	541.89 cfs @	12.31 hrs, Volume=	86.005 af	
Outflow	=	541.89 cfs @	12.31 hrs, Volume=	86.005 af, Atte	en= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs

Summary for Reach 5ER:

Inflow Are	a =	113.580 ac,	0.00% Impervious, In	flow Depth = 2.09"	for 2-yr event
Inflow	=	115.43 cfs @	12.66 hrs, Volume=	19.748 af	
Outflow	=	115.43 cfs @	12.66 hrs, Volume=	19.748 af, Atte	en= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs

Summary for Reach 6ER:

Inflow Area	a =	42.030 ac,	0.00% Impervious,	Inflow Depth = 2.09	for 2-yr event
Inflow	=	48.20 cfs @	12.64 hrs, Volume	= 7.310 af	
Outflow	=	48.20 cfs @	12.64 hrs, Volume	= 7.310 af, At	tten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs

Summary for Subcatchment 1ES:

Runoff = 30.32 cfs @ 12.65 hrs, Volume= 4.716 af, Depth> 3.53"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 10-yr Rainfall=4.75"

Area (ac	c) CN	N Desc	cription							
16.020 89 Row crops, straight row, Good, HSG D										
16.02	16.020 100.00% Pervious Area									
	ength (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
51.0	1,260	0.0030	0.41		Lag/CN Method,					
	Summary for Subcatchment 2ES:									
Runoff	=	52.55 cfs	s@ 12.6) hrs, Volu	me= 7.925 af, Depth> 3.53"					
Runoff by S IN-Posey 24	24-hr S1	10-yr R	ainfall=4.7		nted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs					
Area (ac	c) CN		cription							
26.92	<u>20 89</u>	9 Row	crops, str	aight row, (Good, HSG D					
26.92	20	100.0	00% Pervi	ous Area						
	ength (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
47.6	1,155	0.0030	0.40		Lag/CN Method,					
	Summary for Subcatchment 3ES:									

Runoff = 229.91 cfs @ 12.47 hrs, Volume= 30.272 af, Depth= 3.33"

Area	(ac)	CN	Desc	cription			
17.	940	78	Row	crops, stra	aight row, C	Good, HSG B	
15.	860	85	Row	crops, stra	aight row, C	Good, HSG C	
75.	210	89	Row	crops, stra	aight row, C	Good, HSG D	
109.	010	87	Weig	ghted Aver	age		
109.	010		100.0	00% Pervi	ous Area		
Tc	Length	۱. ۲	Slope	Velocitv	Capacity	Description	
(min)	(feet		(ft/ft)	(ft/sec)	(cfs)	Decemption	
37.9	2,502	2 0.	0190	1.10		Lag/CN Method,	

Summary for Subcatchment 4ES:

Runoff = 122.79 cfs @ 13.96 hrs, Volume= 41.127 af, Depth= 3.53"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 10-yr Rainfall=4.75"

_	Area	(ac) C	N Desc	cription							
	139.	690 8	9 Row	crops, str	aight row, (Good, HSG D					
	139.	690	100.	00% Pervi	ous Area						
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
	158.9 2,625 0.0010 0.28 Lag/CN Method,										
	Summary for Subcatchment 5ES:										

Runoff	=	36.89 cfs @	13.74 hrs,	Volume=	10.790 af, Depth> 3.53"
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Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 10-yr Rainfall=4.75"

Area	(ac) (CN De	scription		
36.	650	89 Ro	w crops, str	aight row, (Good, HSG D
36.	650	10	0.00% Pervi	ous Area	
Tc (min)	Length (feet)	Slope (ft/ft	,	Capacity (cfs)	Description
133.2	2,106	0.001	0.26		Lag/CN Method,

Summary for Subcatchment 6ES:

Runoff = 66.98 cfs @ 12.60 hrs, Volume= 10.054 af, Depth> 3.53"

 Area	(ac) (CN	Desc	ription		
34.	150	89	Row	crops, stra	aight row, (Good, HSG D
34.	150		100.0	00% Pervi	ous Area	
Tc (min)	Length (feet)		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
 47.5	1,380	0.0	0040	0.48		Lag/CN Method,

Summary for Subcatchment 7ES:

Runoff = 36.79 cfs @ 12.39 hrs, Volume= 4.456 af, Depth> 3.43"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 10-yr Rainfall=4.75"

_	Area	(ac)	CN	Desc	cription		
	2.	110	85	Row	crops, stra	aight row, (Good, HSG C
	13.	470	89	Row	crops, stra	aight row, (Good, HSG D
	15.	580	88	Weig	ghted Aver	age	
	15.	580		100.	00% Pervi	ous Area	
	_						
	Тс	Lengt	h	Slope	Velocity	Capacity	Description
_	(min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)	
	32.2	67	5 0	0.0030	0.35		Lag/CN Method,
							-

Summary for Subcatchment 8ES:

Runoff = 91.02 cfs @ 12.62 hrs, Volume= 13.908 af, Depth> 3.43"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 10-yr Rainfall=4.75"

Area (ac) CN Description							
14.510 85 Row crops, straight row, Good, HSG C							
34.120 89 Row crops, straight row, Good, HSG D							
48.630 88 Weighted Average							
48.630 100.00% Pervious Area							
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)							
49.5 1,783 0.0060 0.60 Lag/CN Method,							
Summary for Subcatchment 9ES:							
Runoff = 81.65 cfs @ 13.25 hrs, Volume= 19.558 af, Depth> 3.53"							
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 10-yr Rainfall=4.75"							
Area (ac) CN Description							
66.430 89 Row crops, straight row, Good, HSG D							
66.430 100.00% Pervious Area							
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)							

Lag/CN Method,

(min)	(feet)	(ft/ft)	(ft/sec)	(
100.1	2,271	0.0020	0.38	

2022-03-23 Combined Pre-Post Model	IN-Posey 24-hr S1 10-yr Rainfall=4.75"
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Summary for Subcatchment 10ES:

Runoff 85.54 cfs @ 12.96 hrs, Volume= 16.997 af, Depth= 3.43" =

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 10-yr Rainfall=4.75"

_	Area	(ac)	CN	Desc	cription		
	10.	900	85	Row	crops, stra	aight row, 0	Good, HSG C
	48.	530	89	Row	crops, stra	aight row, C	Good, HSG D
	59.	430	88	Weig	ghted Aver	age	
	59.	430		100.0	00% Pervi	ous Area	
	Тс	Lengt		Slope	Velocity	Capacity	Description
_	(min)	(feet	t)	(ft/ft)	(ft/sec)	(cfs)	
	75.8	1,96	8 0	.0030	0.43		Lag/CN Method,
		-					

Summary for Subcatchment 11ES:

Runoff 95.35 cfs @ 13.03 hrs, Volume= 19.646 af, Depth> 3.53" =

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 10-yr Rainfall=4.75"

Area (ac) CN Description					
5.740 85 Row crops, straight row, Good, HSG C					
60.990 89 Row crops, straight row, Good, HSG D					
66.730 89 Weighted Average					
66.730 100.00% Pervious Area					
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)					
80.1 1,720 0.0020 0.36 Lag/CN Method,					
Summary for Subcatchment 12ES:					
Runoff = 124.98 cfs @ 13.64 hrs, Volume= 35.810 af, Depth> 3.53"					
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 10-yr Rainfall=4.75"					
Area (ac) CN Description					
121.630 89 Row crops, straight row, Good, HSG D					
121.630 100.00% Pervious Area					
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)					

(min) (feet) (ft/ft) (ft/sec)

129.2	3,125	0.0020	0.40	Lag/CN Method,
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2022-03-23 Combined Pre-Post Model	IN-Posey 24-hr S1 10-yr Rainfall=4.75"
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Summary for Subcatchment 13ES:

73.08 cfs @ 12.67 hrs, Volume= Runoff 11.632 af, Depth> 3.43" =

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 10-yr Rainfall=4.75"

 Area	(ac)	CN	Desc	cription		
12.	570	85	Row	crops, stra	aight row, 0	Good, HSG C
 28.	100	89	Row	crops, stra	aight row, C	Good, HSG D
40.	670	88	Weig	ghted Aver	age	
40.	670		100.0	00% Pervi	ous Area	
_						
Tc	Lengtl	n S	Slope	Velocity	Capacity	Description
(min)	(feet	:)	(ft/ft)	(ft/sec)	(cfs)	
 52.8	1,50	0 0	.0040	0.47		Lag/CN Method,
	,					

Summary for Subcatchment 14ES:

Runoff 101.84 cfs @ 12.41 hrs, Volume= 12.621 af, Depth> 3.43" =

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 10-yr Rainfall=4.75"

Area (ac) C	N Des	cription					
2.920 78 Row crops, straight row, Good, HSG B								
41.2	210 8	39 Row	crops, str	aight row, (Good, HSG D			
44.1	130 8	38 Wei	ghted Aver	age				
44.1	44.130 100.00% Pervious Area							
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
33.6	852	0.0040	0.42		Lag/CN Method,			
Summary for Subcatchment 15ES:								

Runoff 120.10 cfs @ 12.32 hrs, Volume= 12.864 af, Depth= 2.77" =

 Area (ac)	CN	Description
34.820	78	Row crops, straight row, Good, HSG B
18.940	89	Row crops, straight row, Good, HSG D
 2.060	55	Woods, Good, HSG B
 55.820	81	Weighted Average
55.820		100.00% Pervious Area

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Tc Length Slope Velocity Capacity Description					
(min) (feet) (ft/ft) (ft/sec) (cfs)					
25.9 1,535 0.0280 0.99 Lag/CN Method,					
Summary for Subcatchment 16ES:					
Runoff = 78.59 cfs @ 12.37 hrs, Volume= 9.054 af, Depth= 2.68"					
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0 IN-Posey 24-hr S1 10-yr Rainfall=4.75").05 hrs				
Area (ac) CN Description					
34.110 78 Row crops, straight row, Good, HSG B					
6.490 89 Row crops, straight row, Good, HSG D					
40.60080Weighted Average40.600100.00% Pervious Area					
Tc Length Slope Velocity Capacity Description _(min) (feet) (ft/ft) (ft/sec) (cfs)					
29.9 1,600 0.0240 0.89 Lag/CN Method,					
Summary for Subcatchment 17ES:					
Runoff = 71.56 cfs @ 12.29 hrs, Volume= 7.472 af, Depth= 2.86"					
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 10-yr Rainfall=4.75"					
Area (ac) CN Description					
19.050 78 Row crops, straight row, Good, HSG B					
2.610 85 Row crops, straight row, Good, HSG C					
9.730 89 Row crops, straight row, Good, HSG D					
31.390 82 Weighted Average 31.390 100.00% Pervious Area					
Tc Length Slope Velocity Capacity Description					
(min) (feet) (ft/ft) (ft/sec) (cfs)					
24.5 1,390 0.0250 0.94 Lag/CN Method,					

IN-Posey 24-hr S1 10-yr Rainfall=4.75"

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2022-03-23 Combined Pre-Post Model

Summary for Subcatchment 18ES:

Runoff = 21.09 cfs @ 12.26 hrs, Volume= 2.146 af, Depth> 3.53"

IN-Posey 24-hr S1 10-yr Rainfall=4.75" Printed 4/4/2022 LLC Page 23

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Area (ac) CN Description					
7.290 89 Row crops, straight row, Good, HSG D					
7.290 100.00% Pervious Area					
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)					
22.8 850 0.0080 0.62 Lag/CN Method,					
~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~					
Summary for Subcatchment 19ES:					
Runoff = 31.56 cfs @ 12.38 hrs, Volume= 3.815 af, Depth> 3.53"					
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 10-yr Rainfall=4.75"					
Area (ac) CN Description					
12.960 89 Row crops, straight row, Good, HSG D					
12.960 100.00% Pervious Area					
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)					
31.8 1,185 0.0070 0.62 Lag/CN Method,					
Summary for Subcatchment 20ES:					
Summary for Subcatchinent 2023.					
Runoff = 86.54 cfs @ 12.28 hrs, Volume= 8.980 af, Depth> 3.43"					
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 10-yr Rainfall=4.75"					
Area (ac) CN Description					
2.950 78 Row crops, straight row, Good, HSG B					
28.450 89 Row crops, straight row, Good, HSG D					
31.400 88 Weighted Average 31.400 100.00% Pervious Area					
Tc Length Slope Velocity Capacity Description					
(min) (feet) (ft/ft) (ft/sec) (cfs)					
23.9 789 0.0070 0.55 Lag/CN Method,					

Summary for Subcatchment 21ES:

Runoff = 28.61 cfs @ 12.12 hrs, Volume= 2.148 af, Depth= 2.77"

IN-Posey 24-hr S1 10-yr Rainfall=4.75" Printed 4/4/2022 LLC Page 24

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Area (ac) CN Description
7.130 78 Row crops, straight row, Good, HSG B 2.190 89 Row crops, straight row, Good, HSG D
9.320 81 Weighted Average
9.320 100.00% Pervious Area
Tc Length Slope Velocity Capacity Description
(min) (feet) (ft/ft) (ft/sec) (cfs) 12.1 690 0.0360 0.95 Lag/CN Method,
12.1 690 0.0360 0.95 Lag/CN Method,
Summary for Subcatchment 22ES:
Runoff = 54.63 cfs @ 12.16 hrs, Volume= 4.600 af, Depth> 3.53"
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 10-yr Rainfall=4.75"
Area (ac) CN Description
15.630 89 Row crops, straight row, Good, HSG D
15.630 100.00% Pervious Area
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
15.2 790 0.0160 0.87 Lag/CN Method ,
Summary for Subcatchment 23ES:
Runoff = 274.77 cfs @ 12.28 hrs, Volume= 28.492 af, Depth= 3.33"
Runoff = 274.77 cfs @ 12.28 hrs, Volume= 28.492 af, Depth= 3.33" Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 10-yr Rainfall=4.75"
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 10-yr Rainfall=4.75" Area (ac) CN Description 15.700 78 Row crops, straight row, Good, HSG B
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 10-yr Rainfall=4.75" Area (ac) CN Description 15.700 78 Row crops, straight row, Good, HSG B 86.900 89 Row crops, straight row, Good, HSG D
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 10-yr Rainfall=4.75" Area (ac) CN Description 15.700 78 Row crops, straight row, Good, HSG B
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 10-yr Rainfall=4.75" Area (ac) CN Description 15.700 78 Row crops, straight row, Good, HSG B 86.900 89 Row crops, straight row, Good, HSG D 102.600 87 Weighted Average

Summary for Subcatchment 24ES:

Runoff = 137.77 cfs @ 13.78 hrs, Volume= 40.789 af, Depth= 3.33"

IN-Posey 24-hr S1 10-yr Rainfall=4.75" Printed 4/4/2022 LLC Page 25

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Area	(ac)	CN	Desc	cription		
7.	500	78	Row	crops, stra	aight row, C	Good, HSG B
122.	350	89	Row	crops, stra	aight row, C	Good, HSG D
17.	030	77	Woo	ds, Good,	HSG D	
146.	880	87	Weig	ghted Aver	age	
146.	880		100.0	00% Pervi	ous Area	
т	1 4				0	Description
Tc	Lengt		Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
136.5	3,042	2 0.	.0020	0.37		Lag/CN Method,

Summary for Subcatchment 25ES:

Runoff = 77.25 cfs @ 12.64 hrs, Volume= 12.021 af, Depth> 3.43"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 10-yr Rainfall=4.75"

Area	(ac)	CN	Desc	cription		
5	.340	78	Row	crops, str	aight row,	Good, HSG B
36	.690	89	Row	crops, str	aight row,	Good, HSG D
42	.030	88	Weig	ghted Aver	age	
42	.030		100.	00% Pervi	ous Area	
Tc	Lengt	th	Slope	Velocity	Capacity	1
(min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)	
50.9	93	80 0	.0020	0.30		Lag/CN Method,
						.

Summary for Subcatchment 26ES:

Runoff = 105.08 cfs @ 12.65 hrs, Volume= 16.348 af, Depth> 3.53"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 10-yr Rainfall=4.75"

 Area	(ac) (N Des	cription			
55.530 89 Row crops, straight row, Good, HSG D						
55.530 100.00% Pervious Area						
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
 51.0	1,260		0.41	(0.0)	Lag/CN Method,	

Summary for Subcatchment 27ES:

Runoff = 43.20 cfs @ 13.04 hrs, Volume= 9.086 af, Depth= 3.33"

IN-Posey 24-hr S1 10-yr Rainfall=4.75" Printed 4/4/2022 LLC Page 26

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_	Area	(ac)	CN	Desc	cription		
	4.	470	78	Row	crops, stra	aight row, (Good, HSG B
_	28.	250	89	Row	crops, stra	aight row, (Good, HSG D
	32.	720	87	Weig	ghted Aver	age	
	32.	720		100.	00% Pervi	ous Area	
	Тс	Lengt	h	Slope	Velocity	Capacity	Description
_	(min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)	
	83.1	1,63	5 (0.0020	0.33		Lag/CN Method,
					•		

Summary for Subcatchment 28ES:

Runoff = 50.44 cfs @ 12.53 hrs, Volume= 7.034 af, Depth= 3.33"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 10-yr Rainfall=4.75"

 Area	(ac)	CN	Desc	cription		
3.	970	78	8 Row	crops, stra	aight row, 0	Good, HSG B
 21.	360	89	Row	crops, stra	aight row, (Good, HSG D
25.	330	87	′ Weig	ghted Aver	age	
25.	330		100.	00% Pervi	ous Area	
Tc (min)	Lengt (fee		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
 . /			/		(CIS)	
42.0	1,07	5	0.0040	0.43		Lag/CN Method,

Summary for Reach 1ER:

Inflow Are	a =	176.340 ac,	0.00% Impervious, Inflow	Depth > 3.53" for 10-yr event
Inflow	=	158.23 cfs @	13.94 hrs, Volume=	51.918 af
Outflow	=	158.23 cfs @	13.94 hrs, Volume=	51.918 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs

Summary for Reach 2ER:

Inflow Are	ea =	151.950 ac,	0.00% Impervious, Inflow	/ Depth > 3.39"	for 10-yr event
Inflow	=	307.90 cfs @	12.50 hrs, Volume=	42.914 af	
Outflow	=	307.90 cfs @	12.50 hrs, Volume=	42.914 af, Atte	en= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs

Summary for Reach 3ER:

Inflow Are	ea =	412.580 ac,	0.00% Impervious, Inflow	/ Depth > 3.50"	for 10-yr event
Inflow	=	467.46 cfs @	12.91 hrs, Volume=	120.429 af	
Outflow	=	467.46 cfs @	12.91 hrs, Volume=	120.429 af, Atte	en= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs

Summary for Reach 4ER:

Inflow Are	a =	538.690 ac,	0.00% Impervious, Inflow	Depth > 3.22"	for 10-yr event
Inflow	=	894.76 cfs @	12.31 hrs, Volume=	144.612 af	-
Outflow	=	894.76 cfs @	12.31 hrs, Volume=	144.612 af, Atte	en= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs

Summary for Reach 5ER:

Inflow Are	ea =	113.580 ac,	0.00% Impervious, Infl	ow Depth > 3.43"	for 10-yr event
Inflow	=	185.30 cfs @	12.65 hrs, Volume=	32.469 af	•
Outflow	=	185.30 cfs @	12.65 hrs, Volume=	32.469 af, Atte	en= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs

Summary for Reach 6ER:

Inflow Area	a =	42.030 ac,	0.00% Impervious, Inflo	ow Depth > 3.43"	for 10-yr event
Inflow	=	77.25 cfs @	12.64 hrs, Volume=	12.021 af	
Outflow	=	77.25 cfs @	12.64 hrs, Volume=	12.021 af, Atte	en= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs

47.6

Summary for Subcatchment 1ES:

Runoff = 47.40 cfs @ 12.64 hrs, Volume= 7.821 af, Depth> 5.86"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 100-yr Rainfall=7.17"

Area (ac) CN Description										
16.020 89 Row crops, straight row, Good, HSG D										
16.020 100.00% Pervious Area										
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)										
51.0 1,260 0.0030 0.41 Lag/CN Method,										
Summary for Subcatchment 2ES: Runoff = 82.07 cfs @ 12.60 hrs. Volume= 13.139 af. Depth> 5.86"										
Runoff = 82.07 cfs @ 12.60 hrs, Volume= 13.139 af, Depth> 5.86"										
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 100-yr Rainfall=7.17"										
Area (ac) CN Description										
26.920 89 Row crops, straight row, Good, HSG D										
26.920 100.00% Pervious Area										
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)										

1,155 0.0030 0.40 Lag/CN Method,

Summary for Subcatchment 3ES:

Runoff = 364.63 cfs @ 12.47 hrs, Volume= 51.191 af, Depth> 5.64"

Area	(ac)	CN	Desc	ription					
17.	17.940 78 Row crops, straight row, Good, HSG B								
15.	15.860 85 Row crops, straight row, Good, HSG C								
75.	210	89	Row	crops, stra	aight row, C	Good, HSG D			
109.	010	87							
109.	010		100.0	00% Pervi	ous Area				
-			0		0				
Tc	Lengt		Slope	Velocity	Capacity	Description			
<u>(min)</u>	(feet	:)	(ft/ft)	(ft/sec)	(cfs)				
37.9	2,502	2 0.	.0190	1.10		Lag/CN Method,			

Summary for Subcatchment 4ES:

Runoff = 196.81 cfs @ 13.96 hrs, Volume= 68.355 af, Depth> 5.87"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 100-yr Rainfall=7.17"

Area	(ac) C	N Desc	cription									
139.690 89 Row crops, straight row, Good, HSG D												
139.690 100.00% Pervious Area												
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description							
158.9	2,625	0.0010	0.28		Lag/CN Method,							
	Summary for Subcatchment 5ES:											

Runoff = 58.81 cfs @ 13.73 hrs, Volume= 17.930 af, Depth> 5.87"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 100-yr Rainfall=7.17"

_	Area	(ac) C	N Des	scription								
_	36.	650 8	39 Rov	ow crops, straight row, Good, HSG D								
	36.650 100.00% Pervious Area											
	Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)							
_	133.2	2,106	0.0010	0.26		Lag/CN Method,						

Summary for Subcatchment 6ES:

Runoff = 104.58 cfs @ 12.60 hrs, Volume= 16.668 af, Depth> 5.86"

 Area	(ac) (CN	Desc	ription								
34.	150	89	Row	ow crops, straight row, Good, HSG D								
34.150 100.00% Pervious Area												
Tc (min)	Length (feet)		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description						
 47.5	1,380	0.0	0040	0.48		Lag/CN Method,						

Summary for Subcatchment 7ES:

Runoff = 57.47 cfs @ 12.39 hrs, Volume= 7.457 af, Depth> 5.74"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 100-yr Rainfall=7.17"

_	Area	(ac)	CN	Desc	cription						
	2.	2.110 85 Row crops, straight row, Good, HSG C									
	13.	470	89	Row	crops, stra	aight row, 0	Good, HSG D				
	15.	580	88	Weig	ghted Aver	age					
	15.	580		100.	00% Pervi	ous Area					
	_										
	Tc	Lengt		Slope	Velocity	Capacity	Description				
_	(min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)					
	32.2	67	5 (0.0030	0.35		Lag/CN Method,				
							-				

Summary for Subcatchment 8ES:

Runoff = 143.80 cfs @ 12.61 hrs, Volume= 23.294 af, Depth> 5.75"

Area (ac) CN Description									
14.510 85 Row crops, straight row, Good, HSG C									
34.120 89 Row crops, straight row, Good, HSG D									
48.630 88 Weighted Average									
48.630 100.00% Pervious Area									
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)									
49.5 1,783 0.0060 0.60 Lag/CN Method,									
Summary for Subcatchment 9ES:									
Runoff = 129.69 cfs @ 13.25 hrs, Volume= 32.484 af, Depth> 5.87"									
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 100-yr Rainfall=7.17"									
Area (ac) CN Description									
66.430 89 Row crops, straight row, Good, HSG D									
66.430 100.00% Pervious Area									

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
100.1	2,271	0.0020	0.38		Lag/CN Method,

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Summary for Subcatchment 10ES:	
Runoff = 136.54 cfs @ 12.95 hrs, Volume= 28.490 af, Depth> 5.75"	
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 100-yr Rainfall=7.17"	
Area (ac) CN Description	
10.900 85 Row crops, straight row, Good, HSG C 48.530 89 Row crops, straight row, Good, HSG D	
59.43088Weighted Average59.430100.00% Pervious Area	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
75.8 1,968 0.0030 0.43 Lag/CN Method,	
Summary for Subcatchment 11ES:	
Runoff = 150.65 cfs @ 13.02 hrs, Volume= 32.614 af, Depth> 5.86"	
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 100-yr Rainfall=7.17"	
Area (ac) CN Description	
5.740 85 Row crops, straight row, Good, HSG C 60.990 89 Row crops, straight row, Good, HSG D	
66.730 89 Weighted Average 66.730 100.00% Pervious Area	
(min) (feet) (ft/ft) (ft/sec) (cfs)	
80.1 1,720 0.0020 0.36 Lag/CN Method,	
Summary for Subcatchment 12ES:	
Runoff = 199.37 cfs @ 13.63 hrs, Volume= 59.503 af, Depth> 5.87"	
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 100-yr Rainfall=7.17"	
Area (ac) CN Description	
121.630 89 Row crops, straight row, Good, HSG D 121.630 100.00% Pervious Area	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
129.2 3,125 0.0020 0.40 Lag/CN Method,	

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IN-Posey 24-hr S1 100-yr Rainfall=7.17" Printed 4/4/2022 Page 31

_	Area (a	c) C	N Desc	cription					
	5.74	40 8	5 Row	crops, str	aight row, (Good, HSG C			
_	60.99	90 8	9 Row	crops, str	aight row, (
_	66.73	30 8	9 Weig						
	66.73	30	100.	00% Pervi	ous Area				
	Tc L	ength	Slope	Velocity	Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				

2022-03-23 Combined Pre-Post Model	IN-Posey 24-hr S1 100-yr Rainfall=7.17"
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Summary for Subcatchment 13ES:

Runoff 115.66 cfs @ 12.66 hrs, Volume= 19.483 af, Depth> 5.75" =

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 100-yr Rainfall=7.17"

	Area	(ac)	CN	Desc	Description							
	12.	570	70 85 Row crops, straight row, Good, HSG C									
	28.100 89 Row crops, straight row, Good, HSG D											
40.670 88 Weighted Average												
	40.	670										
	_					_						
	Тс	Lengt	h :	Slope	Velocity	Capacity	Description					
	(min)	(feet	:)	(ft/ft)	(ft/sec)	(cfs)						
	52.8	1,50	0 0	.0040	0.47		Lag/CN Method,					
		,										

Summary for Subcatchment 14ES:

Runoff 159.29 cfs @ 12.41 hrs, Volume= 21.123 af, Depth> 5.74" =

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 100-yr Rainfall=7.17"

Area (ac)	Area (ac) CN Description									
2.920 78 Row crops, straight row, Good, HSG B										
41.210 89 Row crops, straight row, Good, HSG D										
44.130	44.130 88 Weighted Average									
44.130		100.	00% Pervi	ous Area						
	ngth ^f eet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
33.6	852	0.0040	0.42		Lag/CN Method,					
Summary for Subcatchment 15ES:										

Runoff 202.46 cfs @ 12.31 hrs, Volume= 23.088 af, Depth= 4.96" =

Area (ac)	CN	Description						
34.820	78	Row crops, straight row, Good, HSG B						
18.940	89	Row crops, straight row, Good, HSG D						
2.060	55	Woods, Good, HSG B						
55.820 55.820	81	Weighted Average 100.00% Pervious Area						

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Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)										
25.9 1,535 0.0280 0.99 Lag/CN Method,										
Summary for Subcatchment 16ES:										
Runoff = 134.68 cfs @ 12.37 hrs, Volume= 16.414 af, Depth= 4.85"										
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 100-yr Rainfall=7.17"										
Area (ac) CN Description										
34.110 78 Row crops, straight row, Good, HSG B										
6.490 89 Row crops, straight row, Good, HSG D										
40.60080Weighted Average40.600100.00% Pervious Area										
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)										
29.9 1,600 0.0240 0.89 Lag/CN Method,										
Summary for Subcatchment 17ES:										
Runoff = 118.97 cfs @ 12.29 hrs, Volume= 13.277 af, Depth> 5.08"										
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 100-yr Rainfall=7.17"										
Area (ac) CN Description										
19.050 78 Row crops, straight row, Good, HSG B										
2.610 85 Row crops, straight row, Good, HSG C										
9.730 89 Row crops, straight row, Good, HSG D										
31.39082Weighted Average31.390100.00% Pervious Area										
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)										
24.5 1.200 0.0250 0.04 Lag/CN Mothod										

IN-Posey 24-hr S1 100-yr Rainfall=7.17"

24.5 1,390 0.0250 0.94 Lag/CN Method,

2022-03-23 Combined Pre-Post Model

Summary for Subcatchment 18ES:

Runoff = 32.29 cfs @ 12.26 hrs, Volume= 3.553 af, Depth> 5.85"

IN-Posey 24-hr S1 100-yr Rainfall=7.17" Prepared by Westwood Professional Services HydroCAD® 10.10-3a s/n 03363 © 2020 HydroCAD Software Solutions LLC Printed 4/4/2022 Page 34

Area (ac) CN Description								
7.290 89 Row crops, straight row, Good, HSG D								
7.290 100.00% Pervious Area								
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)								
22.8 850 0.0080 0.62 Lag/CN Method,								
Summary for Subcatchment 19ES:								
Runoff = 48.76 cfs @ 12.38 hrs, Volume= 6.320 af, Depth> 5.85"								
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 100-yr Rainfall=7.17"								
Area (ac) CN Description								
12.960 89 Row crops, straight row, Good, HSG D								
12.960 100.00% Pervious Area								
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)								
31.8 1,185 0.0070 0.62 Lag/CN Method,								
Summary for Subcatchment 20ES:								
Runoff = 134.30 cfs @ 12.27 hrs, Volume= 15.022 af, Depth> 5.74"								
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 100-yr Rainfall=7.17"								
Area (ac) CN Description								
2.950 78 Row crops, straight row, Good, HSG B								
28.450 89 Row crops, straight row, Good, HSG D 31.400 88 Weighted Average								
31.400 100.00% Pervious Area								
Tc Length Slope Velocity Capacity Description								
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)								
Tc Length Slope Velocity Capacity Description								
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)								
TcLengthSlopeVelocityCapacityDescription(min)(feet)(ft/ft)(ft/sec)(cfs)23.97890.00700.55Lag/CN Method,								

2022-03-23 Combined Pre-Post Model Prepared by Westwood Professional Services

IN-Posey 24-hr S1 100-yr Rainfall=7.17" Printed 4/4/2022 HydroCAD® 10.10-3a s/n 03363 © 2020 HydroCAD Software Solutions LLC Page 35

Area (ac) CN Description									
7.130 78 Row crops, straight row, Good, HSG B									
2.190 89 Row crops, straight row, Good, HSG D									
9.320 81 Weighted Average 9.320 100.00% Pervious Area									
Tc Length Slope Velocity Capacity Description									
(min) (feet) (ft/ft) (ft/sec) (cfs)									
12.1 690 0.0360 0.95 Lag/CN Method ,									
Summary for Subcatchment 22ES:									
Runoff = 82.74 cfs @ 12.16 hrs, Volume= 7.613 af, Depth> 5.84"									
Pureff by SCS TP 20 method LIU-SCS Weighted CN. Time Span- 5.00.40.00 bro. dt- 0.05 bro.									
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 100-yr Rainfall=7.17"									
Area (ac) CN Description									
15.630 89 Row crops, straight row, Good, HSG D									
15.630 100.00% Pervious Area									
Tc Length Slope Velocity Capacity Description									
(min) (feet) (ft/ft) (ft/sec) (cfs)									
15.2 790 0.0160 0.87 Lag/CN Method ,									
15.2 790 0.0160 0.87 Lag/CN Method, Summary for Subcatchment 23ES:									
Summary for Subcatchment 23ES:									
Summary for Subcatchment 23ES:Runoff = 430.51 cfs @ 12.28 hrs, Volume= 48.153 af, Depth> 5.63"									
Summary for Subcatchment 23ES:									
Summary for Subcatchment 23ES: Runoff = 430.51 cfs @ 12.28 hrs, Volume= 48.153 af, Depth> 5.63" Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 100-yr Rainfall=7.17"									
Summary for Subcatchment 23ES: Runoff = 430.51 cfs @ 12.28 hrs, Volume= 48.153 af, Depth> 5.63" Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 100-yr Rainfall=7.17" Area (ac) CN Description									
Summary for Subcatchment 23ES: Runoff = 430.51 cfs @ 12.28 hrs, Volume= 48.153 af, Depth> 5.63" Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 100-yr Rainfall=7.17" Area (ac) CN Description 15.700 78 Row crops, straight row, Good, HSG B									
Summary for Subcatchment 23ES: Runoff = 430.51 cfs @ 12.28 hrs, Volume= 48.153 af, Depth> 5.63" Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 100-yr Rainfall=7.17" Area (ac) CN Description 15.700 78 Row crops, straight row, Good, HSG B 86.900 89 Row crops, straight row, Good, HSG D 102.600 87									
Summary for Subcatchment 23ES: Runoff = 430.51 cfs @ 12.28 hrs, Volume= 48.153 af, Depth> 5.63" Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 100-yr Rainfall=7.17" Area (ac) CN Description 15.700 78 Row crops, straight row, Good, HSG B 86.900 89 Row crops, straight row, Good, HSG D									
Summary for Subcatchment 23ES:Runoff = 430.51 cfs @ 12.28 hrs, Volume= 48.153 af, Depth> 5.63"Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 100-yr Rainfall=7.17" $Area (ac) CN Description$ 15.700 78 Row crops, straight row, Good, HSG B 86.900 89 Row crops, straight row, Good, HSG D 102.600 87 Weighted Average 102.600 100.00% Pervious Area									
Summary for Subcatchment 23ES: Runoff = 430.51 cfs @ 12.28 hrs, Volume= 48.153 af, Depth> 5.63" Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 100-yr Rainfall=7.17" Area (ac) CN Description 15.700 78 Row crops, straight row, Good, HSG B 86.900 89 Row crops, straight row, Good, HSG D 102.600 87									
Summary for Subcatchment 23ES: Runoff = 430.51 cfs @ 12.28 hrs, Volume= 48.153 af, Depth> 5.63" Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 100-yr Rainfall=7.17" Area (ac) CN Description 15.700 78 Row crops, straight row, Good, HSG B 86.900 89 Row crops, straight row, Good, HSG D 102.600 87 Weighted Average 102.600 100.00% Pervious Area Tc Length Slope Velocity Capacity Description									
Summary for Subcatchment 23ES:Runoff= $430.51 \text{ cfs} @ 12.28 \text{ hrs, Volume} = 48.153 \text{ af, Depth> 5.63"}$ Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 100-yr Rainfall=7.17"Area (ac)CNDescription15.70078Row crops, straight row, Good, HSG B86.90089Row crops, straight row, Good, HSG D102.60087Weighted Average102.600100.00% Pervious AreaTcLengthSlopeVelocityCapacityDescription(min)(ft/ft)(ft/ft)(ft/sec)24.06150.00500.43Lag/CN Method,									
Summary for Subcatchment 23ES:Runoff=430.51 cfs @ 12.28 hrs, Volume=48.153 af, Depth> 5.63"Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 100-yr Rainfall=7.17" $Area (ac)$ CNDescription15.70078Row crops, straight row, Good, HSG B86.90089Row crops, straight row, Good, HSG B102.60087Weighted Average102.600100.00% Pervious AreaTcLengthSlopeVelocityCapacityDescription(min)(ft/ft)(ft/sec)24.06150.00500.43Lag/CN Method,Summary for Subcatchment 24ES:									
Summary for Subcatchment 23ES:Runoff= $430.51 \text{ cfs} @ 12.28 \text{ hrs, Volume} = 48.153 \text{ af, Depth> 5.63"}$ Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 100-yr Rainfall=7.17"Area (ac)CNDescription15.70078Row crops, straight row, Good, HSG B86.90089Row crops, straight row, Good, HSG D102.60087Weighted Average102.600100.00% Pervious AreaTcLengthSlopeVelocityCapacityDescription(min)(ft/ft)(ft/ft)(ft/sec)24.06150.00500.43Lag/CN Method,									

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IN-Posey 24-hr S1 100-yr Rainfall=7.17" Printed 4/4/2022 HydroCAD® 10.10-3a s/n 03363 © 2020 HydroCAD Software Solutions LLC Page 36

Area	a (ac)	CN	Desc	Description							
-	7.500 78 Row crops, straight row, Good, HSG B										
122.350 89 Row crops, straight row, Good, HSG D											
17.030 77 Woods, Good, HSG D											
14	6.880	87	′ Weig	ghted Aver	age						
140	146.880 100.00% Pervious Area										
Тс	: Leng	nth	Slope	Velocity	Capacity	Description					
(min)			(ft/ft)	(ft/sec)	(cfs)	Description					
		_/	/		(05)						
136.5	5 3,0 [,]	42	0.0020	0.37		Lag/CN Method,					

Summary for Subcatchment 25ES:

Runoff 122.16 cfs @ 12.63 hrs, Volume= 20.134 af, Depth> 5.75" =

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 100-yr Rainfall=7.17"

_	Area (ac) CN Description							
5.340 78 Row crops, straight row, Good, HSG B								
36.690 89 Row crops, straight row, Good, HSG D								
	42.	030	88	Weig	ghted Aver	age		
42.030 100.00% Pervious Area								
	Тс	Lengt	h :	Slope	Velocity	Capacity	Description	
_	(min)	(feet	:)	(ft/ft)	(ft/sec)	(cfs)		
	50.9	93	0 0	.0020	0.30		Lag/CN Method,	
							-	

Summary for Subcatchment 26ES:

Runoff 164.30 cfs @ 12.64 hrs, Volume= 27.109 af, Depth> 5.86" =

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 100-yr Rainfall=7.17"

Area (a	ac) Cl	N Desc	cription								
55.5	55.530 89 Row crops, straight row, Good, HSG D										
55.5	55.530 100.00% Pervious Area										
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description						
51.0	1,260	0.0030	0.41		Lag/CN Method,						
Summer of an Such actacher ant 27ES.											

Summary for Subcatchment 27ES:

Runoff 69.87 cfs @ 13.04 hrs, Volume= 15.381 af, Depth> 5.64" =

IN-Posey 24-hr S1 100-yr Rainfall=7.17" Printed 4/4/2022 is LLC Page 37

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_	Area	(ac)	CN	Desc	cription			
4.470 78 Row crops, straight row, Good, HSG B								
28.250 89 Row crops, straight row, Good, HSG D								
32.720 87 Weighted Average								
32.720 100.00% Pervious Area								
	То	Longt	h	Slope	Volocity	Consoity	Description	
	Tc (min)	Lengt (fee		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
	83.1	1,63	/).0020	0.33		Lag/CN Method,	
		,					•	

Summary for Subcatchment 28ES:

Runoff = 80.20 cfs @ 12.53 hrs, Volume= 11.897 af, Depth> 5.64"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 100-yr Rainfall=7.17"

Area	(ac)	CN	Desc	cription		
3.	.970	78	Row	crops, stra	aight row, 0	Good, HSG B
21.	.360	89	Row	crops, stra	aight row, (Good, HSG D
25.	.330	87	Weig	ghted Aver	age	
25.	25.330 100.00% Pervious Area					
Тс	Lengt	h	Slope	Velocity	Capacity	Description
(min)	(feet		(ft/ft)	(ft/sec)	(cfs)	Description
42.0	1,07	,	.0040	0.43	()	Lag/CN Method,

Summary for Reach 1ER:

Inflow Area =		176.340 ac,	0.00% Impervious, Inflow	v Depth > 5.87"	for 100-yr event
Inflow	=	253.24 cfs @	13.93 hrs, Volume=	86.286 af	
Outflow	=	253.24 cfs @	13.93 hrs, Volume=	86.286 af, Atte	en= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs

Summary for Reach 2ER:

Inflow Area =		151.950 ac,	0.00% Impervious, Inflo	w Depth > 5.70"	for 100-yr event
Inflow	=	486.76 cfs @	12.50 hrs, Volume=	72.151 af	
Outflow	=	486.76 cfs @	12.50 hrs, Volume=	72.151 af, Att	en= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs

Summary for Reach 3ER:

Inflow Area =		412.580 ac,	0.00% Impervious, Inflow	v Depth > 5.83"	for 100-yr event
Inflow	=	747.51 cfs @	12.91 hrs, Volume=	200.509 af	
Outflow	=	747.51 cfs @	12.91 hrs, Volume=	200.509 af, Atte	en= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs

Summary for Reach 4ER:

Inflow Area =		538.690 ac,	0.00% Impervious, Inflow	/ Depth > 5.50" fo	or 100-yr event
Inflow	=	1,446.73 cfs @	12.31 hrs, Volume=	246.972 af	-
Outflow	=	1,446.73 cfs @	12.31 hrs, Volume=	246.972 af, Atten=	: 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs

Summary for Reach 5ER:

Inflow Area =		113.580 ac,	0.00% Impervious, Inflow	v Depth > 5.75"	for 100-yr event
Inflow	=	293.79 cfs @	12.65 hrs, Volume=	54.386 af	-
Outflow	=	293.79 cfs @	12.65 hrs, Volume=	54.386 af, Atte	en= 0%, Lag= 0.0 min

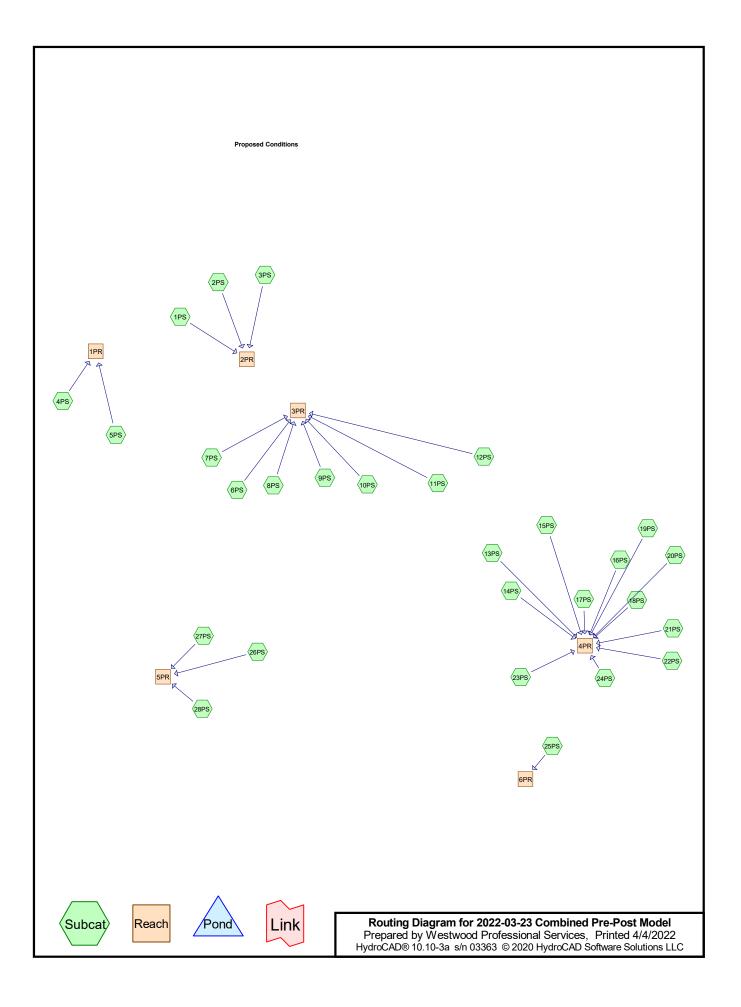
Routing by Stor-Ind+Trans method, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs

Summary for Reach 6ER:

Inflow Area =		42.030 ac,	0.00% Impervious, Infle	ow Depth > 5.75"	for 100-yr event
Inflow	=	122.16 cfs @	12.63 hrs, Volume=	20.134 af	
Outflow	=	122.16 cfs @	12.63 hrs, Volume=	20.134 af, Atte	en= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs

Appendix C Proposed HydroCAD Results



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Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
1	1-yr	IN-Posey 24-hr S1	1-yr	Default	24.00	1	2.74	2
2	2-yr	IN-Posey 24-hr S1	2-yr	Default	24.00	1	3.30	2
3	5-yr	IN-Posey 24-hr S1	5-yr	Default	24.00	1	4.10	2
4	10-yr	IN-Posey 24-hr S1	10-yr	Default	24.00	1	4.75	2
5	25-yr	IN-Posey 24-hr S1	25-yr	Default	24.00	1	5.66	2
6	50-yr	IN-Posey 24-hr S1	50-yr	Default	24.00	1	6.40	2
7	100-yr	IN-Posey 24-hr S1	100-yr	Default	24.00	1	7.17	2

Rainfall Events Listing

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Area Listing (selected nodes)

Area	CN	Description
(acres)		(subcatchment-numbers)
2.140	96	Gravel surface, HSG B (15PS, 16PS, 17PS, 21PS)
29.020	96	Gravel surface, HSG D (1PS, 2PS, 3PS, 4PS, 5PS, 6PS, 7PS, 8PS, 9PS, 10PS, 11PS,
		12PS, 13PS, 14PS, 18PS, 19PS, 20PS, 22PS, 23PS, 24PS, 25PS, 26PS, 27PS, 28PS)
155.820	58	Meadow, non-grazed, HSG B (3PS, 14PS, 15PS, 16PS, 17PS, 20PS, 21PS, 23PS,
		24PS, 25PS, 27PS, 28PS)
64.300	71	Meadow, non-grazed, HSG C (3PS, 7PS, 8PS, 10PS, 11PS, 13PS, 17PS)
1,183.890	78	Meadow, non-grazed, HSG D (1PS, 2PS, 3PS, 4PS, 5PS, 6PS, 7PS, 8PS, 9PS, 10PS,
		11PS, 12PS, 13PS, 14PS, 15PS, 16PS, 17PS, 18PS, 19PS, 20PS, 21PS, 22PS, 23PS,
		24PS, 25PS, 26PS, 27PS, 28PS)
1,435.170	76	TOTAL AREA

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Soil Listing (selected nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
0.000	HSG A	
157.960	HSG B	3PS, 14PS, 15PS, 16PS, 17PS, 20PS, 21PS, 23PS, 24PS, 25PS, 27PS, 28PS
64.300	HSG C	3PS, 7PS, 8PS, 10PS, 11PS, 13PS, 17PS
1,212.910	HSG D	1PS, 2PS, 3PS, 4PS, 5PS, 6PS, 7PS, 8PS, 9PS, 10PS, 11PS, 12PS, 13PS, 14PS, 15PS, 16PS, 17PS, 18PS, 19PS, 20PS, 21PS, 22PS, 23PS, 24PS, 25PS, 26PS, 27PS, 28PS
0.000	Other	
1,435.170		TOTAL AREA

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HSG-A	HSG-B	HSG-C	HSG-D	Other	Total	Ground	Subcatchment Numbers
(acres) 0.000	(acres) 2.140	(acres) 0.000	(acres) 29.020	(acres) 0.000	(acres) 31.160	Cover Gravel surface	Numbers 1PS, 2PS, 3PS, 4PS, 5PS, 6PS, 7PS, 8PS, 9PS, 10PS, 11PS, 12PS, 13PS, 14PS, 15PS, 16PS, 17PS, 18PS, 19PS, 20PS, 21PS, 22PS, 23PS, 24PS, 25PS, 26PS, 27PS,
0.000	155.820	64.300	1,183.890	0.000	1,404.010	Meadow, non-grazed	28PS 1PS, 2PS, 3PS, 4PS, 5PS, 6PS, 7PS, 8PS, 9PS, 10PS, 11PS, 12PS, 13PS, 14PS, 15PS, 16PS, 17PS, 18PS, 19PS, 20PS, 21PS, 22PS, 23PS, 24PS, 25PS, 26PS, 27PS, 28PS
0.000	157.960	64.300	1,212.910	0.000	1,435.170	TOTAL AREA	

Ground Covers (selected nodes)

Summary for Subcatchment 1PS:

Runoff = 9.11 cfs @ 12.99 hrs, Volume= 1.798 af, Depth= 1.35"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 2-yr Rainfall=3.30"

_	Area	(ac)	CN	Desc	Description						
	15.670 78 Meadow, non-grazed, HSG D										
_	0.	350	96	Grav	el surface	, HSG D					
	16.	020	78	Weig	ghted Aver	age					
	16.	020		100.	00% Pervi	ous Area					
	Тс	Lengt		Slope	Velocity	Capacity	Description				
_	(min)	(feet	t)	(ft/ft)	(ft/sec)	(cfs)					
	74.2	1,26	0 0	0.0030	0.28		Lag/CN Method,				
		-					-				

Summary for Subcatchment 2PS:

Runoff = 15.93 cfs @ 12.93 hrs, Volume= 3.022 af, Depth= 1.35"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 2-yr Rainfall=3.30"

Area (a	c) Cl	N Dese	cription				
26.37	70 7	8 Mea	dow, non-g	grazed, HS	GD		
0.55	50 9	6 Grav	el surface	, HSG D			
26.92	20 7	8 Weig	ghted Aver	age			
26.92	20	100.	00% Pervi	ous Area			
Tc L (min)	_ength (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
69.2	1,155	0.0030	0.28		Lag/CN Method,		

Summary for Subcatchment 3PS:

Runoff = 57.75 cfs @ 12.80 hrs, Volume= 10.028 af, Depth= 1.10"

_	Area (ac)	CN	Description				
	17.940	58	Meadow, non-grazed, HSG B				
	15.860	71	Meadow, non-grazed, HSG C				
	72.950	78	Meadow, non-grazed, HSG D				
_	2.260	96	Gravel surface, HSG D				
	109.010 109.010	74	Weighted Average 100.00% Pervious Area				
	109.010		100.00% Fervious Area				

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Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)									
57.4 2,502 0.0190 0.73 Lag/CN Method,									
Summary for Subcatchment 4PS:									
Runoff = 34.56 cfs @ 15.16 hrs, Volume= 15.682 af, Depth= 1.35"									
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 2-yr Rainfall=3.30"									
Area (ac) CN Description									
138.240 78 Meadow, non-grazed, HSG D 1.450 96 Gravel surface, HSG D									
139.690 78 Weighted Average 139.690 100.00% Pervious Area	139.690 78 Weighted Average								
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)									
231.2 2,625 0.0010 0.19 Lag/CN Method,									
Summary for Subcatchment 5PS:									
Runoff = 11.11 cfs @ 14.45 hrs, Volume= 4.313 af, Depth= 1.41"									
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 2-yr Rainfall=3.30"									
Area (ac) CN Description									
35.500 78 Meadow, non-grazed, HSG D 1.150 96 Gravel surface, HSG D									
36.65079Weighted Average36.650100.00% Pervious Area									
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)									
188.1 2,106 0.0010 0.19 Lag/CN Method,									

IN-Posey 24-hr S1 2-yr Rainfall=3.30"

Summary for Subcatchment 6PS:

Runoff = 20.26 cfs @ 12.94 hrs, Volume= 3.834 af, Depth= 1.35"

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IN-Posey 24-hr S1 2-yr Rainfall=3.30" Printed 4/4/2022 HydroCAD® 10.10-3a s/n 03363 © 2020 HydroCAD Software Solutions LLC Page 8

Area (ac) CN Description								
33.400 78 Meadow, non-grazed, HSG D								
0.750 96 Gravel surface, HSG D								
34.150 78 Weighted Average								
34.150 100.00% Pervious Area								
To Longth Clans, Malasity Consolity Description								
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)								
69.1 1,380 0.0040 0.33 Lag/CN Method,								
09.1 1,500 0.0040 0.55 Lag/CN Method ,								
Summary for Subcatchment 7PS:								
Runoff = 12.11 cfs @ 12.60 hrs, Volume= 1.749 af, Depth= 1.35"								
Duraff hu COC TD 20 m athad UU - COC Wainkted ON Times Craw - 5 00 40 00 hm atha 0.05 hm								
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 2-yr Rainfall=3.30"								
IN-FOSEY 24-III ST Z-YI Raillall-3.50								
Area (ac) CN Description								
2.110 71 Meadow, non-grazed, HSG C								
13.030 78 Meadow, non-grazed, HSG D								
0.440 96 Gravel surface, HSG D								
15.580 78 Weighted Average								
15.580 100.00% Pervious Area								
To Longth Slong Volgetty Conseity Description								
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)								
45.0 675 0.0030 0.25 Lag/CN Method,								
-0.0 070 0.0000 0.20 Lug/on monou,								
Summary for Subcatchment 8PS:								

Runoff 24.95 cfs @ 13.01 hrs, Volume= 4.953 af, Depth= 1.22" =

 Area	(ac)	CN	Desc	Description					
14.	510	71			grazed, HS				
32.	700	78	Mea	dow, non-g	grazed, HS	SG D			
 1.	420	96	Grav	el surface	, HSG D				
48.	630	76	Weig	ghted Aver	age				
48.	630		100.0	00% Pervi	ous Area				
Tc	Lengt	h	Slope	Velocity	Capacity	Description			
 (min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)				
 73.5	1,78	3 0	0.0060	0.40		Lag/CN Method,			
	-					-			

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Summary for Subcatchment 9PS:

Runoff = 23.07 cfs @ 14.05 hrs, Volume= 7.457 af, Depth= 1.35"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 2-yr Rainfall=3.30"

_	Area	(ac)	CN	Desc	Description					
65.520 78 Meadow, non-grazed,						grazed, HS	GD			
_	0.	910	96	Grav	el surface	, HSG D				
	66.	430	78	Weig	ghted Aver	age				
	66.	430		100.	00% Pervi	ous Area				
	_			~		a 14	—			
	Tc	Lengt	h	Slope	Velocity	Capacity	Description			
	(min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)				
	145.6	2,27	1 0	.0020	0.26		Lag/CN Method,			
		,					•			

Summary for Subcatchment 10PS:

Runoff = 24.51 cfs @ 13.48 hrs, Volume= 6.358 af, Depth= 1.28"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 2-yr Rainfall=3.30"

_	Area	(ac)	CN	Desc	Description					
	10.	900	71	Mea	dow, non-g	grazed, HS	SG C			
	47.	540	78	Mea	dow, non-g	grazed, HS	G D			
	0.	990	96	Grav	el surface	, HSG D				
	59.	430	77	Weig	ghted Aver	age				
	59.	430		100.0	00% Pervi	ous Area				
	Тс	Lengt	h	Slope	Velocity	Capacity	Description			
	(min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)				
	109.2	1,96	8 0	.0030	0.30		Lag/CN Method,			
		-					-			

Summary for Subcatchment 11PS:

Runoff = 27.72 cfs @ 13.60 hrs, Volume= 7.491 af, Depth= 1.35"

Area (ac)	CN	Description				
5.740	71	Meadow, non-grazed, HSG C				
60.260	78	Meadow, non-grazed, HSG D				
0.730	96	Gravel surface, HSG D				
66.730	78	Weighted Average				
66.730		100.00% Pervious Area				

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Tc I (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
116.6	1,720	0.0020	0.25		Lag/CN Method	i,		
Summary for Subcatchment 12PS:								
Runoff	= 3	34.89 cfs	@ 14.4	5 hrs, Volu	ime= 13.65	54 af, Depth= 1.35"		
	Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 2-yr Rainfall=3.30"							
Area (a	ac) CN	Desc	ription					
119.4				grazed, HS	G D			
2.2			el surface					
121.63 121.63			hted Aver)0% Pervi					
Tc I (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
188.0	3,125	0.0020	0.28		Lag/CN Method	1,		
			Su	mmary fo	r Subcatchme	nt 13PS:		
Runoff	= 1	19.82 cfs	@ 13.0	7 hrs, Volu	ime= 4.14	42 af, Depth= 1.22"		
	Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 2-yr Rainfall=3.30"							
Area (a	ac) CN	Desc	ription					
12.5				grazed, HS				
27.43				grazed, HS	G D			
0.6			el surface	,				
40.6 [°] 40.6 [°]			hted Aver)0% Pervi					
Tc I (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			

IN-Posey 24-hr S1 2-yr Rainfall=3.30"

2022-03-23 Combined Pre-Post Model

1,500 0.0040

78.4

Summary for Subcatchment 14PS:

Lag/CN Method,

Runoff = 31.02 cfs @ 12.66 hrs, Volume= 4.721 af, Depth= 1.28"

0.32

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IN-Posey 24-hr S1 2-yr Rainfall=3.30" Printed 4/4/2022 HydroCAD® 10.10-3a s/n 03363 © 2020 HydroCAD Software Solutions LLC Page 11

Area (ac) CN Description	Description					
2.920 58 Meadow, non-grazed, HSG B						
39.940 78 Meadow, non-grazed, HSG D						
1.270 96 Gravel surface, HSG D						
44.130 77 Weighted Average						
44.130 100.00% Pervious Area						
Tc Length Slope Velocity Capacity Description						
(min) (feet) (ft/ft) (ft/sec) (cfs)						
48.4 852 0.0040 0.29 Lag/CN Method,						
Summary for Subcatchment 15PS:						
Runoff = 18.05 cfs @ 12.64 hrs, Volume= 3.022 af, Depth= 0.65"						
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs						

IN-Posey 24-hr S1 2-yr Rainfall=3.30"

Area (ac)	CN	Desc	Description					
36.410 58 Meadow, non-grazed, HSG B								
18.940	78	Mea	dow, non-	grazed, HS	GD			
0.470	96	Grav	el surface	, HSG B				
55.820	65	Weig	ghted Aver	age				
55.820		100.0	00% Pervi	ous Area				
Tc Leng (min) (fe	gth et)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
40.8 1,5	35 ().0280	0.63		Lag/CN Method,			
Summary for Subcatchment 16PS:								

1.774 af, Depth= 0.52" Runoff 8.74 cfs @ 12.80 hrs, Volume= =

Area	(ac)	CN	Desc	cription		
33	8.590	58	3 Mea	dow, non-g	grazed, HS	G B
6	6.490	78	3 Mea	dow, non-g	grazed, HS	G D
0).520	96	6 Grav	el surface	, HSG B	
40	0.600	62	2 Weig	ghted Aver	age	
40	0.600		100.	00% Pervi	ous Area	
_			~		a 14	
Tc		· .	Slope	Velocity	Capacity	Description
(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)	
49.2	1,6	00	0.0240	0.54		Lag/CN Method,

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Summary for Subcatchment 17PS:

Runoff = 11.44 cfs @ 12.59 hrs, Volume= 1.816 af, Depth= 0.69"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 2-yr Rainfall=3.30"

	Area	(ac)	CN	Desc	cription			
	18.	320	58	Mea	dow, non-g	grazed, HS	GB	
	2.	610	71			grazed, HS		
	9.	730	78	Mea	dow, non-g	grazed, HS	GD	
_	0.	730	96	Grav	el surface	, HSG B		
	31.	390	66	Weig	ghted Aver	age		
	31.	390		100.0	00% Pervi	ous Area		
	Tc (min)	Leng (fee		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
_	38.8	1,39	90 ().0250	0.60		Lag/CN Method,	

Summary for Subcatchment 18PS:

Runoff	=	6.69 cfs @	12.43 hrs,	Volume=	0.818 af, Depth= 1.35"
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Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 2-yr Rainfall=3.30"

Area	a (ac)	C١	Deso	cription		
	7.100	78	8 Mea	dow, non-g	grazed, HS	SG D
	0.190	96	6 Grav	el surface	, HSG D	
	7.290	78	8 Weig	ghted Aver	age	
	7.290		100.	00% Pervi	ous Area	
Тс	: Leng	jth	Slope	Velocity	Capacity	Description
(min)) (fe	et)	(ft/ft)	(ft/sec)	(cfs)	
33.2	2 8	50	0.0080	0.43		Lag/CN Method,

Summary for Subcatchment 19PS:

Runoff = 9.87 cfs @ 12.62 hrs, Volume= 1.455 af, Depth= 1.35"

Area (ac)	CN	Description
12.790	78	Meadow, non-grazed, HSG D
0.170	96	Gravel surface, HSG D
12.960	78	Weighted Average
12.960		100.00% Pervious Area

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Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)						
46.3 1,185 0.0070 0.43 Lag/CN Method,						
Summary for Subcatchment 20PS:						
Runoff = 30.82 cfs @ 12.42 hrs, Volume= 3.695 af, Depth= 1.41"						
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= IN-Posey 24-hr S1 2-yr Rainfall=3.30"	0.05 hrs					
Area (ac) CN Description						
2.950 58 Meadow, non-grazed, HSG B						
23.180 78 Meadow, non-grazed, HSG D 5.270 96 Gravel surface, HSG D						
5.270 96 Gravel surface, HSG D 31.400 79 Weighted Average						
31.400 100.00% Pervious Area						
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)						
32.4 789 0.0070 0.41 Lag/CN Method,						
Summary for Subcatchment 21PS:						
Summary for Subcatchment 211 S.						
Runoff = 3.84 cfs @ 12.28 hrs, Volume= 0.471 af, Depth= 0.61"						
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 2-yr Rainfall=3.30"						
Area (ac) CN Description						
6.710 58 Meadow, non-grazed, HSG B						
2.190 78 Meadow, non-grazed, HSG D						
0.420 96 Gravel surface, HSG B						
9.320 64 Weighted Average 9.320 100.00% Pervious Area	9.32064Weighted Average9.320100.00% Pervious Area					
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)						
19.5 690 0.0360 0.59 Lag/CN Method,						

IN-Posey 24-hr S1 2-yr Rainfall=3.30"

Summary for Subcatchment 22PS:

Runoff = 17.77 cfs @ 12.27 hrs, Volume= 1.755 af, Depth= 1.35"

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IN-Posey 24-hr S1 2-yr Rainfall=3.30" Printed 4/4/2022 HydroCAD® 10.10-3a s/n 03363 © 2020 HydroCAD Software Solutions LLC Page 14

· · · · · · · · · · · · · · · · · · ·					
Area (ac) CN Description					
15.380 78 Meadow, non-grazed, HSG D					
0.250 96 Gravel surface, HSG D					
15.63078Weighted Average15.630100.00% Pervious Area					
Tc Length Slope Velocity Capacity Description _(min) (feet) (ft/ft) (ft/sec) (cfs)					
22.1 790 0.0160 0.60 Lag/CN Method,					
Summary for Subcatchment 23PS:Runoff = 76.49 cfs @ 12.48 hrs, Volume= 9.937 af, Depth= 1.16"					
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 2-yr Rainfall=3.30"					
Area (ac) CN Description					
15.700 58 Meadow, non-grazed, HSG B					
84.200 78 Meadow, non-grazed, HSG D					
2.700 96 Gravel surface, HSG D					
102.600 75 Weighted Average					
102.600 100.00% Pervious Area					
To Longth Slong Velocity Conscity Description					

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
35.4	615	0.0050	0.29		Lag/CN Method,

Summary for Subcatchment 24PS:

Runoff	=	39.93 cfs @	14.55 hrs, Volume=	15.713 af, Depth= 1.28"
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 Area	(ac)	CN	Desc	cription		
7.	500	58	Mea	dow, non-g	grazed, HS	GG B
136.	740	78	Mea	dow, non-g	grazed, HS	G D
 2.	640	96	Grav	el surface	, HSG D	
146.	880	77	Weig	ghted Aver	age	
146.	880		100.	00% Pervi	ous Area	
Tc	Leng	th	Slope	Velocity	Capacity	Description
 (min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)	
 189.5	3,04	2 (0.0020	0.27		Lag/CN Method,
	,					

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Summary for Subcatchment 25PS:

Runoff = 21.12 cfs @ 13.04 hrs, Volume= 4.280 af, Depth= 1.22"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 2-yr Rainfall=3.30"

Area	(ac)	CN	Desc	cription		
5.	340	58	Mea	dow, non-g	grazed, HS	G B
36.	400	78	Mea	dow, non-g	grazed, HS	ig D
0.	290	96	Grav	el surface	, HSG D	
42.	030	76	Weig	ghted Aver	age	
42.	030		100.0	00% Pervi	ous Area	
Tc	Leng	th	Slope	Velocity	Capacity	Description
(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)	
75.6	93	30	0.0020	0.20		Lag/CN Method,
						-

Summary for Subcatchment 26PS:

Runoff = 31.58 cfs @ 12.99 hrs, Volume= 6.234 af, Depth= 1.35"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 2-yr Rainfall=3.30"

 Area	(ac)	CN	Desc	cription		
54.	190	78	Mea	dow, non-g	grazed, HS	GD
 1.	340	96	Grav	el surface	, HSG D	
55.	530	78	Weig	ghted Aver	age	
55.530 100.00% Pervious Area						
-			<u>.</u>		A	
Tc	Lengt		Slope	Velocity	Capacity	Description
(min)	(feet	:)	(ft/ft)	(ft/sec)	(cfs)	
74.2	1,26	0 C	.0030	0.28		Lag/CN Method,
	,					-

Summary for Subcatchment 27PS:

Runoff = 11.98 cfs @ 13.61 hrs, Volume= 3.332 af, Depth= 1.22"

Area (ac)	CN	Description
4.470	58	Meadow, non-grazed, HSG B
27.350	78	Meadow, non-grazed, HSG D
0.900	96	Gravel surface, HSG D
32.720	76	Weighted Average
32.720		100.00% Pervious Area

HydroCA	HydroCAD® 10.10-3a s/n 03363 © 2020 HydroCAD Software Solutions LLC Pa						
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
118.8	1,635	0.0020	0.23		Lag/CN Method,		
	Summary for Subcatchment 28PS:						
Runoff	=	13.69 cf	s@ 12.8	6 hrs, Volu	me= 2.453 af, Depth= 1.16"		
IN-Pose	Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 2-yr Rainfall=3.30" Area (ac) CN Description						
				grazed, HS	GB		
21	.260 7	78 Mea	dow, non-	grazed, HS	G D		
0	.100 9	96 Grav	vel surface	e, HSG D			
25	.330 7		ghted Ave				
25	25.330 100.00% Pervious Area						
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
61 8	1 075	0 0040	0.20				

Lag/CN Method, 61.8 1,075 0.0040 0.29

Summary for Reach 1PR:

Inflow Area =	176.340 ac,	0.00% Impervious, Inflow	Depth = 1.36" for 2-yr event
Inflow =	44.58 cfs @	15.08 hrs, Volume=	19.995 af
Outflow =	44.58 cfs @	15.08 hrs, Volume=	19.995 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs

Summary for Reach 2PR:

Inflow Area	a =	151.950 ac,	0.00% Impervious, Inflow	Depth = 1.17"	for 2-yr event
Inflow	=	81.89 cfs @	12.84 hrs, Volume=	14.849 af	
Outflow	=	81.89 cfs @	12.84 hrs, Volume=	14.849 af, Atte	n= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs

Summary for Reach 3PR:

Inflow Are	ea =	412.580 ac,	0.00% Impervious, Ir	nflow Depth = 1.32"	for 2-yr event
Inflow	=	128.57 cfs @	13.50 hrs, Volume=	45.496 af	
Outflow	=	128.57 cfs @	13.50 hrs, Volume=	45.496 af, Atte	en= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs

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IN-Posey 24-hr S1 2-yr Rainfall=3.30" Printed 4/4/2022

Summary for Reach 4PR:

Inflow Area =		538.690 ac,	0.00% Impervious, Inflow	v Depth = 1.10"	for 2-yr event
Inflow	=	211.59 cfs @	12.54 hrs, Volume=	49.319 af	
Outflow	=	211.59 cfs @	12.54 hrs, Volume=	49.319 af, Atte	en= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs

Summary for Reach 5PR:

Inflow Area =	113.580 ac,	0.00% Impervious, Inflow	/ Depth = 1.27"	for 2-yr event
Inflow =	52.63 cfs @	13.03 hrs, Volume=	12.019 af	
Outflow =	52.63 cfs @	13.03 hrs, Volume=	12.019 af, Atte	en= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs

Summary for Reach 6PR:

Inflow Area =		42.030 ac,	0.00% Impervious, Inflow	/ Depth = 1.22"	for 2-yr event
Inflow	=	21.12 cfs @	13.04 hrs, Volume=	4.280 af	-
Outflow	=	21.12 cfs @	13.04 hrs, Volume=	4.280 af, Atte	en= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs

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Summary for Subcatchment 1PS:

Runoff = 17.23 cfs @ 12.97 hrs, Volume= 3.339 af, Depth= 2.50"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 10-yr Rainfall=4.75"

_	Area	(ac)	CN	Desc	Description						
	15.	670	78	Mea	Meadow, non-grazed, HSG D						
0.350 96 Gravel surface, HSG D											
	16.020 78 Weighted Average										
	16.020			100.	00% Pervi	ous Area					
	Тс	Lengt	h	Slope	Velocity	Capacity	Description				
	(min)	(feet	t)	(ft/ft)	(ft/sec)	(cfs)					
_	74.2	1,26	0 0	.0030	0.28		Lag/CN Method,				
		,									

Summary for Subcatchment 2PS:

Runoff = 30.06 cfs @ 12.90 hrs, Volume= 5.610 af, Depth= 2.50"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 10-yr Rainfall=4.75"

Area (ad	c) Cl	N Dese	Description						
26.37	07	8 Mea	dow, non-g	grazed, HS	GD				
0.55	50 9	6 Grav	el surface	, HSG D					
26.92	20 7	8 Weig	ghted Aver	age					
26.92	20	100.	00% Pervi	ous Area					
	ength (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
69.2	1,155	0.0030	0.28		Lag/CN Method,				
	Summary for Subastable 205								

Summary for Subcatchment 3PS:

Runoff = 117.31 cfs @ 12.78 hrs, Volume= 19.681 af, Depth= 2.17"

A	Area (ac)	CN	Description			
	17.940	58	Meadow, non-grazed, HSG B			
	15.860	71	Meadow, non-grazed, HSG C			
	72.950	78	Meadow, non-grazed, HSG D			
	2.260	96	Gravel surface, HSG D			
	109.010 109.010	74	Weighted Average 100.00% Pervious Area			

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Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)										
57.4 2,502 0.0190 0.73 Lag/CN Method,										
Summary for Subcatchment 4PS:										
Runoff = 65.73 cfs @ 15.14 hrs, Volume= 29.112 af, Depth= 2.50"										
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 10-yr Rainfall=4.75"										
Area (ac) CN Description										
138.240 78 Meadow, non-grazed, HSG D 1.450 96 Gravel surface, HSG D										
139.690 78 Weighted Average										
139.690 100.00% Pervious Area										
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)										
231.2 2,625 0.0010 0.19 Lag/CN Method,										
Summary for Subcatchment 5PS:										
Runoff = 20.87 cfs @ 14.43 hrs, Volume= 7.903 af, Depth= 2.59"										
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 10-yr Rainfall=4.75"										
Area (ac) CN Description										
35.500 78 Meadow, non-grazed, HSG D 1.150 96 Gravel surface, HSG D										
36.650 79 Weighted Average										
36.650 100.00% Pervious Area										
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)										
188.1 2,106 0.0010 0.19 Lag/CN Method,										
Summary for Subcatchment 6PS:										

IN-Posey 24-hr S1 10-yr Rainfall=4.75"

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Runoff = 38.17 cfs @ 12.92 hrs, Volume= 7.117 af, Depth= 2.50"

IN-Posey 24-hr S1 10-yr Rainfall=4.75" Printed 4/4/2022 LLC Page 20

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Area (ac) CN Description								
33.400 78 Meadow, non-grazed, HSG D								
0.750 96 Gravel surface, HSG D								
34.150 78 Weighted Average								
34.150 100.00% Pervious Area								
Tc Length Slope Velocity Capacity Description								
(min) (feet) (ft/ft) (ft/sec) (cfs)								
69.1 1,380 0.0040 0.33 Lag/CN Method,								
Summary for Subcatchment 7PS:								
Runoff = 22.64 cfs @ 12.58 hrs, Volume= 3.247 af, Depth= 2.50"								
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 10-yr Rainfall=4.75"								
Area (ac) CN Description								
2.110 71 Meadow, non-grazed, HSG C								
13.030 78 Meadow, non-grazed, HSG D								
0.440 96 Gravel surface, HSG D								
15.580 78 Weighted Average 15.580 100.00% Pervious Area								
10.000 100.00 % 1 el vious Alea								
Tc Length Slope Velocity Capacity Description								
(min) (feet) (ft/ft) (ft/sec) (cfs)								
45.0 675 0.0030 0.25 Lag/CN Method,								
Summary for Subcatchment 8PS:								
Runoff = 48.81 cfs @ 12.99 hrs, Volume= 9.447 af, Depth= 2.33"								
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 10-yr Rainfall=4.75"								

_	Area	(ac)	CN	Desc	Description						
	14.	510	71	Mead	Meadow, non-grazed, HSG C						
	32.	700									
1.420 96			Grav	Gravel surface, HSG D							
	48.	630	76	Weig	ghted Aver	age					
	48.	630		100.0	00% Pervi	ous Area					
	Тс	Lengt	h	Slope	Velocity	Capacity	Description				
_	(min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)					
	73.5	1,78	30	.0060	0.40		Lag/CN Method,				
							-				

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Summary for Subcatchment 9PS:

Runoff = 43.92 cfs @ 13.91 hrs, Volume= 13.844 af, Depth= 2.50"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 10-yr Rainfall=4.75"

_	Area	(ac)	CN	Desc	Description					
	65.520 78 Meadow, non-grazed, HSG						G D			
0.910 96			Grav	Gravel surface, HSG D						
66.430 78 Weighted Average										
	66.430			100.	00% Pervi	ous Area				
	Tc	Leng	th	Slope	Velocity	Capacity	Description			
	(min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)				
	145.6	2,27	<u>′1</u> (0.0020	0.26		Lag/CN Method,			
		,								

Summary for Subcatchment 10PS:

Runoff = 47.35 cfs @ 13.46 hrs, Volume= 11.962 af, Depth= 2.42"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 10-yr Rainfall=4.75"

	Area	(ac)	CN	Desc	cription		
10.900 71 Meadow, non-grazed, HSG C							
	47.	540	78	Mea	dow, non-g	grazed, HS	G D
	0.	990	96	Grav	el surface	, HSG D	
	59.	430	77	Weig	ghted Aver	age	
	59.	430		100.	00% Pervi	ous Area	
	_						
	Тс	Lengt		Slope	Velocity	Capacity	Description
	(min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)	
_	109.2	1,96	8 0	0.0030	0.30		Lag/CN Method,
		-,					

Summary for Subcatchment 11PS:

Runoff = 52.48 cfs @ 13.58 hrs, Volume= 13.907 af, Depth= 2.50"

Area (ac)	CN	Description
5.740	71	Meadow, non-grazed, HSG C
60.260	78	Meadow, non-grazed, HSG D
0.730	96	Gravel surface, HSG D
66.730	78	Weighted Average
66.730		100.00% Pervious Area

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	Faye 22							
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)								
116.6 1,720 0.0020 0.25 Lag/CN Method,								
Summary for Subcatchment 12PS:								
Runoff = 66.76 cfs @ 14.42 hrs, Volume= 25.348 af, Depth= 2.50"								
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0. IN-Posey 24-hr S1 10-yr Rainfall=4.75"	.05 hrs							
Area (ac) CN Description								
119.400 78 Meadow, non-grazed, HSG D								
2.230 96 Gravel surface, HSG D								
121.63078Weighted Average121.630100.00% Pervious Area								
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)								
188.0 3,125 0.0020 0.28 Lag/CN Method,								
Summary for Subcatchment 13PS:								
Runoff = 38.95 cfs @ 13.03 hrs, Volume= 7.900 af, Depth= 2.33"								
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0. IN-Posey 24-hr S1 10-yr Rainfall=4.75"	.05 hrs							
Area (ac) CN Description								
12.570 71 Meadow, non-grazed, HSG C								
27.430 78 Meadow, non-grazed, HSG D								
0.670 96 Gravel surface, HSG D								
40.67076Weighted Average40.670100.00% Pervious Area								
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)								

IN-Posey 24-hr S1 10-yr Rainfall=4.75"

78.4 1,500 0.0040 0.32 Lag/CN Method,

2022-03-23 Combined Pre-Post Model

Summary for Subcatchment 14PS:

Runoff = 59.09 cfs @ 12.65 hrs, Volume= 8.882 af, Depth= 2.42"

IN-Posey 24-hr S1 10-yr Rainfall=4.75" Printed 4/4/2022 Page 23

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Area (ac) CN Description							
2.920 58 Meadow, non-grazed, HSG B							
39.940 78 Meadow, non-grazed, HSG D							
1.270 96 Gravel surface, HSG D							
44.130 77 Weighted Average							
44.130 100.00% Pervious Area							
Tc Length Slope Velocity Capacity Description							
(min) (feet) (ft/ft) (ft/sec) (cfs)							
48.4 852 0.0040 0.29 Lag/CN Method,							
Summary for Subcatchment 15PS:							
Runoff = 47.22 cfs @ 12.58 hrs, Volume= 6.929 af, Depth= 1.49"							
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs							
IN-Posey 24-hr S1 10-yr Rainfall=4.75"							
Area (ac) CN Description							

_	71100	(40)		0030	npuon					
	36.	410	58	Mea	dow, non-g	grazed, HS	G B			
	18.940 78 Meadow, non-grazed, HSG D									
_	0.	470	96	Grav	el surface	, HSG B				
	55.	820	65	Weig	ghted Aver	age				
	55.	820		100.	00% Pervi	ous Area				
	Tc	Lengt	h	Slope	Velocity	Capacity	Description			
_	(min)	(feet	:)	(ft/ft)	(ft/sec)	(cfs)				
	40.8	1,53	5 0	0.0280	0.63		Lag/CN Method,			
	Summary for Subcatchment 16PS:									

Runoff 25.90 cfs @ 12.73 hrs, Volume= 4.353 af, Depth= 1.29" =

ac) Cl	N Desc	cription				
90 5	8 Mea	dow, non-g	grazed, HS	SG B		
6.490 78 Meadow, non-grazed, HSG D						
20 9	6 Grav	el surface	, HSG B			
40.600 62 Weighted Average						
00	100.	00% Pervi	ous Area			
			- ··			
Length		,		Description		
(feet)	(ft/ft)	(ft/sec)	(cfs)			
1,600	0.0240	0.54		Lag/CN Method,		
	90 5 90 7 20 9 00 6 00 _ength (feet)	90 58 Mea 90 78 Mea 20 96 Grav 00 62 Weiq 00 62 Weiq 00 100. ∟ength Slope (feet) (ft/ft)	90 58 Meadow, non- <u>c</u> 90 78 Meadow, non- <u>c</u> 20 96 Gravel surface 00 62 Weighted Aver 00 100.00% Pervio _ength Slope Velocity _feet) (ft/ft) (ft/sec)	90 58 Meadow, non-grazed, HS 90 78 Meadow, non-grazed, HS 20 96 Gravel surface, HSG B 00 62 Weighted Average 00 100.00% Pervious Area Length Slope Velocity Capacity (feet) (ft/ft) (ft/sec) (cfs)		

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Summary for Subcatchment 17PS:

Runoff = 28.89 cfs @ 12.55 hrs, Volume= 4.080 af, Depth= 1.56"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 10-yr Rainfall=4.75"

 Area	(ac)	CN	Desc	cription		
18.	320	58	Mea	dow, non-g	grazed, HS	SG B
2.	610	71			grazed, HS	
9.	730	78	Mea	dow, non-g	grazed, HS	SG D
 0.	730	96	Grav	el surface	, HSG B	
31.	390	66	Weig	ghted Aver	age	
31.	390		100.	00% Pervi	ous Area	
 Tc (min)	Leng (fee		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
38.8	1,39	0 0	0.0250	0.60		Lag/CN Method,

Summary for Subcatchment 18PS:

Runoff	=	12.43 cfs @	12.42 hrs, ∖	/olume=	1.519 af, Depth= 2.50"
--------	---	-------------	--------------	---------	------------------------

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 10-yr Rainfall=4.75"

Area	(ac)	CN	Desc	cription			
7.100 78 Meadow, non-grazed, HSG D							
0	.190	96	Grav	el surface	, HSG D		
7	.290	78	Weig	ghted Aver	age		
7	.290		100.	00% Pervi	ous Area		
Tc	Leng	th	Slope	Velocity	Capacity	Description	
(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)		
33.2	85	50 (0800.0	0.43		Lag/CN Method,	

Summary for Subcatchment 19PS:

Runoff = 18.45 cfs @ 12.61 hrs, Volume= 2.701 af, Depth= 2.50"

Area (ac)) CN	Description
12.790) 78	Meadow, non-grazed, HSG D
0.170) 96	Gravel surface, HSG D
12.960) 78	Weighted Average
12.960)	100.00% Pervious Area

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Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)									
46.3 1,185 0.0070 0.43 Lag/CN Method,									
Summary for Subcatchment 20PS:									
Runoff = 56.19 cfs @ 12.41 hrs, Volume= 6.771 af, Depth= 2.59"									
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= IN-Posey 24-hr S1 10-yr Rainfall=4.75"	0.05 hrs								
Area (ac) CN Description									
2.950 58 Meadow, non-grazed, HSG B									
23.180 78 Meadow, non-grazed, HSG D									
5.270 96 Gravel surface, HSG D									
31.40079Weighted Average31.400100.00% Pervious Area									
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)									
32.4 789 0.0070 0.41 Lag/CN Method,									
Summer for Subactebrant 24DS									
Summary for Subcatchment 21PS:									
Runoff = 10.70 cfs @ 12.25 hrs, Volume= 1.103 af, Depth= 1.42"									
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 10-yr Rainfall=4.75"									
Area (ac) CN Description									
6.710 58 Meadow, non-grazed, HSG B									
2.190 78 Meadow, non-grazed, HSG D									
0.420 96 Gravel surface, HSG B									
9.320 64 Weighted Average 9.320 100.00% Pervious Area									
Tc Length Slope Velocity Capacity Description									
(min) (feet) (ft/ft) (ft/sec) (cfs)									
19.5 690 0.0360 0.59 Lag/CN Method,									

IN-Posey 24-hr S1 10-yr Rainfall=4.75"

2022-03-23 Combined Pre-Post Model

Summary for Subcatchment 22PS:

Runoff = 32.72 cfs @ 12.27 hrs, Volume= 3.257 af, Depth= 2.50"

IN-Posey 24-hr S1 10-yr Rainfall=4.75" Printed 4/4/2022 LLC Page 26

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Area (ac) CN Description					
15.380 78 Meadow, non-grazed, HSG D					
0.250 96 Gravel surface, HSG D					
15.630 78 Weighted Average					
15.630 100.00% Pervious Area					
To Length Clans Malasity Connector Description					
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)					
22.1 790 0.0160 0.60 Lag/CN Method,					
22.1 700 0.0100 0.000 Lag ren montal					
Summary for Subcatchment 23PS:					
Runoff = 150.87 cfs @ 12.46 hrs, Volume= 19.221 af, Depth= 2.25"					
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs					
IN-Posey 24-hr S1 10-yr Rainfall=4.75"					
Area (ac) CN Description					
15.700 58 Meadow, non-grazed, HSG B					
84.200 78 Meadow, non-grazed, HSG D					
2.700 96 Gravel surface, HSG D					
102.600 75 Weighted Average					
102.600 100.00% Pervious Area					
Tc Length Slope Velocity Capacity Description					
(min) (feet) (ft/ft) (ft/sec) (cfs)					
35.4 615 0.0050 0.29 Lag/CN Method,					
Summary for Subcatchment 24PS:					
-					
Runoff = 77.48 cfs @ 14.53 hrs, Volume= 29.563 af, Depth= 2.42"					

 Area	(ac)	CN	Desc	cription		
7.	500	58	Mea	dow, non-g	grazed, HS	SG B
136.	740	78	Mea	dow, non-g	grazed, HS	SG D
 2.	640	96	Grav	el surface	, HSG D	
146.	880	77	Weig	ghted Aver	age	
146.	880		100.0	00% Pervi	ous Area	
Тс	Leng	th	Slope	Velocity	Capacity	Description
 (min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)	
189.5	3,04	2 (0.0020	0.27		Lag/CN Method,
						-

2022-03-23	Combined	Pre-Post	Model	
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Summary for Subcatchment 25PS:

Runoff = 41.38 cfs @ 13.01 hrs, Volume= 8.164 af, Depth= 2.33"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 10-yr Rainfall=4.75"

 Area	(ac)	CN	Desc	cription		
5.	340	58	Mea	dow, non-g	grazed, HS	GB
36.	400	78	Mea	dow, non-g	grazed, HS	G D
 0.	290	96	Grav	el surface	, HSG D	
42.	030	76	Weig	ghted Aver	age	
42.	030		100.0	00% Pervi	ous Area	
Тс	Lengt	h	Slope	Velocity	Capacity	Description
 (min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)	
75.6	93	0 (0.0020	0.20		Lag/CN Method,
						-

Summary for Subcatchment 26PS:

Runoff = 59.73 cfs @ 12.97 hrs, Volume= 11.573 af, Depth= 2.50"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 10-yr Rainfall=4.75"

 Area	(ac)	CN	Desc	cription		
54.	190	78	Mea	dow, non-g	grazed, HS	GD
 1.	340	96	Grav	el surface	, HSG D	
55.	530	78	Weig	ghted Aver	age	
55.	530		100.	00% Pervi	ous Area	
-			~		A	
Tc	Lengt	h	Slope	Velocity	Capacity	Description
 (min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)	
 74.2	1,26	0 0	0.0030	0.28		Lag/CN Method,
						-

Summary for Subcatchment 27PS:

Runoff = 23.60 cfs @ 13.59 hrs, Volume= 6.356 af, Depth= 2.33"

Area (ac)	CN	Description
4.470	58	Meadow, non-grazed, HSG B
27.350	78	Meadow, non-grazed, HSG D
0.900	96	Gravel surface, HSG D
32.720	76	Weighted Average
32.720		100.00% Pervious Area

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s LLC Page 28
i,
nt 28PS:
45 af, Depth= 2.25"
ban= 5.00-40.00 hrs, dt= 0.05 hrs
1,

Summary for Reach 1PR:

Inflow Area =	176.340 ac,	0.00% Impervious, Inflow	Depth = 2.52" for 10-yr event
Inflow =	84.61 cfs @	14.91 hrs, Volume=	37.015 af
Outflow =	84.61 cfs @	14.91 hrs, Volume=	37.015 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs

Summary for Reach 2PR:

Inflow Are	a =	151.950 ac,	0.00% Impervious, Inflow	/ Depth = 2.26"	for 10-yr event
Inflow	=	162.74 cfs @	12.82 hrs, Volume=	28.630 af	
Outflow	=	162.74 cfs @	12.82 hrs, Volume=	28.630 af, Atte	n= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs

Summary for Reach 3PR:

Inflow Are	a =	412.580 ac,	0.00% Impervious, In	flow Depth = 2.47 "	for 10-yr event
Inflow	=	248.07 cfs @	13.44 hrs, Volume=	84.871 af	
Outflow	=	248.07 cfs @	13.44 hrs, Volume=	84.871 af, Atte	en= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs

2022-03-23 Combined Pre-Post Model

IN-Posey 24-hr S1 10-yr Rainfall=4.75"

2022-03-23 Combined Pre-Post Model Prepared by Westwood Professional Services

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Summary for Reach 4PR:

 Inflow Area =
 538.690 ac,
 0.00% Impervious, Inflow Depth =
 2.14"
 for 10-yr event

 Inflow =
 443.15 cfs @
 12.52 hrs, Volume=
 96.280 af

 Outflow =
 443.15 cfs @
 12.52 hrs, Volume=
 96.280 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs

Summary for Reach 5PR:

Inflow Are	ea =	113.580 ac,	0.00% Impervious, Inflow	Depth = 2.40"	for 10-yr event
Inflow	=	101.88 cfs @	12.99 hrs, Volume=	22.674 af	•
Outflow	=	101.88 cfs @	12.99 hrs, Volume=	22.674 af, Atte	en= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs

Summary for Reach 6PR:

Inflow Are	ea =	42.030 ac,	0.00% Impervious, Inflow	Depth = 2.33"	for 10-yr event
Inflow	=	41.38 cfs @	13.01 hrs, Volume=	8.164 af	
Outflow	=	41.38 cfs @	13.01 hrs, Volume=	8.164 af, Atte	en= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs

Summary for Subcatchment 1PS:

Runoff = 31.00 cfs @ 12.96 hrs, Volume= 6.180 af, Depth= 4.63"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 100-yr Rainfall=7.17"

_	Area	(ac)	CN	Desc	cription					
	15.	670	78	Mea	Meadow, non-grazed, HSG D					
_	0.350 96 Gravel surface, HSG D									
	16.	020	78	Weig	ghted Aver	age				
	16.	020		100.	00% Pervi	ous Area				
	Тс	Lengt		Slope	Velocity	Capacity	Description			
_	(min)	(feet	t)	(ft/ft)	(ft/sec)	(cfs)				
	74.2	1,26	0 0	0.0030	0.28		Lag/CN Method,			
		-					-			

Summary for Subcatchment 2PS:

Runoff = 54.04 cfs @ 12.88 hrs, Volume= 10.385 af, Depth= 4.63"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 100-yr Rainfall=7.17"

Area (ad	c) Cl	CN Description								
26.37	26.370 78 Meadow, non-grazed, HSG D									
0.55	50 9	6 Grav	el surface	, HSG D						
26.92	20 7	8 Weig	ghted Aver	age						
26.92	20	100.	00% Pervi	ous Area						
	ength (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
69.2	1,155	0.0030	0.28		Lag/CN Method,					
	Summary for Subastable 205									

Summary for Subcatchment 3PS:

Runoff = 221.90 cfs @ 12.75 hrs, Volume= 38.068 af, Depth= 4.19"

Area (ac)	CN	escription					
17.940	58	eadow, non-grazed, HSG B					
15.860	71	adow, non-grazed, HSG C					
72.950	78	Meadow, non-grazed, HSG D					
2.260	96	Gravel surface, HSG D					
109.010 109.010	74	Weighted Average 100.00% Pervious Area					

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Hydrocad® 10.10-3a s/n 03363 © 2	120 Hydrocad Software Solutions LLC	Page 31						
Tc Length Slope Veloci (min) (feet) (ft/ft) (ft/se								
57.4 2,502 0.0190 0.7	3 Lag/CN Method,							
	Summary for Subcatchment 4PS:							
Runoff = 120.35 cfs @ 15	13 hrs, Volume= 53.889 af, Depth= 4.63"							
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 100-yr Rainfall=7.17"								
Area (ac) CN Description								
138.240 78 Meadow, no 1.450 96 Gravel surfa	n-grazed, HSG D ce, HSG D							
139.690 78 Weighted A 139.690 100.00% Pe								
Tc Length Slope Veloci (min) (feet) (ft/ft) (ft/se) (cfs)							
231.2 2,625 0.0010 0.1	Exactly Lag/CN Method,							
	Summary for Subcatchment 5PS:							
Runoff = 37.75 cfs @ 14	42 hrs, Volume= 14.477 af, Depth= 4.74"							
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 100-yr Rainfall=7.17"								
Area (ac) CN Description								
35.500 78 Meadow, no 1.150 96 Gravel surfa	n-grazed, HSG D ce, HSG D							
36.650 79 Weighted A 36.650 100.00% Pe								
Tc Length Slope Veloci (min) (feet) (ft/ft) (ft/se								
188.1 2,106 0.0010 0.1	Exag/CN Method,							

IN-Posey 24-hr S1 100-yr Rainfall=7.17"

Summary for Subcatchment 6PS:

Runoff = 68.56 cfs @ 12.89 hrs, Volume= 13.174 af, Depth= 4.63"

IN-Posey 24-hr S1 100-yr Rainfall=7.17" Prepared by Westwood Professional Services HydroCAD® 10.10-3a s/n 03363 © 2020 HydroCAD Software Solutions LLC Printed 4/4/2022 Page 32

Area (ac) CN Description									
33.400 78 Meadow, non-grazed, HSG D									
0.750 96 Gravel surface, HSG D									
34.150 78 Weighted Average									
34.150 100.00% Pervious Area									
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)									
69.1 1,380 0.0040 0.33 Lag/CN Method,									
Summary for Subcatchment 7PS:									
Runoff = 40.32 cfs @ 12.57 hrs, Volume= 6.010 af, Depth= 4.63"									
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 100-yr Rainfall=7.17"									
Area (ac) CN Description									
2.110 71 Meadow, non-grazed, HSG C									
13.030 78 Meadow, non-grazed, HSG D									
0.440 96 Gravel surface, HSG D									
15.580 78 Weighted Average 15.580 100.00% Pervious Area									
15.580 100.00% Pervious Area									
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)									
45.0 675 0.0030 0.25 Lag/CN Method,									
Summary for Subcatchment 8PS:									
Runoff = 90.07 cfs @ 12.98 hrs, Volume= 17.867 af, Depth= 4.41"									
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 100-yr Rainfall=7.17"									
Area (ac) CN Description									
14.510 71 Meadow, non-grazed, HSG C									

_	,	(0.0)	<u>.</u>					
	14.	510	71	Mea	dow, non-g	grazed, HS	GC	
	32.	700	78	Mea	dow, non-g	grazed, HS	GD	
_	1.	420	96	Grav	el surface	, HSG D		
	48.	630	76	Weig	ghted Aver	age		
	48.	630		100.	00% Pervi	ous Area		
	Тс	Length	า S	Slope	Velocity	Capacity	Description	
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	73.5	1,783	30.	.0060	0.40		Lag/CN Method,	
							•	

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Summary for Subcatchment 9PS:

Runoff = 80.10 cfs @ 13.88 hrs, Volume= 25.627 af, Depth= 4.63"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 100-yr Rainfall=7.17"

_	Area	(ac)	CN	Dese	Description						
	65.520 78 Meadow, non-grazed, HSG D										
	0.	910	96	Grav	el surface	, HSG D					
_	66.	430	78	Weig	ghted Aver	age					
	66.	430		100.	00% Pervi	ous Area					
	Тс	Leng		Slope	Velocity	Capacity	Description				
_	(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)					
	145.6	2,27	'1	0.0020	0.26		Lag/CN Method,				

Summary for Subcatchment 10PS:

Runoff = 86.87 cfs @ 13.45 hrs, Volume=

22.380 af, Depth= 4.52"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 100-yr Rainfall=7.17"

	Area	(ac)	CN	Desc	Description						
	10.	900	71	Mea	dow, non-g	grazed, HS	SG C				
	47.	540	78	Mea	dow, non-g	grazed, HS	SG D				
_	0.	990	96	Grav	el surface	, HSG D					
59.430 77 Weighted Average						age					
	59.430 100.00%			00% Pervi	ous Area						
	Тс	Lengt	th	Slope	Velocity	Capacity	Description				
_	(min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)					
	109.2	1,96	68 (0.0030	0.30		Lag/CN Method,				
							-				

Summary for Subcatchment 11PS:

Runoff = 94.92 cfs @ 13.53 hrs, Volume= 25.743 af, Depth= 4.63"

Area (ac)	CN	Description			
5.740	71	eadow, non-grazed, HSG C			
60.260	78	eadow, non-grazed, HSG D			
0.730	96	Gravel surface, HSG D			
66.730	78	Veighted Average			
66.730		100.00% Pervious Area			

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HydroCAD® 10.10-3a	s/n 03363 © 2020	0 HydroCAD	D Software Solutions LLC	Page 34					
5	Slope Velocity (ft/ft) (ft/sec)	Capacity (cfs)	Description						
116.6 1,720 0.	.0020 0.25		Lag/CN Method,						
Summary for Subcatchment 12PS:									
Runoff = 122	.42 cfs @ 14.4	1 hrs, Volu	ume= 46.922 af, Depth= 4.63"						
	Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 100-yr Rainfall=7.17"								
Area (ac) CN	Description								
119.400 78 2.230 96	Meadow, non-g Gravel surface		G D						
121.630 78 121.630	Weighted Aver 100.00% Pervi								
5	Slope Velocity (ft/ft) (ft/sec)	Capacity (cfs)	Description						
188.0 3,125 0.	.0020 0.28		Lag/CN Method,						
	Sur	mmary fo	or Subcatchment 13PS:						
Runoff = 72	2.25 cfs @ 13.01	1 hrs, Volu	ume= 14.943 af, Depth= 4.41"						
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 100-yr Rainfall=7.17"									
Area (ac) CN	Description								
12.570 71 27.430 78	Meadow, non-g Meadow, non-g	grazed, HS							
<u>0.670 96</u> 40.670 76	Gravel surface Weighted Aver	,							
40.670 78	100.00% Pervi								
Tc Length S	Slope Velocity	Capacity	Description						

IN-Posey 24-hr S1 100-yr Rainfall=7.17"

(min) (feet) (ft/ft) (ft/sec) (cfs)

2022-03-23 Combined Pre-Post Model

78.4 1,500 0.0040 0.32 Lag/CN Method,

Summary for Subcatchment 14PS:

Runoff = 106.58 cfs @ 12.63 hrs, Volume= 16.618 af, Depth= 4.52"

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IN-Posey 24-hr S1 100-yr Rainfall=7.17" Printed 4/4/2022 HydroCAD® 10.10-3a s/n 03363 © 2020 HydroCAD Software Solutions LLC Page 35

	Area	(ac)	CN	Desc	cription		
	2.	920	58	Mea	dow, non-g	grazed, HS	GB
	39.	940	78	Mea	dow, non-g	grazed, HS	GD
	1.	270	96	6 Grav	el surface	, HSG D	
44.130 77 Weig					ghted Aver	age	
	44.130			100.	00% Pervi	ous Area	
	_					• •	
	Тс	Lengt	th	Slope	Velocity	Capacity	Description
	(min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)	
	48.4	85	52	0.0040	0.29		Lag/CN Method,
							-
					•	-	

Summary for Subcatchment 15PS:

104.19 cfs @ 12.55 hrs, Volume= 15.046 af, Depth= 3.23" Runoff =

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 100-yr Rainfall=7.17"

Area	(ac)	CN	Description					
36.	410	58	Mea	dow, non-g	grazed, HS	GB		
18.	940	78	Mea	dow, non-g	grazed, HS	G D		
0.	470	96	Grav	el surface	, HSG B			
55.	820	65	Weig	ghted Aver	age			
55.	820		100.0	00% Pervi	ous Area			
Tc	Lengt	h	Slope	Velocity	Capacity	Description		
<u>(min)</u>	(fee	t)	(ft/ft)	(ft/sec)	(cfs)			
40.8	1,53	5 0	.0280	0.63		Lag/CN Method,		
Summary for Subcatchment 16PS:								

Runoff 61.50 cfs @ 12.68 hrs, Volume= 9.902 af, Depth= 2.93" =

Area	Area (ac) CN Description					
33	33.590 58 Meadow, non-grazed, HSG B					
(6.490	78	3 Mea	dow, non-g	grazed, HS	G D
(0.520	96	6 Grav	el surface	, HSG B	
4(0.600	62	2 Weig	ghted Aver	age	
40	0.600		100.	00% Pervi	ous Area	
_						
To		·	Slope	Velocity	Capacity	Description
(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)	
49.2	1,6	00	0.0240	0.54		Lag/CN Method,

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Summary for Subcatchment 17PS:

Runoff = 62.29 cfs @ 12.52 hrs, Volume= 8.733 af, Depth= 3.34"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 100-yr Rainfall=7.17"

_	Area	(ac)	CN	Desc	cription		
	18.	320	58	Mea	dow, non-g	grazed, HS	6G B
	2.	610	71			grazed, HS	
	9.	730	78	Mea	dow, non-g	grazed, HS	SG D
_	0.	730	96	Grav	el surface	, HSG B	
	31.	390	66	Weig	ghted Aver	age	
	31.	390		100.	00% Pervi	ous Area	
	Tc (min)	Leng (fee		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	38.8	1,39	0 0	0.0250	0.60		Lag/CN Method,

Summary for Subcatchment 18PS:

Runoff = 21.93 cfs @ 12.41 hrs, Volume= 2.812 af, Depth= 4.63"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 100-yr Rainfall=7.17"

Area	(ac)	CN	Desc	cription			
7	7.100 78 Meadow, non-grazed, HSG D						
0	0.190 96 Gravel surface, HSG D						
7	.290	78	3 Weig	ghted Aver	age		
7	.290		100.	00% Pervi	ous Area		
-			0		0		
Tc	Leng	th	Slope	Velocity	Capacity	Description	
(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)		
33.2	85	50	0.0080	0.43		Lag/CN Method,	

Summary for Subcatchment 19PS:

Runoff = 32.81 cfs @ 12.60 hrs, Volume= 5.000 af, Depth= 4.63"

Area (ac)	CN	Description
12.790	78	Meadow, non-grazed, HSG D
0.170	96	Gravel surface, HSG D
12.960	78	Weighted Average
12.960		100.00% Pervious Area

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	Page 37							
Tc Length Slope Velocity Capacity Description								
(min) (feet) (ft/ft) (ft/sec) (cfs)								
46.3 1,185 0.0070 0.43 Lag/CN Method,								
Summary for Subcatchment 20PS:								
Runoff = 97.79 cfs @ 12.40 hrs, Volume= 12.404 af, De	epth= 4.74"							
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 100-yr Rainfall=7.17"								
Area (ac) CN Description								
2.950 58 Meadow, non-grazed, HSG B								
23.180 78 Meadow, non-grazed, HSG D 5.270 96 Gravel surface, HSG D								
31.400 79 Weighted Average								
31.400 100.00% Pervious Area								
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)								
32.4 789 0.0070 0.41 Lag/CN Method,								
Summary for Subcatchment 21PS	5:							
Runoff = 23.89 cfs @ 12.24 hrs, Volume= 2.432 af, De	epth= 3.13"							
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 100-yr Rainfall=7.17"								
Area (ac) CN Description								
6.710 58 Meadow, non-grazed, HSG B								
2.190 78 Meadow, non-grazed, HSG D 0.420 96 Gravel surface, HSG B								
9.320 64 Weighted Average								
9.320 04 Weighted Average 9.320 100.00% Pervious Area								
Tc Length Slope Velocity Capacity Description								
(min) (feet) (ft/ft) (ft/sec) (cfs)								
19.5 690 0.0360 0.59 Lag/CN Method,								

IN-Posey 24-hr S1 100-yr Rainfall=7.17"

2022-03-23 Combined Pre-Post Model

Summary for Subcatchment 22PS:

Runoff = 57.16 cfs @ 12.26 hrs, Volume= 6.030 af, Depth= 4.63"

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IN-Posey 24-hr S1 100-yr Rainfall=7.17" Printed 4/4/2022 HydroCAD® 10.10-3a s/n 03363 © 2020 HydroCAD Software Solutions LLC

Area (ac) CN Description							
15.380 78 Meadow, non-grazed, HSG D							
0.250 96 Gravel surface, HSG D							
15.630 78 Weighted Average							
15.630 100.00% Pervious Area							
To Length Clans Valority Conscity Description							
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)							
22.1 790 0.0160 0.60 Lag/CN Method,							
Summary for Subcatchment 23PS:							
Runoff = 278.12 cfs @ 12.45 hrs, Volume= 36.761 af, Depth= 4.30"							
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs							
IN-Posey 24-hr S1 100-yr Rainfall=7.17"							
Area (ac) CN Description							
15.700 58 Meadow, non-grazed, HSG B							
84.200 78 Meadow, non-grazed, HSG D							
2.700 96 Gravel surface, HSG D							
102.600 75 Weighted Average							
102.600 100.00% Pervious Area							
To Length Clans Valority Conscity Description							
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)							
35.4 615 0.0050 0.29 Lag/CN Method,							
Summary for Subcatchment 24PS:							
-							
Runoff = 143.60 cfs @ 14.51 hrs, Volume= 55.311 af, Depth= 4.52"							

 Area	(ac)	CN	Desc	cription		
7.	500	58	Mea	dow, non-g	grazed, HS	SG B
136.	740	78	Mea	dow, non-g	grazed, HS	SG D
 2.	640	96	Grav	el surface	, HSG D	
146.	880	77	Weig	ghted Aver	age	
146.	880		100.	00% Pervi	ous Area	
Tc	Leng	th	Slope	Velocity	Capacity	Description
 (min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)	
189.5	3,04	12 (0.0020	0.27		Lag/CN Method,
	,					- .

2022-03-23	Combined Pre-Post Mode	ł
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Summary for Subcatchment 25PS:

Runoff = 76.44 cfs @ 13.00 hrs, Volume= 15.442 af, Depth= 4.41"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 100-yr Rainfall=7.17"

 Area	a (ac) CN Description					
5.	340	58	3 Mea	dow, non-g	grazed, HS	G B
36.	400	78	3 Mea	dow, non-g	grazed, HS	SG D
 0.	290	96	6 Grav	el surface	, HSG D	
42.	030	76	6 Weig	ghted Aver	age	
42.	030		100.	00% Pervi	ous Area	
 Tc (min)	Leng (fee		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
75.6	93	30	0.0020	0.20		Lag/CN Method,

Summary for Subcatchment 26PS:

Runoff = 107.46 cfs @ 12.96 hrs, Volume= 21.422 af, Depth= 4.63"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs IN-Posey 24-hr S1 100-yr Rainfall=7.17"

	Area	(ac)	CN	Desc	cription		
	54.	190	78	Mea	dow, non-g	grazed, HS	G D
_	1.	340	96	Grav	el surface	, HSG D	
	55.	530	78	Weig	ghted Aver	age	
	55.	530		100.	00% Pervi	ous Area	
	т.	1		01	\/_l!#.	O an a site	Description
	Тс	Lengt		Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	74.2	1,26	0 C	.0030	0.28		Lag/CN Method,
							-

Summary for Subcatchment 27PS:

Runoff = 43.97 cfs @ 13.58 hrs, Volume= 12.022 af, Depth= 4.41"

Area (ac)	CN	Description
4.470	58	Meadow, non-grazed, HSG B
27.350	78	Meadow, non-grazed, HSG D
0.900	96	Gravel surface, HSG D
32.720	76	Weighted Average
32.720		100.00% Pervious Area

2022-03-23 Combined Pre-Post Model//-Prepared by Westwood Professional Services//-HydroCAD® 10.10-3a s/n 03363 © 2020 HydroCAD Software Solutions LL	Posey 24-hr S1 100-yr Rainfall=7.17" Printed 4/4/2022 C Page 40						
Tc Length Slope Velocity Capacity Description _ (min) (feet) (ft/ft) (ft/sec) (cfs)							
118.8 1,635 0.0020 0.23 Lag/CN Method,							
Summary for Subcatchment	28PS:						
Runoff = 50.77 cfs @ 12.82 hrs, Volume= 9.076 a	af, Depth= 4.30"						
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span IN-Posey 24-hr S1 100-yr Rainfall=7.17"	= 5.00-40.00 hrs, dt= 0.05 hrs						
Area (ac) CN Description							
3.970 58 Meadow, non-grazed, HSG B							
21.260 78 Meadow, non-grazed, HSG D 0.100 96 Gravel surface, HSG D							
25.33075Weighted Average25.330100.00% Pervious Area							
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)							
61.8 1,075 0.0040 0.29 Lag/CN Method,							
Summary for Reach 1PR:							
Inflow Area = 176.340 ac, 0.00% Impervious, Inflow Depth = 4.65" for 100-yr event Inflow = 154.90 cfs @ 14.88 hrs, Volume= 68.367 af Outflow = 154.90 cfs @ 14.88 hrs, Volume= 68.367 af, Atten= 0%, Lag= 0.0 min							

Routing by Stor-Ind+Trans method, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs

Summary for Reach 2PR:

Inflow Area =		151.950 ac,	0.00% Impervious, Inflow	Depth = 4.31" for 100-yr event
Inflow =	:	303.52 cfs @	12.80 hrs, Volume=	54.634 af
Outflow =	:	303.52 cfs @	12.80 hrs, Volume=	54.634 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs

Summary for Reach 3PR:

Inflow Area =		412.580 ac,	0.00% Impervious, Inflow	Depth = 4.59" for 100-yr event	
Inflow	=	457.80 cfs @	13.36 hrs, Volume=	157.724 af	
Outflow	=	457.80 cfs @	13.36 hrs, Volume=	157.724 af, Atten= 0%, Lag= 0.0 mi	in

Routing by Stor-Ind+Trans method, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs

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Summary for Reach 4PR:

 Inflow Area =
 538.690 ac,
 0.00% Impervious,
 Inflow Depth =
 4.14"
 for
 100-yr event

 Inflow =
 860.01 cfs @
 12.51 hrs,
 Volume=
 185.992 af

 Outflow =
 860.01 cfs @
 12.51 hrs,
 Volume=
 185.992 af,

Routing by Stor-Ind+Trans method, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs

Summary for Reach 5PR:

Inflow Area =		113.580 ac,	0.00% Impervious, Inf	flow Depth = 4.49 "	for 100-yr event
Inflow	=	187.26 cfs @	12.97 hrs, Volume=	42.520 af	•
Outflow	=	187.26 cfs @	12.97 hrs, Volume=	42.520 af, Atte	en= 0%, Lag= 0.0 min

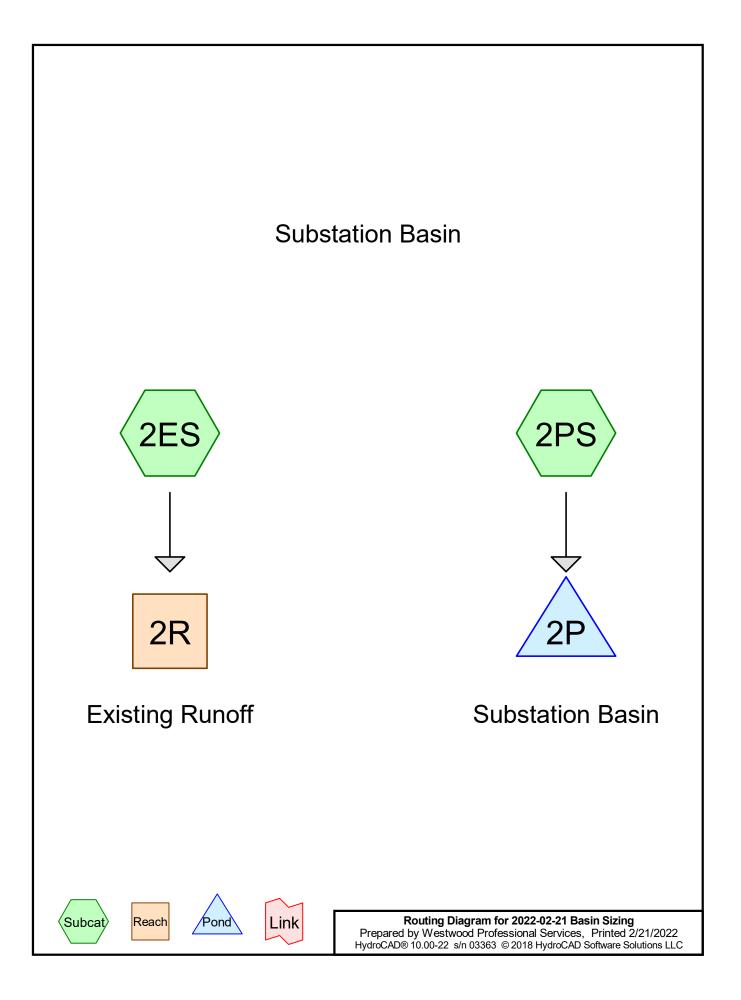
Routing by Stor-Ind+Trans method, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs

Summary for Reach 6PR:

Inflow Area =	42.030 ac,	0.00% Impervious, In	flow Depth = 4.41"	for 100-yr event
Inflow =	76.44 cfs @	13.00 hrs, Volume=	15.442 af	•
Outflow =	76.44 cfs @	13.00 hrs, Volume=	15.442 af, Atte	en= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-40.00 hrs, dt= 0.05 hrs

Appendix D Basin Sizing Calculations



Area Listing (selected nodes)

Area	CN	Description
(acres)		(subcatchment-numbers)
7.200	96	Gravel surface, HSG D (2PS)
4.600	78	Meadow, non-grazed, HSG D (2PS)
11.800	89	Row crops, straight row, Good, HSG D (2ES)

Soil Listing (selected nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
0.000	HSG A	
0.000	HSG B	
0.000	HSG C	
23.600	HSG D	2ES, 2PS
0.000	Other	

Ground Covers (selected nodes)

HSG-A	HSG-B	HSG-C	HSG-D	Other	Total	Ground	Subcatchment
 (acres)	(acres)	(acres)	(acres)	(acres)	(acres)	Cover	Numbers
 0.000	0.000	0.000	7.200	0.000	7.200	Gravel surface	2PS
0.000	0.000	0.000	4.600	0.000	4.600	Meadow, non-grazed	2PS
0.000	0.000	0.000	11.800	0.000	11.800	Row crops, straight row, Good	2ES

Summary for Subcatchment 2ES:

Runoff = 50.17 cfs @ 12.29 hrs, Volume= 5.776 af, Depth= 5.87"

Area	(ac)	CN	Desc	Description							
11	.800	89	Row	ow crops, straight row, Good, HSG D							
11	11.800 100.00% Pervious Area										
Tc (min)	Lengt (feet		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
24.7	55	0 0.	.0034	0.37		Lag/CN Method,					

Summary for Subcatchment 2PS:

Runoff = 50.17 cfs @ 12.29 hrs, Volume= 5.776 af, Depth= 5.87"

_	Area	(ac)	CN	Desc	Description						
	4.	600	78	Mea	Meadow, non-grazed, HSG D						
_	7.	7.200 96 Gravel surface, HSG D									
11.800 89 Weighted Average											
	11.	.800		100.	00% Pervi	ous Area					
	Тс	Leng	h	Slope	Velocity	Capacity	Description				
	(min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)					
	24.7	55	0	0.0034	0.37		Lag/CN Method,				
							-				

Summary for Reach 2R: Existing Runoff

Inflow Are	a =	11.800 ac,	0.00% Impervious, Infl	ow Depth = 5.87"	for 100-yr event
Inflow	=	50.17 cfs @	12.29 hrs, Volume=	5.776 af	
Outflow	=	50.17 cfs @	12.29 hrs, Volume=	5.776 af, Atte	en= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-60.00 hrs, dt= 0.05 hrs

Summary for Pond 2P: Substation Basin

Inflow Area =	11.800 ac,	0.00% Impervious, Inflow D	epth = 5.87" for 100-yr event
Inflow =	50.17 cfs @	12.29 hrs, Volume=	5.776 af
Outflow =	46.52 cfs @	12.38 hrs, Volume=	5.755 af, Atten= 7%, Lag= 5.4 min
Primary =	13.45 cfs @	12.38 hrs, Volume=	4.235 af
Secondary =	33.07 cfs @	12.38 hrs, Volume=	1.520 af

Routing by Stor-Ind method, Time Span= 0.00-60.00 hrs, dt= 0.05 hrs Peak Elev= 389.43' @ 12.38 hrs Surf.Area= 0.834 ac Storage= 1.115 af

Plug-Flow detention time= 84.7 min calculated for 5.755 af (100% of inflow) Center-of-Mass det. time= 82.3 min (884.7 - 802.4)

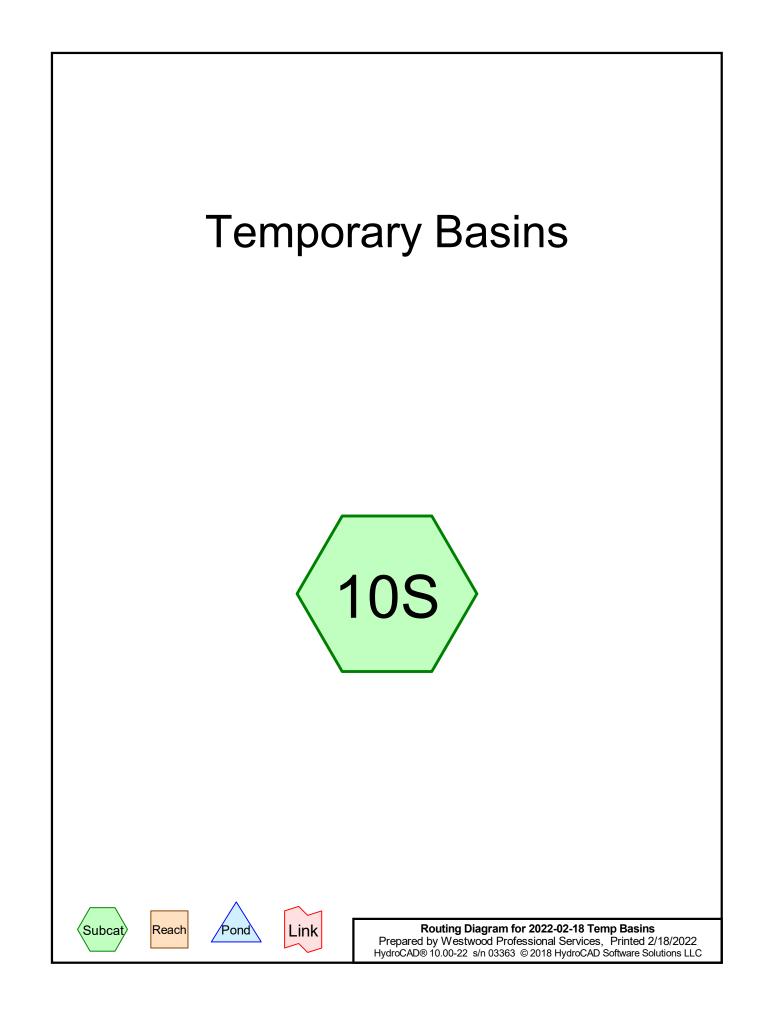
Volume	Invert A	vail.Storag	ge Storag	age Description		
#1	388.00'	1.605 a	af Custo	tom Stage Data (Prismatic) Listed below (Recalc)		
Elevatio (fee 388.0 389.0 390.0	t) (acres) 0 0.730 0 0.800	(acre	Store e-feet) 0.000 0.765 0.840	Cum.Store (acre-feet) 0.000 0.765 1.605		
Device	Routing	Invert	Outlet Dev	evices		
#1	Secondary	389.00'	40.0' long	g x 0.5' breadth Broad-Crested Rectangular Weir		
#2 Primary 388.00'		Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32 18.0'' Round Culvert X 2.00 L= 50.0' Ke= 0.500 Inlet / Outlet Invert= 388.00' / 386.75' S= 0.0250 '/' Cc= 0.900 n= 0.025, Flow Area= 1.77 sf				
Primary OutFlow Max=13.43 cfs @ 12.38 hrs HW=389.43' (Free Discharge)						

←2=Culvert (Barrel Controls 13.43 cfs @ 4.98 fps)

Secondary OutFlow Max=32.83 cfs @ 12.38 hrs HW=389.43' (Free Discharge) —1=Broad-Crested Rectangular Weir (Weir Controls 32.83 cfs @ 1.92 fps)

Stage-Area-Storage for Pond 2P: Substation Basin

Elevation	Surface	Storage	Elevation	Surface	Storage
(feet)	(acres)	(acre-feet)	(feet)	(acres)	(acre-feet)
388.00	0.730	0.000	389.08	0.806	0.829
388.02	0.731	0.015	389.10	0.808	0.845
388.04	0.733	0.029	389.12	0.810	0.862
388.06	0.734	0.023	389.14	0.811	0.878
388.08	0.736	0.059	389.16	0.813	0.894
388.10	0.737	0.073	389.18	0.814	0.910
388.12	0.738	0.088	389.20	0.816	0.927
388.14	0.740	0.103	389.22	0.818	0.943
388.16	0.741	0.118	389.24	0.819	0.959
388.18	0.743	0.133	389.26	0.821	0.976
388.20	0.744	0.147	389.28	0.822	0.992
388.22	0.745	0.162	389.30	0.824	1.009
388.24	0.747	0.177	389.32	0.826	1.025
388.26	0.748	0.192	389.34	0.827	1.042
388.28	0.750	0.207	389.36	0.829	1.058
388.30	0.751	0.222	389.38	0.830	1.075
388.32	0.752	0.222	389.40	0.832	1.091
388.34	0.754	0.252	389.42	0.834	1.108
388.36	0.755	0.267	389.44	0.835	1.125
388.38	0.757	0.282	389.46	0.837	1.141
388.40	0.758	0.298	389.48	0.838	1.158
388.42	0.759	0.313	389.50	0.840	1.175
388.44	0.761	0.328	389.52	0.842	1.192
388.46	0.762	0.343	389.54	0.843	1.209
388.48	0.764	0.358	389.56	0.845	1.226
388.50	0.765	0.374	389.58	0.846	1.242
388.52	0.766	0.389	389.60	0.848	1.259
388.54	0.768	0.404	389.62	0.850	1.276
388.56	0.769	0.420	389.64	0.851	1.293
388.58	0.771	0.435	389.66	0.853	1.310
388.60	0.772	0.451	389.68	0.854	1.327
388.62	0.772	0.466	389.70	0.856	1.345
388.64	0.775	0.482	389.72	0.858	1.362
388.66	0.776	0.497	389.74	0.859	1.379
388.68	0.778	0.513	389.76	0.861	1.396
388.70	0.779	0.528	389.78	0.862	1.413
388.72	0.780	0.544	389.80	0.864	1.431
388.74	0.782	0.559	389.82	0.866	1.448
388.76	0.783	0.575	389.84	0.867	1.465
388.78	0.785	0.591	389.86	0.869	1.483
388.80	0.786	0.606	389.88	0.870	1.500
388.82	0.787	0.622	389.90	0.872	1.517
388.84	0.789	0.638	389.92	0.874	1.535
388.86	0.790	0.654	389.94	0.875	1.552
388.88	0.792	0.670	389.96	0.877	1.570
388.90	0.793	0.685	389.98	0.878	1.587
388.92	0.794	0.701	390.00	0.880	1.605
388.94	0.796	0.717	500.00	0.000	1.000
388.96	0.797	0.733			
388.98	0.799	0.749			
389.00	0.800	0.765			
389.02	0.802	0.781			
389.04	0.803	0.797			
389.06	0.805	0.813			



Area Listing (selected nodes)

Area	CN	Description
(acres)		(subcatchment-numbers)
28.870	86	Fallow, bare soil, HSG B (10S)
12.230	94	Fallow, bare soil, HSG D (10S)

2022-02-18 Temp Basins

Soil Listing (selected nodes)

Area (acres)	Soil Group	Subcatchment Numbers
0.000	HSG A	
28.870	HSG B	10S
0.000	HSG C	
12.230	HSG D	10S
0.000	Other	

2022-02-18 Temp Basins

		,				;5)	
HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
0.000	28.870	0.000	12.230	0.000	41.100	Fallow, bare soil	10S

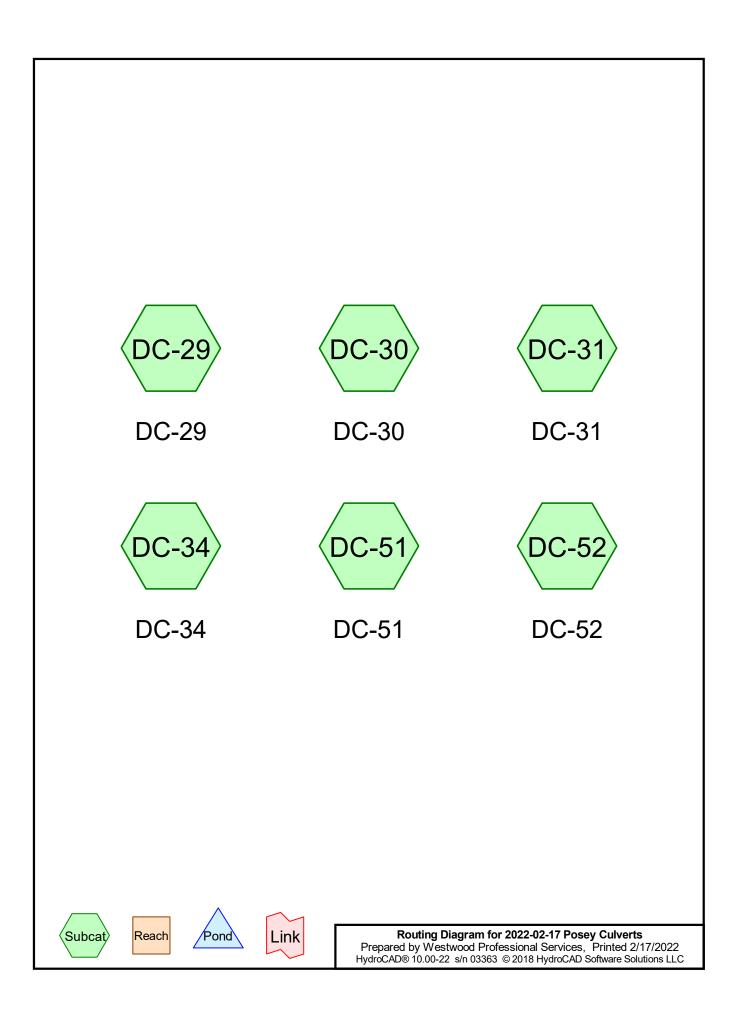
Ground Covers (selected nodes)

Summary for Subcatchment 10S:

Runoff = 89.41 cfs @ 12.47 hrs, Volume= 11.754 af, Depth> 3.43"

 Area	(ac)	CN	Desc	cription		
12.	230	94	Fallo	w, bare so	oil, HSG D	
 28.	.870	86	6 Fallo	w, bare so	oil, HSG B	
41.	100	88	Weig	ghted Aver	age	
41.	100		100.	00% Pervi	ous Area	
_			<u>.</u>		•	
Tc	Leng	th	Slope	Velocity	Capacity	Description
(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)	
 37.7	2,84	6	0.0218	1.26		Lag/CN Method,
	,					

Appendix E Culvert Sizing Calculations



Area Listing (selected nodes)

Area	CN	Description		
(acres)		(subcatchment-numbers)		
10.870	58	Meadow, HSG B (DC-29, DC-30, DC-31, DC-51, DC-52)		
156.520	78	Meadow, HSG D (DC-29, DC-30, DC-31, DC-34, DC-52)		
11.350	78	Row Crop, HSG B (DC-29, DC-30, DC-51, DC-52)		
37.920	89	Row Crop, HSG D (DC-29, DC-30, DC-31, DC-34, DC-52)		

Soil Listing (selected nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
0.000	HSG A	
22.220	HSG B	DC-29, DC-30, DC-31, DC-51, DC-52
0.000	HSG C	
194.440	HSG D	DC-29, DC-30, DC-31, DC-34, DC-52
0.000	Other	

Ground Covers (selected nodes)

 HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
 0.000	10.870	0.000	156.520	0.000	167.390	Meadow	DC-29, DC-30, DC-31,
							DC-34, DC-51, DC-52
0.000	11.350	0.000	37.920	0.000	49.270	Row Crop	DC-29, DC-30, DC-31,
							DC-34, DC-51, DC-52

Summary for Subcatchment DC-29: DC-29

Runoff = 16.07 cfs @ 14.69 hrs, Volume= 6.320 af, Depth= 2.42"

	Area ((ac)	CN	Desc	cription		
*	2.	860	58	Mea	dow, HSG	В	
*	23.	390	78	Mea	dow, HSG	D	
*	1.4	480	78	Row	Crop, HS	G B	
*	3.	670	89	Row	Crop, HS	G D	
	31.4	400	77	Weig	ghted Aver	age	
	31.4	400		100.0	00% Pervi	ous Area	
	Тс	Lengt	h	Slope	Velocity	Capacity	Description
	(min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)	
	197.4	1,50	8 0	.0006	0.13		Lag/CN Method,

Summary for Subcatchment DC-30: DC-30

Runoff = 22.22 cfs @ 13.11 hrs, Volume= 4.690 af, Depth= 2.59"

	Area	(ac)	CN	Desc	cription		
*	3.	100	58	Mea	dow, HSG	В	
*	9.	540	78	Mea	dow, HSG	D	
*	0.	930	78	8 Row	Crop, HS	G B	
*	8.	180	89	Row	Crop, HS	G D	
	21.750 79 Weighted Average					age	
	21.750 100.00% Pervious Area				00% Pervi	ous Area	
	Тс	Leng		Slope	Velocity	Capacity	Description
	(min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)	
	84.8	1,64	-0	0.0033	0.32		Lag/CN Method,

Summary for Subcatchment DC-31: DC-31

Runoff = 40.63 cfs @ 16.09 hrs, Volume= 22.543 af, Depth= 2.50"

Area (a	ic) C	N Dese	cription		
0.66	60 5	8 Mea	dow, HSG	В	
101.67	70 7	8 Mea	dow, HSG	D	
5.84	40 8	9 Row	Crop, HS	GD	
108.170 78 Weighted Average 108.170 100.00% Pervious Area					
Tc L (min)	_ength (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
307.9	3,515	0.0009	0.19		Lag/CN Method,
	0.60 101.6 5.84 108.1 108.1 Tc l (min)	0.660 5 101.670 7 5.840 8 108.170 7 108.170 Tc Length (min) (feet)	0.660 58 Mea 101.670 78 Mea <u>5.840 89 Row</u> 108.170 78 Weig 108.170 78 Ueig 108.170 100. Tc Length Slope (min) (feet) (ft/ft)	0.660 58 Meadow, HSG 101.670 78 Meadow, HSG 5.840 89 Row Crop, HSG 108.170 78 Weighted Aver 108.170 78 Weighted Aver 108.170 100.00% Pervio Tc Length Slope Velocity (min) (feet) (ft/ft) (ft/sec)	0.66058Meadow, HSG B101.67078Meadow, HSG D5.84089Row Crop, HSG D108.17078Weighted Average108.170100.00% Pervious AreaTcLengthSlopeVelocityCapacity(min)(feet)(ft/ft)

Summary for Subcatchment DC-34: DC-34

Runoff = 6.22 cfs @ 12.83 hrs, Volume= 1.103 af, Depth= 2.95"

	Area	(ac)	CN	Desc	cription					
*	2.	320	78	Mea	dow, HSG	D				
*	2.	170	89	Row	Row Crop, HSG D					
	4.490 83 Weighted Average									
	4.490 100.00% Pervious Area					ous Area				
	Тс	Lengt	h	Slope	Velocity	Capacity	Description			
	(min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)				
	64.2	1,11	4 ().0024	0.29		Lag/CN Method,			
		,								

Summary for Subcatchment DC-51: DC-51

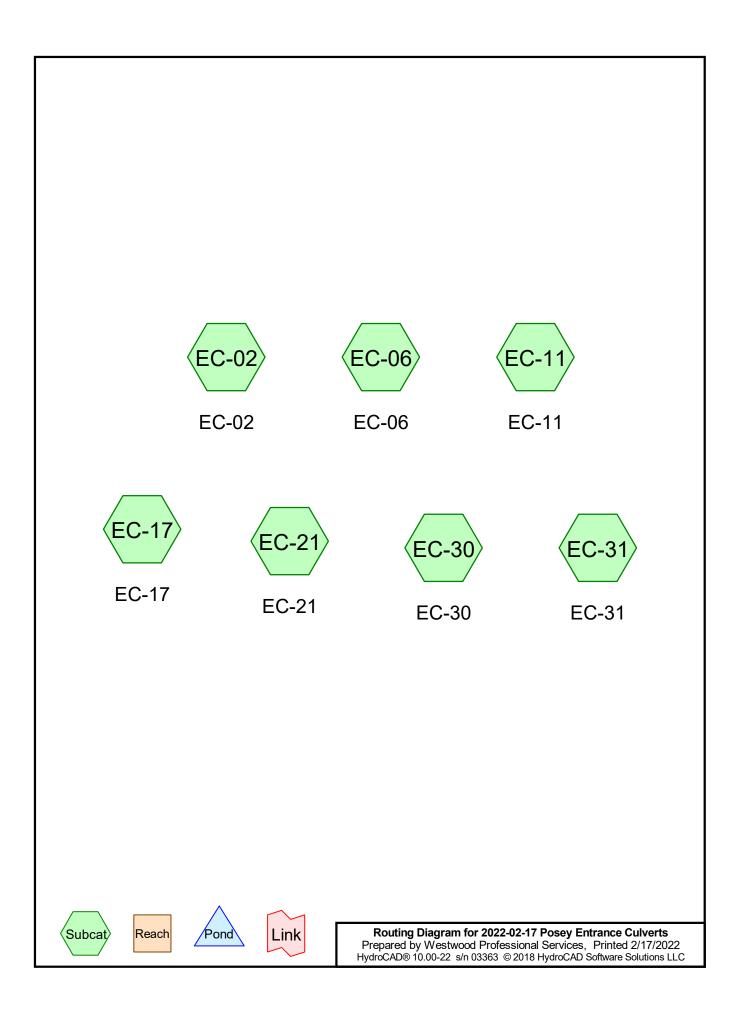
Runoff = 14.40 cfs @ 12.20 hrs, Volume= 1.299 af, Depth= 2.09"

	Area	(ac)	CN	Desc	cription		
*	1.	770	58	Mea	dow, HSG	В	
*	5.	700	78	Row	Crop, HS	G B	
	7.470 73 Weighted Average						
	7.470 100.00% Pervious Area						
	Tc	Lengt	h	Slope	Velocity	Capacity	Description
	(min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)	
	17.3	98	9 0	.0501	0.95		Lag/CN Method,

Summary for Subcatchment DC-52: DC-52

Runoff = 29.85 cfs @ 14.00 hrs, Volume= 9.997 af, Depth= 2.77"

	Area (ac)	CN	Desc	ription		
*	2.4	480	58	Mead	dow, HSG	В	
*	19.0	600	78	Mead	dow, HSG	D	
*	3.2	240	78	Row	Crop, HS	GΒ	
*	18.0	060	89	Row	Crop, HS	G D	
	43.380 81 Weighted Average					age	
	43.380 100.00% Pervious Area				00% Pervi	ous Area	
	Тс	Lengt	h	Slope	Velocity	Capacity	Description
	(min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)	
	159.0	2,27	'9 C).0014	0.24		Lag/CN Method,
		-					-



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Area Listing (selected nodes)

Area	CN	Description
(acres)		(subcatchment-numbers)
9.301	98	Impervious (EC-02, EC-06, EC-11, EC-17, EC-21, EC-30, EC-31)
11.250	58	Meadow, HSG B (EC-02, EC-21)
16.590	78	Meadow, HSG D (EC-02, EC-11, EC-21, EC-30, EC-31)
101.850	78	Row Crop, HSG B (EC-02, EC-17, EC-21)
0.850	85	Row Crop, HSG C (EC-17, EC-31)
38.970	89	Row Crop, HSG D (EC-02, EC-06, EC-11, EC-17, EC-21, EC-30)

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Soil Listing (selected nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
0.000	HSG A	
113.100	HSG B	EC-02, EC-17, EC-21
0.850	HSG C	EC-17, EC-31
55.560	HSG D	EC-02, EC-06, EC-11, EC-17, EC-21, EC-30, EC-31
9.301	Other	EC-02, EC-06, EC-11, EC-17, EC-21, EC-30, EC-31

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HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
0.000	0.000	0.000	0.000	9.301	9.301	Impervious	EC-02, EC-06, EC-11, EC-17, EC-21, EC-30, EC-31
0.000	11.250	0.000	16.590	0.000	27.840	Meadow	EC-02, EC-11, EC-21, EC-30, EC-31
0.000	101.850	0.850	38.970	0.000	141.670	Row Crop	EC-02, EC-06, EC-11, EC-17, EC-21, EC-30, EC-31

Ground Covers (selected nodes)

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Summary for Subcatchment EC-02: EC-02

Runoff = 13.47 cfs @ 12.41 hrs, Volume= 1.632 af, Depth= 2.25"

	Area (ac)	CN	Desc	cription			
*	0.283	98	3 Impe	ervious			
*	2.040	58	3 Mea	dow, HSG	В		
*	0.100	78	3 Mea	dow, HSG	D		
*	5.740	78	B Row	Crop, HS	GВ		
*	0.550	89	9 Row	Crop, HS	G D		
	8.713 75 Weighted Average						
	8.430 96.75% Pervious Area						
	0.283 3.25% Impervious Area				ous Area		
	Tc Ler	igth	Slope	Velocity	Capacity	Description	
_	<u>(min) (fe</u>	eet)	(ft/ft)	(ft/sec)	(cfs)		
	32.1 1,4	431	0.0235	0.74		Lag/CN Method,	

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Summary for Subcatchment EC-06: EC-06

Runoff = 69.14 cfs @ 12.68 hrs, Volume= 11.129 af, Depth> 3.63"

	Area	(ac)	CN	Desc	cription		
*	5.	154	98	Impe	ervious		
*	31.	590	89	Row	Crop, HS	G D	
	36.744 90 Weighted Average					age	
	31.	590		85.9	7% Pervio	us Area	
	5.154 14.03% Impervious Area					vious Area	
	_						
	Тс	Lengt		Slope	Velocity	Capacity	Description
	(min)	(feet	:)	(ft/ft)	(ft/sec)	(cfs)	
	53.2	2,77	7 0.	0090	0.87		Lag/CN Method,
		,					

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Summary for Subcatchment EC-11: EC-11

Runoff = 9.96 cfs @ 12.32 hrs, Volume= 1.075 af, Depth= 2.86"

Area (ac)	C	N Desc	cription		
0.136	9	8 Impe	ervious		
3.110	7	8 Mea	dow, HSG	D	
1.270	8	9 Row	Crop, HS	G D	
4.516	8	2 Weig	ghted Aver	age	
4.380 96.99% Pervious Area					
0.136 3.01% Impervious Area					
	ngth		,		Description
<u>min) (1</u>	eet)	(ft/ft)	(ft/sec)	(cfs)	
26.2	686	0.0071	0.44		Lag/CN Method,
<u> </u>	0.136 3.110 <u>1.270</u> 4.516 4.380 0.136 Tc Lei <u>nin) (f</u>	0.136 9 3.110 7 <u>1.270 8</u> 4.516 8 4.380 0.136 Tc Length nin) (feet)	0.136 98 Impe 3.110 78 Mea <u>1.270 89 Row</u> 4.516 82 Weig 4.380 96.9 0.136 3.01 Tc Length Slope hin) (feet) (ft/ft)	0.13698Impervious3.11078Meadow, HSG1.27089Row Crop, HS4.51682Weighted Aver4.38096.99% Pervio0.1363.01% ImpervioTcLengthSlopeVelocity(ft/ft)(feet)(ft/ft)	0.13698Impervious3.11078Meadow, HSG D1.27089Row Crop, HSG D4.51682Weighted Average4.38096.99% Pervious Area0.1363.01% Impervious AreaTcLengthSlopeVelocityCapacitynin)(feet)(ft/ft)

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Summary for Subcatchment EC-17: EC-17

Runoff = 30.99 cfs @ 12.55 hrs, Volume= 4.304 af, Depth= 2.59"

	Area (ac)	CN	Desc	ription		
*	0.5	510	98	Impe	ervious		
*	17.8	310	78	Row	Crop, HS	G B	
*	0.7	710	85	Row	Crop, HS	GC	
*	0.9	930	89	Row	Crop, HS	G D	
	19.960 79 Weighted Average						
	19.450 97.44% Pervious Area						
	0.510 2.56% Impervious Area			% Impervi	ous Area		
	Тс	Lengt		Slope	Velocity	Capacity	Description
_	(min)	(feet	:)	(ft/ft)	(ft/sec)	(cfs)	
	42.1	2,84	9 0).0324	1.13		Lag/CN Method,

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Summary for Subcatchment EC-21: EC-21

Runoff = 86.39 cfs @ 13.29 hrs, Volume= 20.366 af, Depth= 2.42"

	Area (ac)	CN	Desc	cription		
*	2.798	98	3 Impe	ervious		
*	9.210	58	3 Mea	dow, HSG	В	
*	8.730	78	3 Mea	dow, HSG	D	
*	78.300	78	B Row	Crop, HS	GΒ	
*	2.150	89	9 Row	Crop, HS	G D	
	101.188	7	7 Weig	ghted Aver	age	
	98.390		97.2	3% Pervio	us Area	
	2.798		2.77	% Impervi	ous Area	
	Tc Ler	igth	Slope	Velocity	Capacity	Description
	<u>(min) (fe</u>	eet)	(ft/ft)	(ft/sec)	(cfs)	
	98.7 3,	903	0.0110	0.66		Lag/CN Method,

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Summary for Subcatchment EC-30: EC-30

Runoff = 11.24 cfs @ 12.50 hrs, Volume= 1.497 af, Depth= 2.95"

Area (ac)	01	N Desc	cription		
0.090	9	8 Impe	ervious		
3.520	7	8 Mea	dow, HSG	D	
2.480	8	9 Row	Crop, HS	G D	
6.090	8	3 Weig	ghted Aver	age	
6.000		98.5	2% Pervio	us Area	
0.090		1.48	% Impervi	ous Area	
	ngth	Slope	,		Description
nin) (1	eet)	(ft/ft)	(ft/sec)	(cfs)	
39.2 1	,031	0.0057	0.44		Lag/CN Method,
	3.520 2.480 6.090 6.000 0.090 Tc Len hin) (1	3.520 7 2.480 8 6.090 8 6.000 0.090 Tc Length hin) (feet)	3.520 78 Mean 2.480 89 Row 6.090 83 Weig 6.000 98.55 0.090 1.48 Tc Length Slope hin) (feet) (ft/ft)	3.520 78 Meadow, HSG 2.480 89 Row Crop, HS 6.090 83 Weighted Aver 6.000 98.52% Pervio 0.090 1.48% Impervior Tc Length Slope Velocity hin) (feet) (ft/ft) (ft/sec)	3.52078Meadow, HSG D2.48089Row Crop, HSG D6.09083Weighted Average6.00098.52% Pervious Area0.0901.48% Impervious AreaTcLengthSlopeVelocityCapacity(ft/ft)(ft/sec)(cfs)

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Summary for Subcatchment EC-31: EC-31

Runoff = 4.20 cfs @ 12.22 hrs, Volume= 0.393 af, Depth= 2.95"

	Area	(ac)	CN	Desc	cription		
*	0.	330	98	Impe	ervious		
*	1.	130	78	Mea	dow, HSG	D	
*	0.	140	85	Row	Crop, HS	GC	
	1.	600	83	Weig	ghted Aver	age	
	1.	270		79.3	8% Pervio	us Area	
	0.	330		20.62	2% Imperv	vious Area	
	Тс	Lengt		Slope	Velocity	Capacity	Description
	(min)	(feet	t)	(ft/ft)	(ft/sec)	(cfs)	
	19.7	65	5 (0.0109	0.55		Lag/CN Method,

	Method: User-Specified					
Design Discha	rge	16.07 cfs	Check Discha	arge	0.00	cfs
Grades Model: I	nverts					
Invert Upstream	n	371.70 ft	Invert Downst	tream	371.50	ft
Length		44.00 ft	Slope		0.004545	ft/ft
Drop		0.20 ft				
Headwater Mod	lel: Unspecified					
	lel: Unspecified ions: Constant Tailwater					
	ions: Constant Tailwater	N/A ft				
Tailwater Condit	ions: Constant Tailwater	N/A ft Discharge	HW Elev.	Velocity		

Design:Trial-1

Solve For: Headwater Elevation

Culvert Summary					
Allowable HW Elevation	N/A	ft	Storm Event	Design	
Computed Headwater Elevation	373.44	ft	Discharge	16.07	cfs
Headwater Depth/Height	0.87		Tailwater Elevation	N/A	ft
Inlet Control HW Elev.	373.27		Control Type	Outlet Control	
Outlet Control HW Elev.	373.44	ft			
Grades					
Upstream Invert	371.70	ft	Downstream Invert	371.50	ft
Length	44.00	ft	Constructed Slope	0.004545	ft/ft
Hydraulic Profile					
Profile	M2		Depth, Downstream	1.01	ft
Slope Type	Mild		Normal Depth	1.59	ft
Flow Regime	Subcritical		Critical Depth	1.01	ft
Velocity Downstream	5.06	ft/s	Critical Slope	0.016677	ft/ft
Section					
Section Shape	Circular		Mannings Coefficient	0.024	
Section Material	CMP		Span	2.00	ft
Section Size	24 inch		Rise	2.00	ft
Number Sections	2				
Outlet Control Properties					
Outlet Control HW Elev.	373.44	ft	Upstream Velocity Head	0.19	ft
Ke	0.90		Entrance Loss	0.17	ft
Inlet Control Properties					
Inlet Control HW Elev.	373.27	ft	Flow Control	N/A	
Inlet Type	Projecting		Area Full	6.3	ft²
К	0.03400		HDS 5 Chart	2	
Μ	1.50000		HDS 5 Scale	3	
С	0.05530		Equation Form	1	
Υ	0.54000				

Peak Discharge	Method: User-Specified			
Design Discha	rge	22.22 cfs	Check Discharge	0.00 cfs
Grades Model: I	nverts			
Invert Upstream	n	370.40 ft	Invert Downstream	370.30 ft
Length		44.00 ft	Slope	0.002273 ft/ft
Drop		0.10 ft		
Headwater Mod	lel: Unspecified			
	lel: Unspecified ions: Constant Tailwater			
	ions: Constant Tailwater	N/A ft		
Tailwater Condit	ions: Constant Tailwater	N/A ft Discharge	HW Elev. Velocity	

Design:Trial-2

Solve For: Headwater Elevation

Culvert Summary					
Allowable HW Elevation	N/A	ft	Storm Event	Design	
Computed Headwater Elevation	373.36	ft	Discharge	22.22	cfs
Headwater Depth/Height	1.18		Tailwater Elevation	N/A	ft
Inlet Control HW Elev.	373.11	ft	Control Type	Outlet Control	
Outlet Control HW Elev.	373.36	ft			
Grades					
Upstream Invert	370.40	ft	Downstream Invert	370.30	ft
Length	44.00	ft	Constructed Slope	0.002273	ft/ft
Hydraulic Profile					
Profile	M2		Depth, Downstream	1.60	ft
Slope Type	Mild		Normal Depth	N/A	ft
Flow Regime	Subcritical		Critical Depth	1.60	ft
Velocity Downstream	6.68	ft/s	Critical Slope	0.018161	ft/ft
Section					
Section Shape	Circular		Mannings Coefficient	0.024	
Section Material	CMP		Span	2.50	ft
Section Size	30 inch		Rise	2.50	ft
Number Sections	1				
Outlet Control Properties					
Outlet Control HW Elev.	373.36	ft	Upstream Velocity Head	0.34	ft
Ke	0.90		Entrance Loss	0.31	ft
Inlet Control Properties					
Inlet Control HW Elev.	373.11	ft	Flow Control	N/A	
Inlet Type	Projecting		Area Full	4.9	ft²
К	0.03400		HDS 5 Chart	2	
Μ	1.50000		HDS 5 Scale	3	
С	0.05530		Equation Form	1	
Y	0.54000				

Peak Discharge	Method: User-Specified					
Design Discha	rge	40.63 cfs	Check Disch	arge	0.00	cfs
Grades Model: I	nverts					
Invert Upstream	m	372.10 ft	Invert Downs	tream	372.00	ft
Length		48.00 ft	Slope		0.002083	ft/ft
Drop		0.10 ft				
Headwater Mod	lel: Unspecified					
	lel: Unspecified tions: Constant Tailwater					
	tions: Constant Tailwater	N/A ft				
Tailwater Condit	tions: Constant Tailwater	N/A ft Discharge	HW Elev.	Velocity		

Design:Trial-1

Solve For: Headwater Elevation

Culvert Summary				
Allowable HW Elevation	N/A ft	Storm Event	Design	
Computed Headwater Elevation	374.75 ft	Discharge	40.63	cfs
Headwater Depth/Height	1.32	Tailwater Elevation	N/A	ft
Inlet Control HW Elev.	374.37 ft	Control Type	Outlet Control	
Outlet Control HW Elev.	374.75 ft			
Grades				
Upstream Invert	372.10 ft	Downstream Invert	372.00	ft
Length	48.00 ft	Constructed Slope	0.002083	ft/ft
Hydraulic Profile				
Profile CompositeM2F	PressureProfile	Depth, Downstream	1.33	ft
Slope Type	Mild	Normal Depth	N/A	ft
Flow Regime	Subcritical	Critical Depth	1.33	ft
Velocity Downstream	6.13 ft/s	Critical Slope	0.020231	ft/ft
Section				
Section Shape	Circular	Mannings Coefficient	0.024	
Section Material	CMP	Span	2.00	ft
Section Size	24 inch	Rise	2.00	ft
Number Sections	3			
Outlet Control Properties				
Outlet Control HW Elev.	374.75 ft	Upstream Velocity Head	0.29	ft
Ke	0.90	Entrance Loss	0.26	ft
Inlet Control Properties				
Inlet Control HW Elev.	374.37 ft	Flow Control	N/A	
Inlet Type	Projecting	Area Full	9.4	ft²
К	0.03400	HDS 5 Chart	2	
М	1.50000	HDS 5 Scale	3	
С	0.05530	Equation Form	1	
Y	0.54000			

Peak Discharge I	Method: User-Specified					
Design Dischar	ge	6.22 cfs	Check Disc	harge	0.00	cfs
Grades Model: In	iverts					
Invert Upstream	1	381.40 ft	Invert Dowr	nstream	381.30	ft
Length		36.00 ft	Slope		0.002778	ft/ft
Drop		0.10 ft				
	el: Unspecified					
Headwater Mode	el: Unspecified ons: Constant Tailwater					
Headwater Mode	ons: Constant Tailwater	N/A ft				
Headwater Mode	ons: Constant Tailwater		e HW Elev.	Velocity		

Design:Trial-1

Culvert Summary				
Allowable HW Elevation	N/A ft	Storm Event	Design	
Computed Headwater Elevation	383.28 ft	Discharge	6.22	cfs
Headwater Depth/Height	1.26	Tailwater Elevation	N/A	ft
Inlet Control HW Elev.	383.03 ft	Control Type	Outlet Control	
Outlet Control HW Elev.	383.28 ft			
Grades				
Upstream Invert	381.40 ft	Downstream Invert	381.30	ft
Length	36.00 ft	Constructed Slope	0.002778	ft/ft
Hydraulic Profile				
Profile CompositeM2F	PressureProfile	Depth, Downstream	0.96	ft
Slope Type	Mild	Normal Depth	N/A	ft
Flow Regime	Subcritical	Critical Depth	0.96	ft
Velocity Downstream	5.18 ft/s	Critical Slope	0.021593	ft/ft
Section				
Section Shape	Circular	Mannings Coefficient	0.024	
Section Material	CMP	Span	1.50	ft
Section Size	18 inch	Rise	1.50	ft
Number Sections	1			
Outlet Control Properties				
Outlet Control HW Elev.	383.28 ft	Upstream Velocity Head	0.19	ft
Ke	0.90	Entrance Loss	0.17	ft
Inlet Control Properties				
Inlet Control HW Elev.	383.03 ft	Flow Control	N/A	
Inlet Type	Projecting	Area Full	1.8	ft²
К	0.03400	HDS 5 Chart	2	
Μ	1.50000	HDS 5 Scale	3	
С	0.05530	Equation Form	1	
Υ	0.54000			

9	Method: User-Specified				
Design Discha	rge	14.40 cfs	Check Disch	arge	0.00 cfs
Grades Model: I	nverts				
Invert Upstrear	n	395.90 ft	Invert Down	stream	394.80 ft
Length		48.00 ft	Slope		0.022917 ft/ft
Drop		1.10 ft			
Headwater Mod	el: Unspecified				
	el: Unspecified ions: Constant Tailwater				
	ions: Constant Tailwater	N/A ft			
Tailwater Condit	ions: Constant Tailwater	N/A ft Discharge	HW Elev.	Velocity	

Design:Trial-1

Culvert Summary					
Allowable HW Elevation	N/A	ft	Storm Event	Design	
Computed Headwater Elevation	398.44	ft	Discharge	14.40	cfs
Headwater Depth/Height	1.27		Tailwater Elevation	N/A	ft
Inlet Control HW Elev.	398.26		Control Type	Entrance Control	
Outlet Control HW Elev.	398.44	ft			
Grades					
Upstream Invert	395.90	ft	Downstream Invert	394.80	ft
Length	48.00	ft	Constructed Slope	0.022917	ft/ft
Hydraulic Profile					
Profile	S2		Depth, Downstream	1.32	ft
Slope Type	Steep		Normal Depth	1.32	ft
Flow Regime	Supercritical		Critical Depth	1.37	ft
Velocity Downstream	6.52	ft/s	Critical Slope	0.020985	ft/ft
Section					
Section Shape	Circular		Mannings Coefficient	0.024	
Section Material	CMP		Span	2.00	ft
Section Size	24 inch		Rise	2.00	ft
Number Sections	1				
Outlet Control Properties					
Outlet Control HW Elev.	398.44	ft	Upstream Velocity Head	0.62	ft
Ke	0.90		Entrance Loss	0.55	ft
Inlet Control Properties					
Inlet Control HW Elev.	398.26	ft	Flow Control	N/A	
Inlet Type	Projecting		Area Full	3.1	ft²
К	0.03400		HDS 5 Chart	2	
Μ	1.50000		HDS 5 Scale	3	
С	0.05530		Equation Form	1	
Y	0.54000				

Feak Discharge	Method: User-Specified				
Design Discha	rge	29.85 cfs	Check Disc	harge	0.00 cfs
Grades Model: I	nverts				
Invert Upstream	n	370.40 ft	Invert Down	stream	369.50 ft
Length		50.00 ft	Slope		0.018000 ft/ft
Drop		0.90 ft			
Headwater Mod	lel: Unspecified				
	lel: Unspecified ions: Constant Tailwater				
	ions: Constant Tailwater	N/A ft			
Tailwater Condit	ions: Constant Tailwater	N/A ft Discharge	HW Elev.	Velocity	

Design:Trial-1

Culvert Summary					
Allowable HW Elevation	N/A	ft	Storm Event	Design	
Computed Headwater Elevation	373.87	ft	Discharge	29.85	cfs
Headwater Depth/Height	1.39		Tailwater Elevation	N/A	ft
Inlet Control HW Elev.	373.81	ft	Control Type	Outlet Control	
Outlet Control HW Elev.	373.87	ft			
Grades					
Upstream Invert	370.40	ft	Downstream Invert	369.50	ft
Length	50.00	ft	Constructed Slope	0.018000	ft/ft
Hydraulic Profile					
Profile	M2		Depth, Downstream	1.86	ft
Slope Type	Mild		Normal Depth	2.05	ft
Flow Regime	Subcritical		Critical Depth	1.86	ft
Velocity Downstream	7.61	ft/s	Critical Slope	0.022054	ft/ft
Section					
Section Shape	Circular		Mannings Coefficient	0.024	
Section Material	CMP		Span	2.50	ft
Section Size	30 inch		Rise	2.50	ft
Number Sections	1				
Outlet Control Properties					
Outlet Control HW Elev.	373.87	ft	Upstream Velocity Head	0.75	ft
Ke	0.90		Entrance Loss	0.67	ft
Inlet Control Properties					
Inlet Control HW Elev.	373.81	ft	Flow Control	N/A	
Inlet Type	Projecting		Area Full	4.9	ft²
к	0.03400		HDS 5 Chart	2	
Μ	1.50000		HDS 5 Scale	3	
С	0.05530		Equation Form	1	
Y	0.54000				

r oan Bioonaigo	Method: User-Specified					
Design Discha	rge	13.47 cfs	Check Discl	narge	0.00 c	fs
Grades Model: I	nverts					
Invert Upstream	n	389.50 ft	Invert Down	stream	389.00 f	ť
Length		94.00 ft	Slope		0.005319 f	t/ft
Drop		0.50 ft				
Headwater Mod	el: Unspecified					
	el: Unspecified ions: Constant Tailwater					
	ions: Constant Tailwater	N/A ft				
Tailwater Condit	ions: Constant Tailwater	N/A ft Discharge	HW Elev.	Velocity		

Design:Trial-1

Culvert Summary				
Allowable HW Elevation	N/A ft	Storm Event	Design	
Computed Headwater Elevation	392.28 ft	Discharge	13.47	cfs
Headwater Depth/Height	1.39	Tailwater Elevation	N/A	ft
Inlet Control HW Elev.	391.76 ft	Control Type	Outlet Control	
Outlet Control HW Elev.	392.28 ft			
Grades				
Upstream Invert	389.50 ft	Downstream Invert	389.00	ft
Length	94.00 ft	Constructed Slope	0.005319	ft/ft
Hydraulic Profile				
Profile CompositeM2F	PressureProfile	Depth, Downstream	1.32	ft
Slope Type	Mild	Normal Depth	N/A	ft
Flow Regime	Subcritical	Critical Depth	1.32	ft
Velocity Downstream	6.12 ft/s	Critical Slope	0.020159	ft/ft
Section				
Section Shape	Circular	Mannings Coefficient	0.024	
Section Material	CMP	Span	2.00	ft
Section Size	24 inch	Rise	2.00	ft
Number Sections	1			
Outlet Control Properties				
Outlet Control HW Elev.	392.28 ft	Upstream Velocity Head	0.29	ft
Ке	0.90	Entrance Loss	0.26	ft
Inlet Control Properties				
Inlet Control HW Elev.	391.76 ft	Flow Control	N/A	
Inlet Type	Projecting	Area Full	3.1	ft²
К	0.03400	HDS 5 Chart	2	
Μ	1.50000	HDS 5 Scale	3	
С	0.05530	Equation Form	1	
Y	0.54000			

	Method: User-Specified					
Design Discha	rge	69.14 cfs	Check Discha	irge	0.00	cfs
Grades Model: I	nverts					
Invert Upstrear	n	386.00 ft	Invert Downst	ream	385.60	ft
Length		92.00 ft	Slope		0.004348	ft/ft
Drop		0.40 ft				
Headwater Mod	lel: Unspecified					
	lel: Unspecified ions: Constant Tailwater					
	ions: Constant Tailwater	N/A ft				
Tailwater Condit	ions: Constant Tailwater	N/A ft Discharge	HW Elev.	Velocity		

Design:Trial-1

Culvert Summary					
Allowable HW Elevation	N/A	ft	Storm Event	Design	
Computed Headwater Elevation	390.51	ft	Discharge	69.14	cfs
Headwater Depth/Height	1.13		Tailwater Elevation	N/A	ft
Inlet Control HW Elev.	390.20	ft	Control Type	Outlet Control	
Outlet Control HW Elev.	390.51	ft			
Grades					
Upstream Invert	386.00	ft	Downstream Invert	385.60	ft
Length	92.00	ft	Constructed Slope	0.004348	ft/ft
Hydraulic Profile					
Profile	M2		Depth, Downstream	2.51	ft
Slope Type	Mild		Normal Depth	N/A	ft
Flow Regime	Subcritical		Critical Depth	2.51	ft
Velocity Downstream	8.32	ft/s	Critical Slope	0.015235	ft/ft
Section					
Section Shape	Circular		Mannings Coefficient	0.024	
Section Material	CMP		Span	4.00	ft
Section Size	48 inch		Rise	4.00	ft
Number Sections	1				
Outlet Control Properties					
Outlet Control HW Elev.	390.51	ft	Upstream Velocity Head	0.56	ft
Ke	0.90		Entrance Loss	0.50	ft
Inlet Control Properties					
Inlet Control HW Elev.	390.20	ft	Flow Control	N/A	
Inlet Type	Projecting		Area Full	12.6	ft²
К	0.03400		HDS 5 Chart	2	
Μ	1.50000		HDS 5 Scale	3	
С	0.05530		Equation Form	1	
Y	0.54000				

Peak Discharge	Method: User-Specified				
Design Discha	rge	9.96 cfs	Check Disc	harge	0.00 cfs
Grades Model: I	nverts				
Invert Upstream	n	379.10 ft	Invert Dowr	nstream	378.70 ft
Length		92.00 ft	Slope		0.004348 ft/ft
Drop		0.40 ft			
Headwater Mod	lel: Unspecified				
	lel: Unspecified tions: Constant Tailwater				
	tions: Constant Tailwater	N/A ft			
Tailwater Condit	tions: Constant Tailwater	N/A ft Discharge	HW Elev.	Velocity	

Design:Trial-1

Culvert Summary					
Allowable HW Elevation	N/A	ft	Storm Event	Design	
Computed Headwater Elevation	381.18	ft	Discharge	9.96	cfs
Headwater Depth/Height	1.04		Tailwater Elevation	N/A	ft
Inlet Control HW Elev.	380.91	ft	Control Type	Outlet Control	
Outlet Control HW Elev.	381.18	ft			
Grades					
Upstream Invert	379.10	ft	Downstream Invert	378.70	ft
Length	92.00	ft	Constructed Slope	0.004348	ft/ft
Hydraulic Profile					
Profile	M2		Depth, Downstream	1.13	ft
Slope Type	Mild		Normal Depth	N/A	ft
Flow Regime	Subcritical		Critical Depth	1.13	ft
Velocity Downstream	5.45	ft/s	Critical Slope	0.017707	ft/ft
Section					
Section Shape	Circular		Mannings Coefficient	0.024	
Section Material	CMP		Span	2.00	ft
Section Size	24 inch		Rise	2.00	ft
Number Sections	1				
Outlet Control Properties					
Outlet Control HW Elev.	381.18	ft	Upstream Velocity Head	0.19	ft
Ke	0.90		Entrance Loss	0.17	ft
Inlet Control Properties					
Inlet Control HW Elev.	380.91	ft	Flow Control	N/A	
Inlet Type	Projecting		Area Full	3.1	ft²
к	0.03400		HDS 5 Chart	2	
Μ	1.50000		HDS 5 Scale	3	
С	0.05530		Equation Form	1	
Y	0.54000				

r car bisonarge	Method: User-Specified					
Design Discha	rge	30.99 cfs	Check Disch	narge	0.00	cfs
Grades Model: I	nverts					
Invert Upstrea	m	379.20 ft	Invert Down	stream	378.70	ft
Length		82.00 ft	Slope		0.006098	ft/ft
Drop		0.50 ft				
Headwater Moc	lel: Unspecified					
	lel: Unspecified tions: Constant Tailwater					
	tions: Constant Tailwater	N/A ft				
Tailwater Condi	tions: Constant Tailwater	N/A ft Discharge	HW Elev.	Velocity		

Design:Trial-1

Culvert Summary					
Allowable HW Elevation	N/A	ft	Storm Event	Design	
Computed Headwater Elevation	381.48	ft	Discharge	30.99	cfs
Headwater Depth/Height	0.91		Tailwater Elevation	N/A	ft
Inlet Control HW Elev.	381.29	ft	Control Type	Outlet Control	
Outlet Control HW Elev.	381.48	ft			
Grades					
Upstream Invert	379.20	ft	Downstream Invert	378.70	ft
Length	82.00	ft	Constructed Slope	0.006098	ft/ft
Hydraulic Profile					
Profile	M2		Depth, Downstream	1.33	ft
Slope Type	Mild		Normal Depth	1.84	ft
Flow Regime	Subcritical		Critical Depth	1.33	ft
Velocity Downstream	5.85	ft/s	Critical Slope	0.015869	ft/ft
Section					
Section Shape	Circular		Mannings Coefficient	0.024	
Section Material	CMP		Span	2.50	ft
Section Size	30 inch		Rise	2.50	ft
Number Sections	2				
Outlet Control Properties					
Outlet Control HW Elev.	381.48	ft	Upstream Velocity Head	0.27	ft
Ke	0.90		Entrance Loss	0.25	ft
Inlet Control Properties					
Inlet Control HW Elev.	381.29	ft	Flow Control	N/A	
Inlet Type	Projecting		Area Full	9.8	ft²
К	0.03400		HDS 5 Chart	2	
Μ	1.50000		HDS 5 Scale	3	
С	0.05530		Equation Form	1	
Y	0.54000				

Peak Discharge	Method: User-Specified					
Design Discha	rge	86.39 cfs	Check Disch	narge	0.00	cfs
Grades Model:	nverts					
Invert Upstrea	m	388.50 ft	Invert Down	stream	388.30	ft
Length		84.00 ft	Slope		0.002381	ft/ft
Drop		0.20 ft				
Headwater Moo	lel: Unspecified					
	lel: Unspecified tions: Constant Tailwater					
	tions: Constant Tailwater	N/A ft				
Tailwater Condi	tions: Constant Tailwater	N/A ft Discharge	HW Elev.	Velocity		

Design:Trial-1

Culvert Summary					
Allowable HW Elevation	N/A	ft	Storm Event	Design	
Computed Headwater Elevation	392.19	ft	Discharge	86.39	cfs
Headwater Depth/Height	1.05		Tailwater Elevation	N/A	ft
Inlet Control HW Elev.	391.83		Control Type	Outlet Control	
Outlet Control HW Elev.	392.19	ft			
Grades					
Upstream Invert	388.50	ft	Downstream Invert	388.30	ft
Length	84.00	ft	Constructed Slope	0.002381	ft/ft
Hydraulic Profile					
Profile	M2		Depth, Downstream	2.05	ft
Slope Type	Mild		Normal Depth	N/A	ft
Flow Regime	Subcritical		Critical Depth	2.05	ft
Velocity Downstream	7.39	ft/s	Critical Slope	0.015044	ft/ft
Section					
Section Shape	Circular		Mannings Coefficient	0.024	
Section Material	CMP		Span	3.50	ft
Section Size	42 inch		Rise	3.50	ft
Number Sections	2				
Outlet Control Properties					
Outlet Control HW Elev.	392.19	ft	Upstream Velocity Head	0.38	ft
Ke	0.90		Entrance Loss	0.35	ft
Inlet Control Properties					
Inlet Control HW Elev.	391.83	ft	Flow Control	N/A	
Inlet Type	Projecting		Area Full	19.2	ft²
К	0.03400		HDS 5 Chart	2	
Μ	1.50000		HDS 5 Scale	3	
С	0.05530		Equation Form	1	
Y	0.54000				

Peak Discharge	Method: User-Specified					
Design Discha	rge	11.24 cfs	Check Disch	arge	0.00	cfs
Grades Model: I	nverts					
Invert Upstream	n	377.20 ft	Invert Down	stream	376.70	ft
Length		90.00 ft	Slope		0.005556	ft/ft
Drop		0.50 ft				
Headwater Moc	lel: Unspecified					
	lel: Unspecified					
	ions: Constant Tailwater	N/A ft				
Tailwater Condit	ions: Constant Tailwater	N/A ft Discharge	HW Elev.	Velocity		

Design:Trial-1

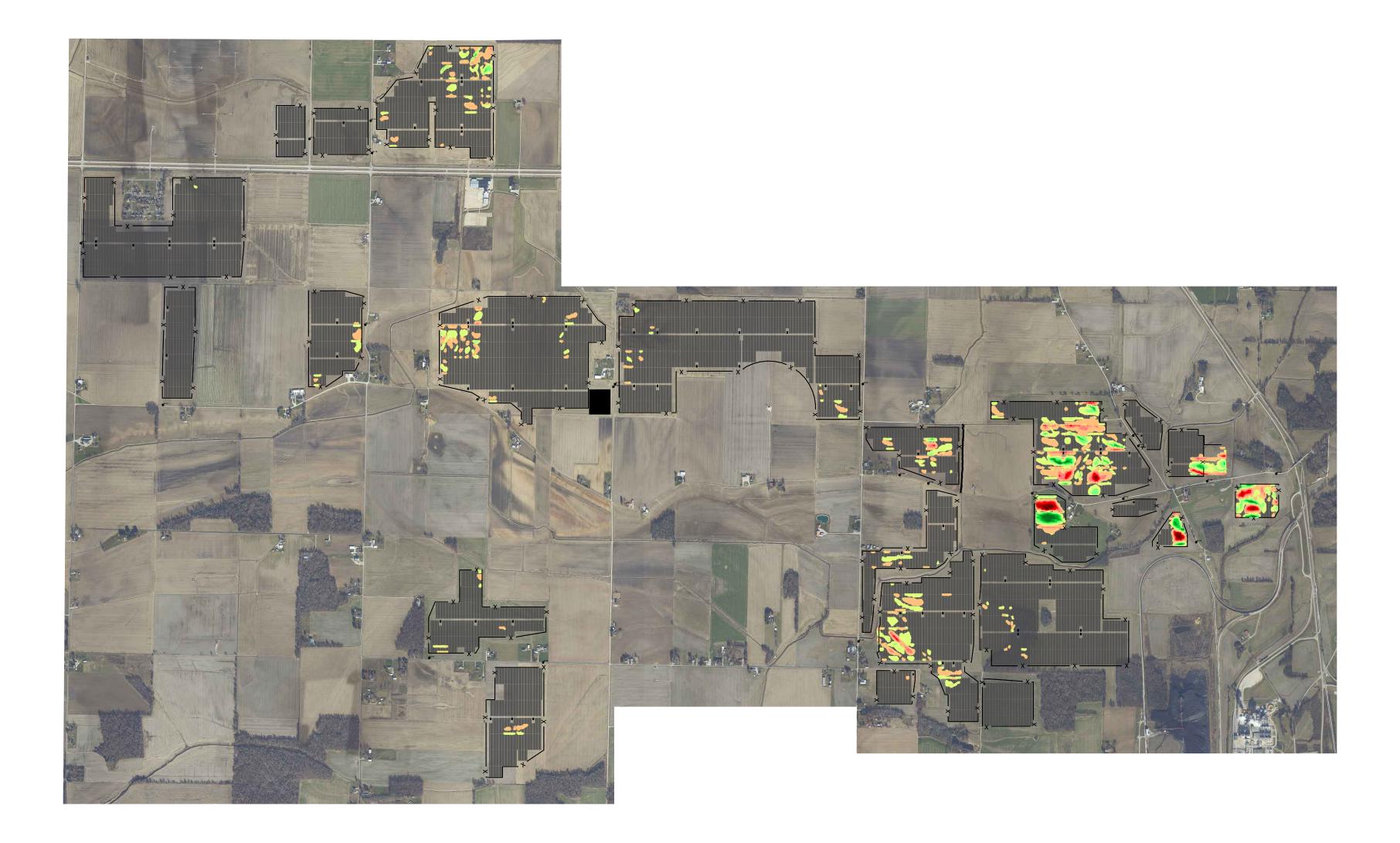
Culvert Summary					
Allowable HW Elevation	N/A	ft	Storm Event	Design	
Computed Headwater Elevation	379.42	ft	Discharge	11.24	cfs
Headwater Depth/Height	1.11		Tailwater Elevation	N/A	ft
Inlet Control HW Elev.	379.17	ft	Control Type	Outlet Control	
Outlet Control HW Elev.	379.42	ft			
Grades					
Upstream Invert	377.20	ft	Downstream Invert	376.70	ft
Length	90.00	ft	Constructed Slope	0.005556	ft/ft
Hydraulic Profile					
Profile	M2		Depth, Downstream	1.20	ft
Slope Type	Mild		Normal Depth	N/A	ft
Flow Regime	Subcritical		Critical Depth	1.20	ft
Velocity Downstream	5.70	ft/s	Critical Slope	0.018515	ft/ft
Section					
Section Shape	Circular		Mannings Coefficient	0.024	
Section Material	CMP		Span	2.00	ft
Section Size	24 inch		Rise	2.00	ft
Number Sections	1				
Outlet Control Properties					
Outlet Control HW Elev.	379.42	ft	Upstream Velocity Head	0.22	ft
Ke	0.90		Entrance Loss	0.20	ft
Inlet Control Properties					
Inlet Control HW Elev.	379.17	ft	Flow Control	N/A	
Inlet Type	Projecting		Area Full	3.1	ft²
К	0.03400		HDS 5 Chart	2	
Μ	1.50000		HDS 5 Scale	3	
С	0.05530		Equation Form	1	
Y	0.54000				

reak Discharge	Method: User-Specified					
Design Discha	rge	4.20 cfs	Check Disch	arge	0.00	cfs
Grades Model: I	nverts					
Invert Upstream	n	377.00 ft	Invert Downs	stream	376.70	ft
Length		86.00 ft	Slope		0.003488	ft/ft
Drop		0.30 ft				
Headwater Moc	lel: Unspecified					
	lel: Unspecified tions: Constant Tailwater					
	ions: Constant Tailwater	N/A ft				
Tailwater Condit	ions: Constant Tailwater	N/A ft Discharge	HW Elev.	Velocity		

Design:Trial-1

Culvert Summary					
Allowable HW Elevation	N/A	ft	Storm Event	Design	
Computed Headwater Elevation	378.48	ft	Discharge	4.20	cfs
Headwater Depth/Height	0.99		Tailwater Elevation	N/A	ft
Inlet Control HW Elev.	378.23	ft	Control Type	Outlet Control	
Outlet Control HW Elev.	378.48	ft			
Grades					
Upstream Invert	377.00	ft	Downstream Invert	376.70	ft
Length	86.00	ft	Constructed Slope	0.003488	ft/ft
Hydraulic Profile					
Profile	M2		Depth, Downstream	0.79	ft
Slope Type	Mild		Normal Depth	N/A	ft
Flow Regime	Subcritical		Critical Depth	0.79	ft
Velocity Downstream	4.48	ft/s	Critical Slope	0.018675	ft/ft
Section					
Section Shape	Circular		Mannings Coefficient	0.024	
Section Material	CMP		Span	1.50	ft
Section Size	18 inch		Rise	1.50	ft
Number Sections	1				
Outlet Control Properties					
Outlet Control HW Elev.	378.48	ft	Upstream Velocity Head	0.11	ft
Ke	0.90		Entrance Loss	0.10	ft
Inlet Control Properties					
Inlet Control HW Elev.	378.23	ft	Flow Control	N/A	
Inlet Type	Projecting		Area Full	1.8	ft²
к	0.03400		HDS 5 Chart	2	
Μ	1.50000		HDS 5 Scale	3	
С	0.05530		Equation Form	1	
Y	0.54000				

Appendix F Conceptual Array Grading



LEGEND:

 ×
 PROPOSED FENCE

CONCEPTUAL CUT/FILL VOLUME COMPARISON



PROJECT QUANTITIES	
ITEM	QUANTITY
Solar Array Cut Volume	297,000 CY
Solar Array Fill Volume	269,000 CY

 FOR ESTIMATING ONLY, ENGINEER MAKES NO GUARANTEE OF ACCURACY
 CUT/FILL QUANTITIES LISTED ARE RAW QUANTITIES AND DO NOT ACCOUNT FOR SITE EXCAVATIONS OR ANY UTILITY EXCAVATIONS. ACCOUNT FOR SITE EXCAVATIONS OR ANY UTILITY EXCAVATIONS. THE QUANTITIES SHOWN WERE DETERMINED USING THE ANTICIPATED GRADES AS SHOWN ON THE SITE PLAN AND ARE FOR ESTIMATING PURPOSES ONLY.
IN THE EVENT OF EXCESS CUT, QUANTITY TO BE SPREAD EVENLY OVER SITE.
PROJECT COORDINATE SYSTEM IS NSRS 2011 INDIANA STATE PLANES, WEST ZONE, US FOOT

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Westwood Professional Services, Inc.

Middleton, WI 53562 westwoodps.com

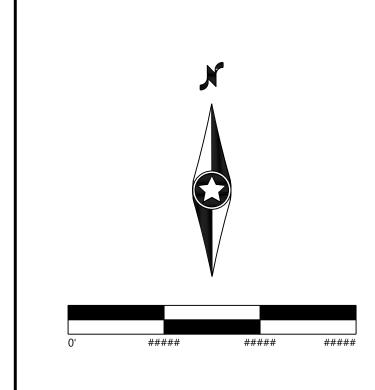
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8800 N. Gainey Center Dr. Suite 250 Scottsdale, AZ 85258

REVI	sions:	
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A	10/11/2021	Conceptual Grading Analysis



Posey Solar Project Posey County, Indiana

Conceptual Array Grading - 1' Reveal Window

CONCEPTUAL NOT FOR CONSTRUCTION

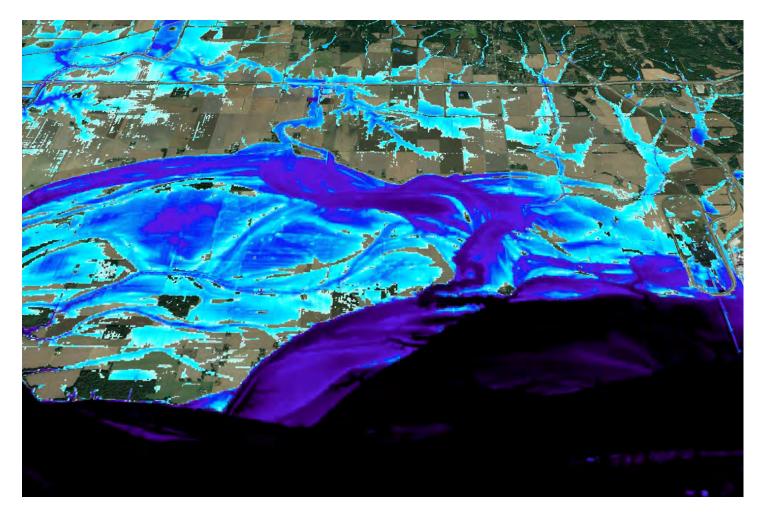
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Appendix G Preliminary Hydrology Study



PRELIMINARY HYDROLOGY STUDY

Posey Solar Project

Posey County, Indiana

JULY 2020

UPDATED APRIL 2021

PREPARED FOR:



PREPARED BY:



Preliminary Hydrology Study

Posey Solar Project

Posey County, Indiana

Prepared For:

Tenaska 14302 FNB Parkway Omaha, NE 68154 Prepared By:

Westwood 12701 Whitewater Drive, Suite 300 Minnetonka, MN 55343 (952) 937-5150

Project Number: R0027286.00 Date: July 24, 2020 Updated: April 14, 2021



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Appendices

- Appendix A: NOAA Atlas 14 Precipitation Data
- Appendix B: Curve Number Table
- Appendix C: FEMA Flood Insurance Rate Map (FIRM)
- Appendix D: FEMA FIS Report Excerpts
- Appendix E: USGS StreamStats Station Report

Executive Summary

The purpose of the study is to analyze and review the existing hydrology of the Posey Solar Project ("the project") and any impacts that the hydrology may play in the design of the proposed solar array. This report was prepared to be used by the project team in the design and layout of the project and not intended for submittal to reviewing agencies for stormwater permitting.

The project site is proposed on approximately 4,100 acres and is located roughly 4 miles east of the city of Mt Vernon in Posey County, Indiana. The site is located on relatively flat land with a few areas of steeper slopes located within the northern parcels. The total combined modeled watershed area encompasses approximately 214 square miles and generally flows to the west.

FEMA has completed a study to determine flood hazards for the selected location; the project area contains FEMA Zone A and Zone AE areas. No preliminary or pending FEMA data was located that will affect the project area. The Indiana DNR Best Available Flood Zones were also reviewed for the project area. Indiana DNR Zone A and AE areas are present within areas of the project parcels.

The majority of the proposed solar facility will consist of above ground mounted solar modules. A meadow grass will be planted below the modules and will make up a majority of the site's land cover. A small amount of impervious surface will also be added from the gravel access roads and electrical equipment pads, but typically only makes up approximately 3-5% of the total project area.

The hydrologic modeling in this report was created using FLO-2D modeling software. FLO-2D was used to review the overall watershed drainage to and through the project to determine if any overland runoff causes flooding, high velocity, or scour impacts to the site.

The analysis shows a range of water depths and velocities ranging from low to high (Exhibits 6 through 7A) across the site. Higher flood depths and velocities exist within and adjacent to the Ohio River floodplain. There are also scattered low-lying areas with localized ponding and flowpaths that contain low velocities but have flood depths greater than 0.5 feet. Minimal velocities and scour are expected on site due to the flat terrain with the exception of a few areas within the Ohio River floodplain. Based on experience with similar projects, the majority of the northern portion of the site is suitable for the planned development by avoiding or designing to areas of high flood depths.

Data Sources

TABLE 1: DATA SOURCES

Task	Format	Source	Use
Elevation	2-meter DEM	USDA Data Gateway KyGovMaps	FLO-2D Model Elevations
Elevation	5-foot DEM	Indiana Spatial Data Portal	FLO-2D Model Elevations
Crop Data	Shapefile	USDA 2013 Crop Data Layer	Landcover
Soils	Shapefile	USGS SSURGO Dataset	Curve Numbers
Precipitation	PDF File	NOAA Atlas 14	Design Storms
HUC-12 Drainage Boundary	Shapefile	USGS	Define Model Extents
Site Boundary	Posey Hydrology Study Area.kmz	Tenaska	Define Model Extents
2014 Aerial Photography	ArcGIS Map Service	USDA FSA	Reference
FEMA Flood Zones	PDF; Shapefile	FEMA	Reference
Indiana DNR Flood Zone	Shapefile	Indiana DNR	Reference
Culvert Locating and Sizing	Aerial Imagery	Google Earth	Culvert Modeling

Existing Conditions

The project area is located roughly 4 miles east of the city of Mt Vernon in Posey County, Indiana. The project site is approximately 4,100 acres and is mostly located on relatively flat terrain.

Watershed Hydrology

The total combined modeled watershed area encompasses approximately 214 square miles that generally slopes to the south and west. The Ohio River flows west to the south of the project area. According to the FEMA FIS Report for Posey County, the Ohio River rises slowly and remains out of the banks for a considerable time during flood events. Little Creek flows west and then northwest to the north of the project area before emptying into Big Creek northwest of the project.

Onsite Conditions

The project is located within and to the north of the Ohio River floodplain, with numerous creeks and ditches crossing the project that generally flow south and west. In general, the site is relatively flat, although there are some areas of steeper slopes up to around 15% within a few of the northern and eastern parcels. The land cover on the project area is primarily agricultural fields consisting of corn, winter wheat and soybeans (Exhibit 4) and has soils that belong to Hydrologic Soil Groups B, B/D, C and C/D (Exhibit 3). Areas with B soils will drain more quickly than areas with C and D soils reducing the duration of ponding. The main potential hydrologic issues on site are flooding and erosive velocities.

FEMA Flood Zones

FEMA has completed a study to determine flood hazards for the selected location; the project area is covered by panels 18129C00230C, 18129C0235C, 18129C0240C, 18129C0245C, and 18129C0265C (Appendix C). The southern portion of the site contains FEMA Zone AE areas within the Ohio River floodplain. A small northeastern corner of the northern parcels contains a FEMA Flood Zone A area where the Little Creek floodplain encroaches upon the site. A FEMA Zone A area also borders a parcel along the western edge of the project. A FEMA Floodway is present to the south of the project parcels. A FEMA Zone A flood hazard is a 100-year flood hazard with no base flood elevation determined while a Zone AE has a base flood elevation determined. No preliminary or pending FEMA changes are proposed within the project area.

Indiana Best Available Floodplain

The Indiana DNR Division of Water has published additional floodplain information of Zone A quality to include additional stream reaches not provided by FEMA on the Flood Insurance Rate Maps. This data is shown on Exhibits 2 and 6 along with the FEMA data. In addition to areas covered by FEMA, Indiana DNR Best Available Zone A areas are present through portions of the central and northwestern project parcels.

Proposed Conditions

The majority of the proposed solar facility will consist of above ground mounted solar modules. Meadow grass will be planted below the modules and will make up a majority of the land cover. A small amount of impervious surface will be added from the gravel access roads and electrical equipment pads. See the Preliminary Stormwater Management Report completed by Westwood for more information regarding the proposed stormwater management for the project.

FLO-2D Modeling

FLO-2D is a physical process model that routes rainfall runoff and flood hydrographs over flow surfaces or in channels using the dynamic wave approximation to the momentum equation. FLO-2D offers advantages over 1-D models and unit hydrograph methods by allowing for breakout flows and visualization of flows across a potential site. The primary inputs are a DTM (elevation data), curve numbers and precipitation. Major culverts impacting the site were modeled based on aerial imagery provided by Google Earth (Exhibits 6-8).

Because of the size of the modeled watershed, complex flowpaths through the project and the proximity to the Ohio River, two FLO-2D models with 50' grid cells were utilized to determine flow depths and velocities throughout the site and surrounding watersheds. A northern model was developed to model the Big Creek watershed and its impact to the site. The southern model incorporated the Ohio River and its impact to the site. The results of the two models were combined to provide one output of flood depths and velocities.

Elevation Data

The elevation data input into the FLO-2D models was 2-meter DEM data from the USDA Geospatial Data Gateway and KyGovMaps for the southern model, and 5-foot DEM data from the Indiana Spatial Data Portal for the northern model (Exhibit 5). The channel bottom of the Ohio River was created using elevation values taken from the FIS Report (Appendix D). These elevations were combined and incorporated into the DTM using the export to xyz function in Global Mapper. These XYZ files are read directly into FLO-2D.

Watershed Soils and Land Cover

USDA-NRCS SSURGO soil data provides soil types within the project boundary and full coverage of the contributing watershed. Soils are classified as Hydrologic Soil Group B, B/D, C and C/D in the project boundary (Exhibit 3). Land cover was obtained from the USDA 2013 Crop Data Layer. Exhibit 4 displays the land cover classes for the entire watershed. Curve numbers were applied to each grid cell in the FLO-2D model based on intersecting the grid with the curve numbers (Exhibit 5).

Precipitation

Precipitation data was downloaded from the NOAA Atlas 14 (Appendix A) and used for the FLO-2D analysis for the 100-year, 24-hour storm. Using the 100-year rainfall depth of 7.17 inches for this location allows for the best initial analysis in order to determine the worst areas of flooding and erosion. Rainfall inputs were distributed based on a nested Atlas 14 distribution pattern.

Inflows

The Ohio River enters the modeled watershed from the east and flows west to the south of the project area. The Flood Insurance Study for Posey County, Indiana reports a 100-year peak flood of 920,000 cfs (Appendix D). Big Creek enters the modeled watershed from the northeast and flows southwest to the north of the project area. USGS streamgage station 03378550 near Wadesville, Indiana reports a 100-year peak flood of 10,700 cfs (Appendix E). Inflow hydrographs were created using this data and added to the FLO-2D models at the points where the inflows enter the watershed (Exhibit 6).

Model confirmation

In order to verify the FLO-2D model results a HECRAS 1-D model was created for the Ohio River. The provided FLO-2d results were compared against the HECRAS model and were deemed accurate for onsite conditions in a 100-year storm event.

Flood Analysis Results

Existing Conditions Flood Analysis

The analysis shows a range of water depths and velocities ranging from low to high (Exhibits 6 through 7A) across the site. During a 100-year storm, the flood depths across the project area range from less than 0.5 feet to greater than 15 feet with velocities ranging from less than 1 foot/second to greater than 3 feet/second. See Exhibits 6 through 7A for areas within the project with higher flood depths and velocities. Overall, the FLO-2D results matched well with the flood extents provided by FEMA. Some of the results, particularly within and adjacent to the Ohio River, vary from the flood extents provided by FEMA. This can be attributed to different availability of elevation data as well as the flat nature of the terrain within the floodplain. Minimal scour is expected outside of the Ohio River floodplain (Exhibit 8). The scour depths calculated for this project consist of local scour only and are based on unarmored soils and pile bases in order to provide the most conservative local scour results.

Recommendations

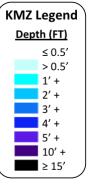
Based on experience on similar projects, portions of the site are suitable for the planned development and hydrologic concerns can be addressed by either avoiding areas of high flood depths or through detailed engineering design.

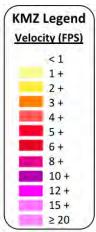
Next Steps

- 1. Final engineering design should account for the flood depths and velocities presented in Exhibits 6-7.
- 2. Facilities to be elevated 1' above the 100-year, 24-hour peak flood elevations.
- 3. Proposed facilities should avoid FEMA Flood Zones located onsite.
- 4. Stormwater management should be revisited to ensure the final design meets the local and state requirements.

Included Output Files

- 1. Shapefile of 100-year Rain Event Flow Depth 2021-04-08_Posey_PrelimFlowDepthatCell_100yr.shp Attribute "ID" = Grid Cell Number Attribute "VAR" = Max Flow Depth (Feet)
- 2. KMZ of 100-year Rain Event Flow Depth 2021-04-08_Posey _PrelimFlowDepth_100yr.kmz Overlay in Google Earth for graphical representation.
- 3. Shapefile of 100-year Rain Event Velocity 2021-04-08_Posey_PrelimVelocityatCell_100yr.shp Attribute "ID" = Grid Cell Number Attribute "VAR" = Velocity (FPS)
- 4. KMZ of 100-year Rain Event Velocity 2021-04-08_Posey _PrelimVelocity_100yr.kmz Overlay in Google Earth for graphical representation.





References Cited

National Engineering Handbook, Part 630 Hydrology. Chapter 9 Hydrologic Soil-Cover Complexes. USDA. NRCS. 210-VI-NEH, July 2004

Indiana Spatial Data Portal, 5-foot DEM, Elevation data, Accessed April 2021, https://gis.iu.edu/downloads/index.html

USDA Geospatial Data Gateway, 2-meter DEM, Elevation data, Accessed July 2020, https://datagateway.nrcs.usda.gov/

KyGovMaps, 2-meter DEM, Elevation data, Accessed July 2020, https://kygeonet.ky.gov/govmaps/

Web soil survey. Retrieved July 2020, from https://websoilsurvey.sc.egov.usda.gov/App/WebSoilSurvey.aspx

NOAA Atlas 14 Point Precipitation Frequency Estimates. Retrieved July 2020 from https://hdsc.nws.noaa.gov/hdsc/pfds/

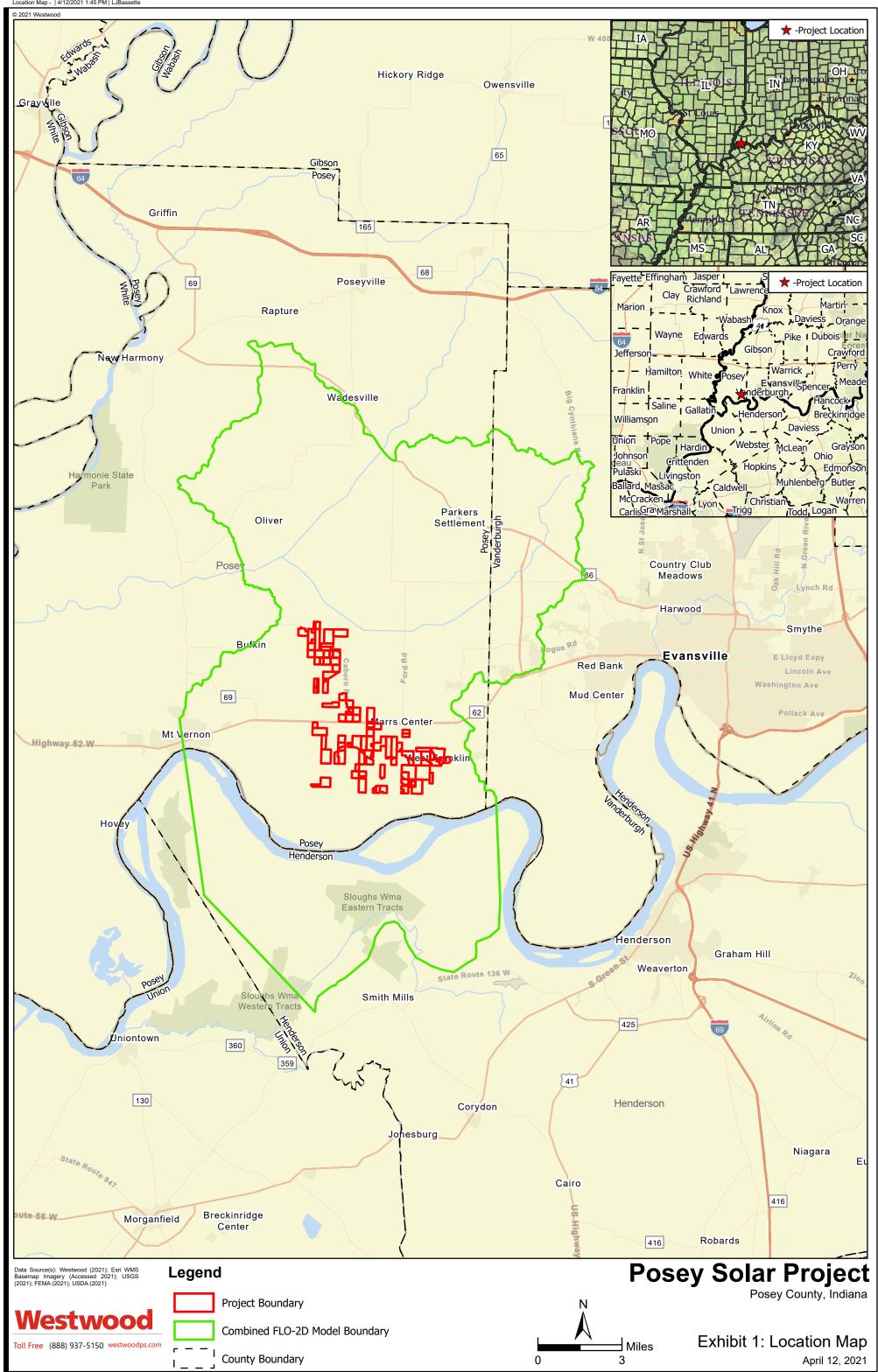
USGS. USGS water resources: About USGS water resources. Retrieved July 2020, from https://water.usgs.gov/GIS/huc.html

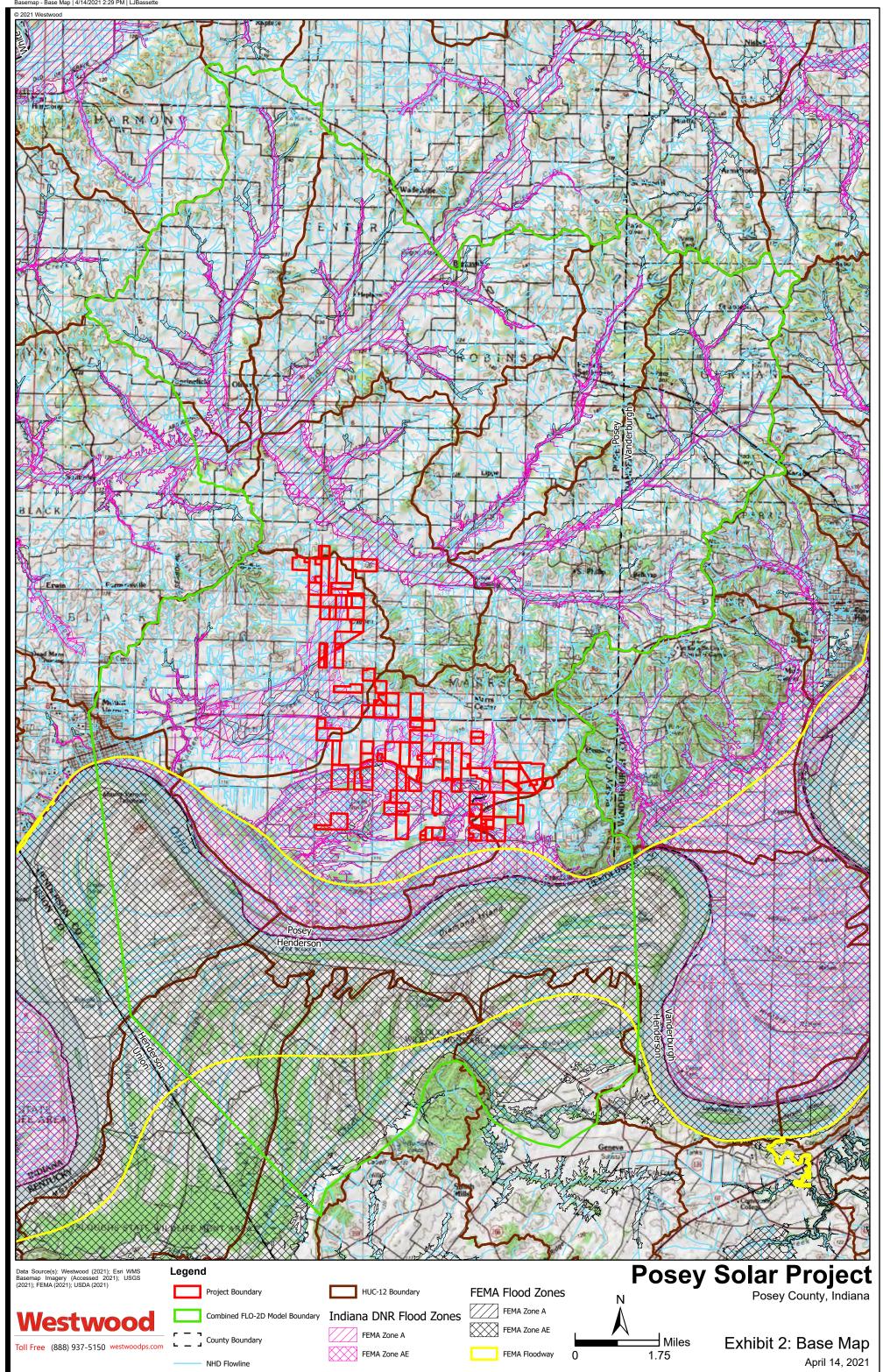
USDA 2013 Crop Data Layer, Landcover data. Retrieved July 2020, from https://www.nass.usda.gov/Research_and_Science/Cropland/SARS1a.php

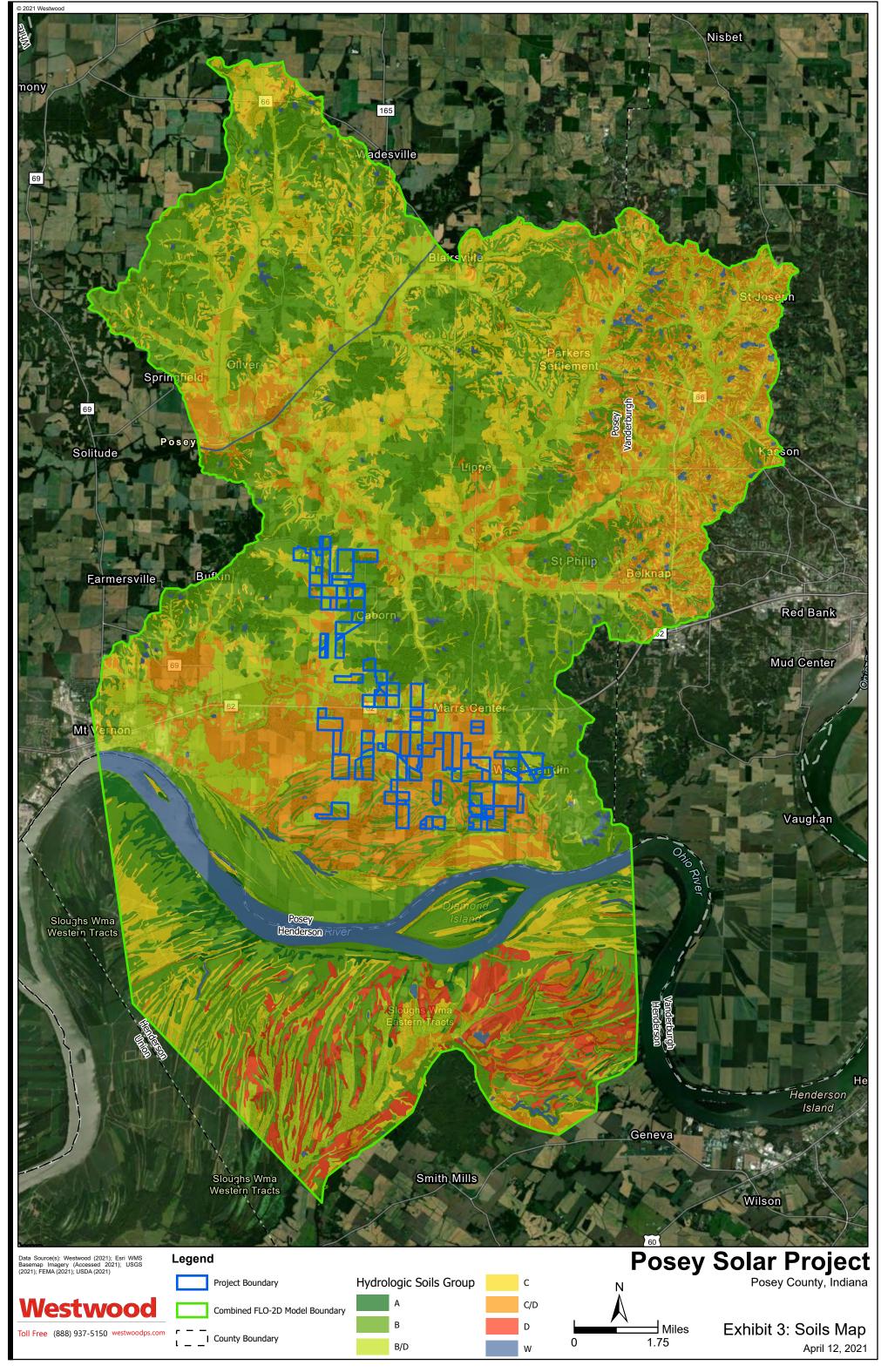
FEMA Flood Insurance Rate Maps. Retrieved July 2020 from https://msc.fema.gov/portal/advanceSearch#searchresultsanchor

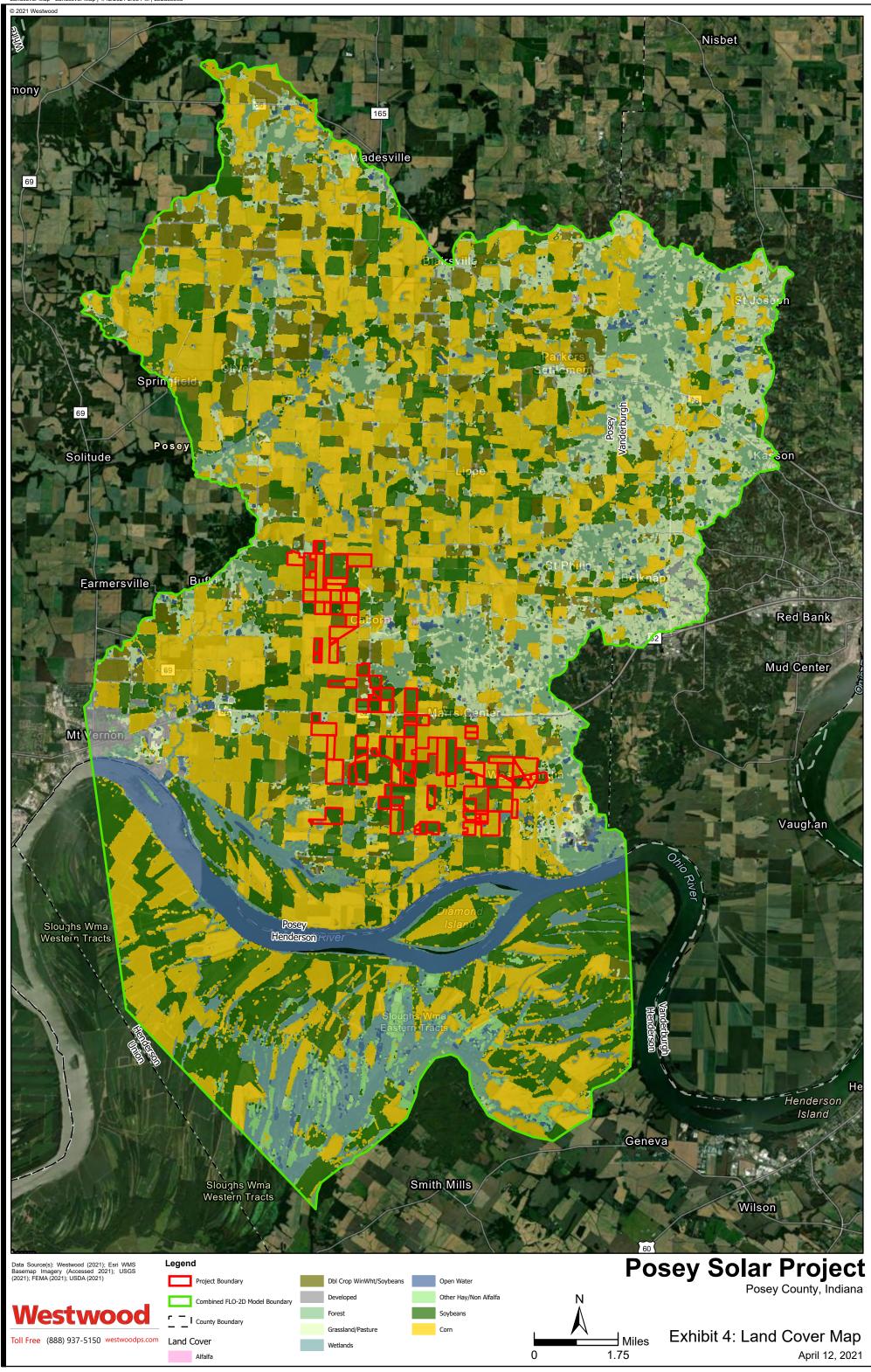
Indiana Department of Natural Resources. Indiana best Available Floodplain Mapping. Retrieved April 2021 from https://www.in.gov/dnr/water/9846.htm

Exhibits

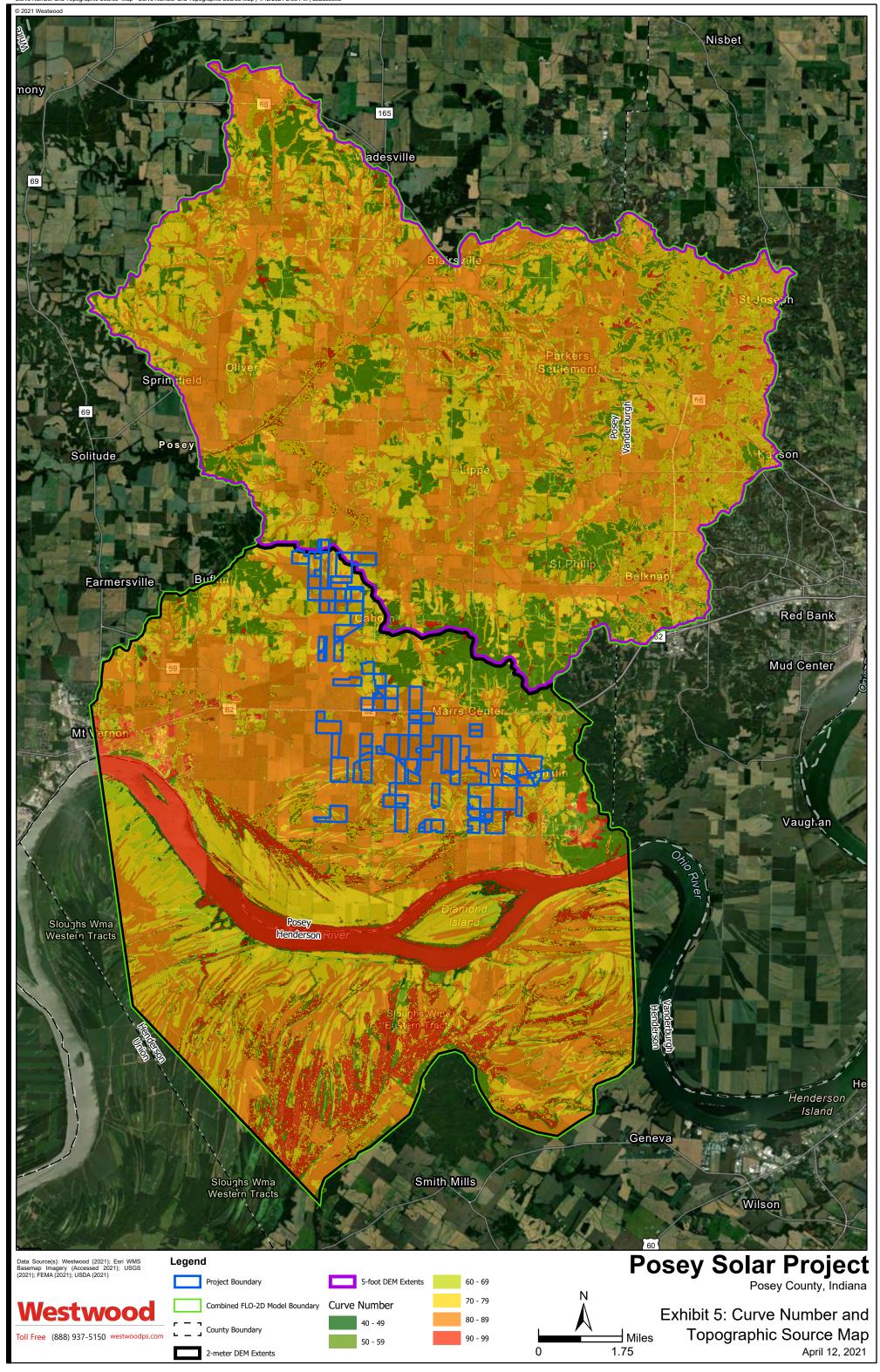


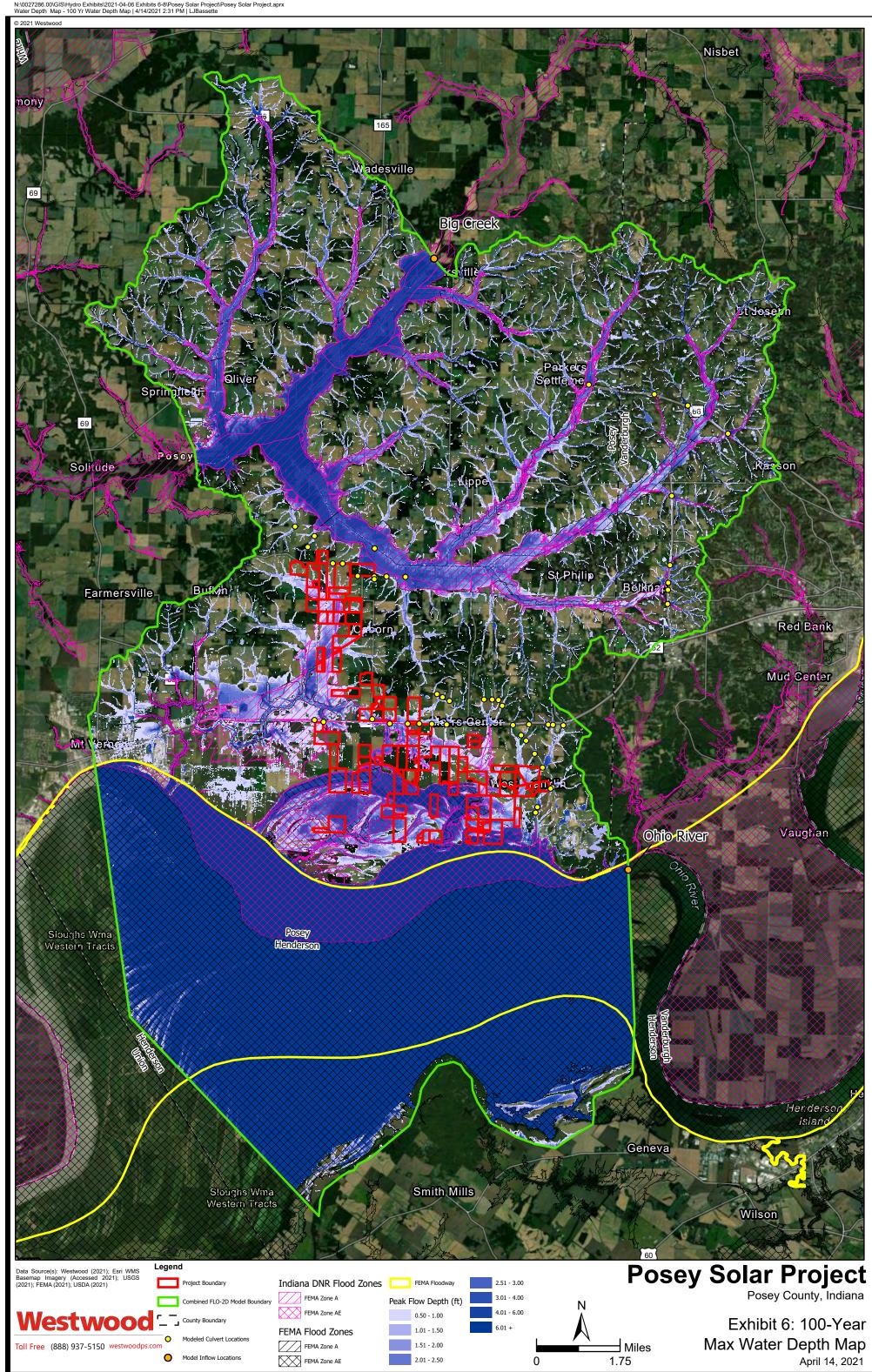


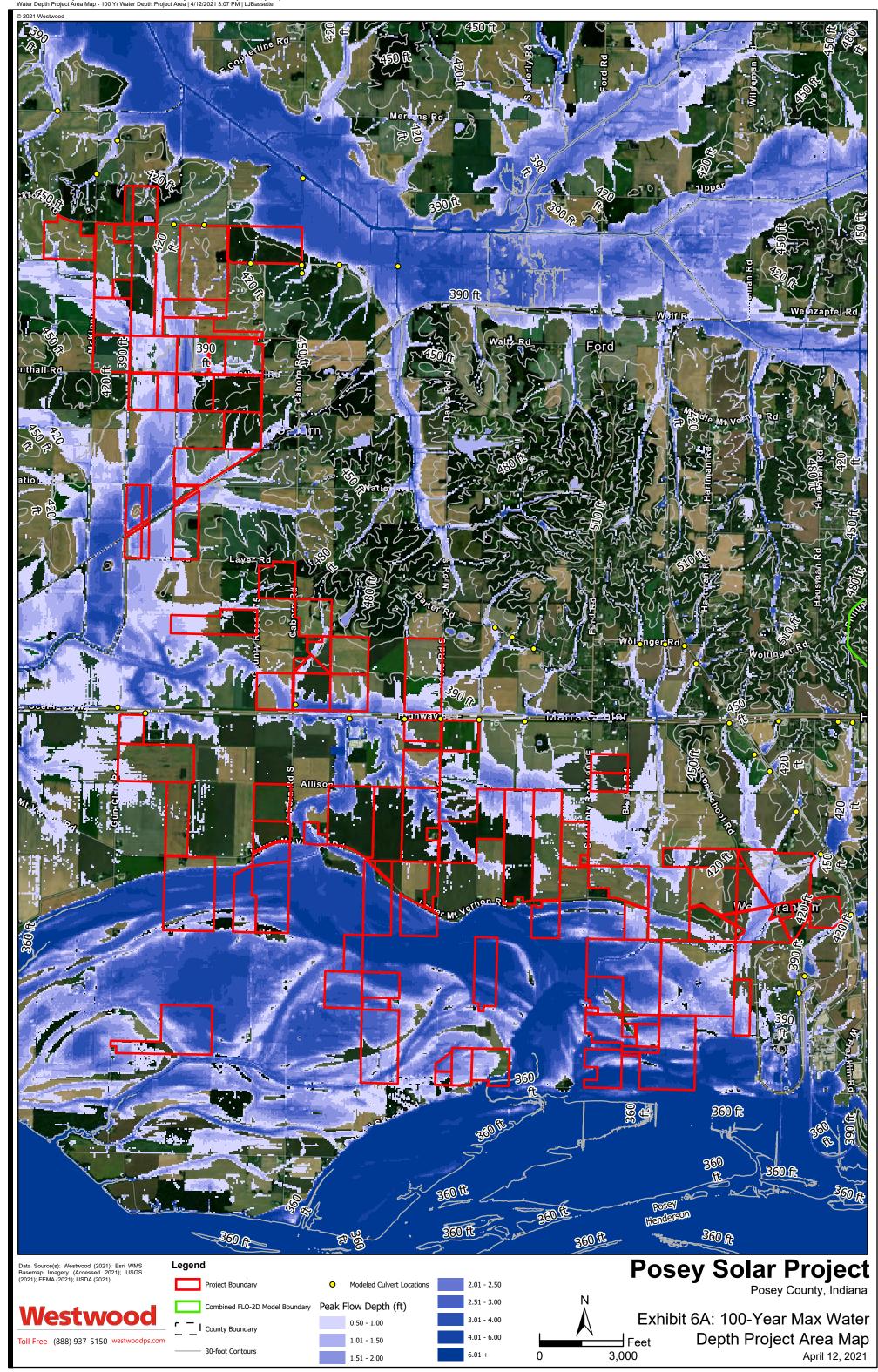


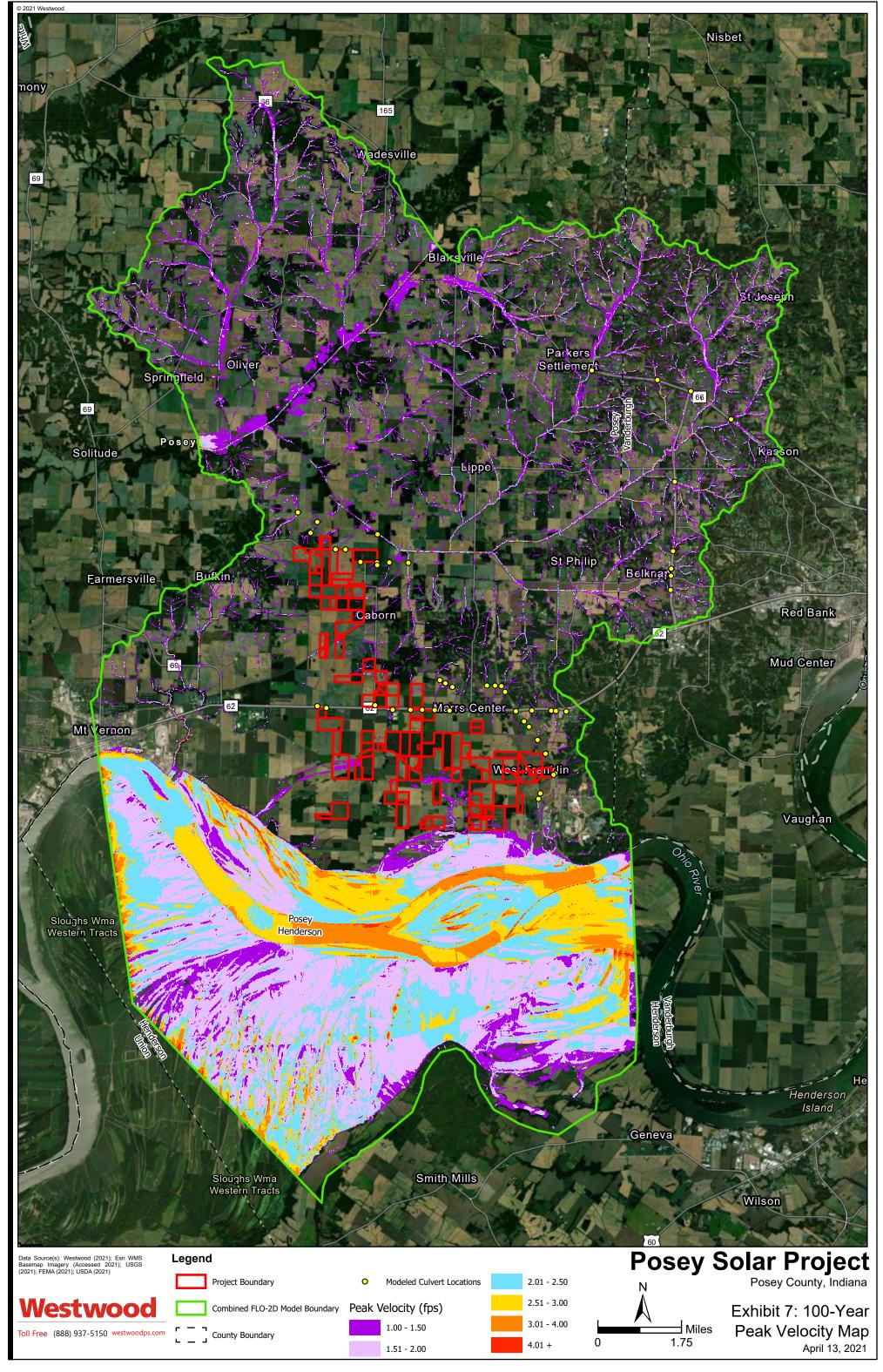


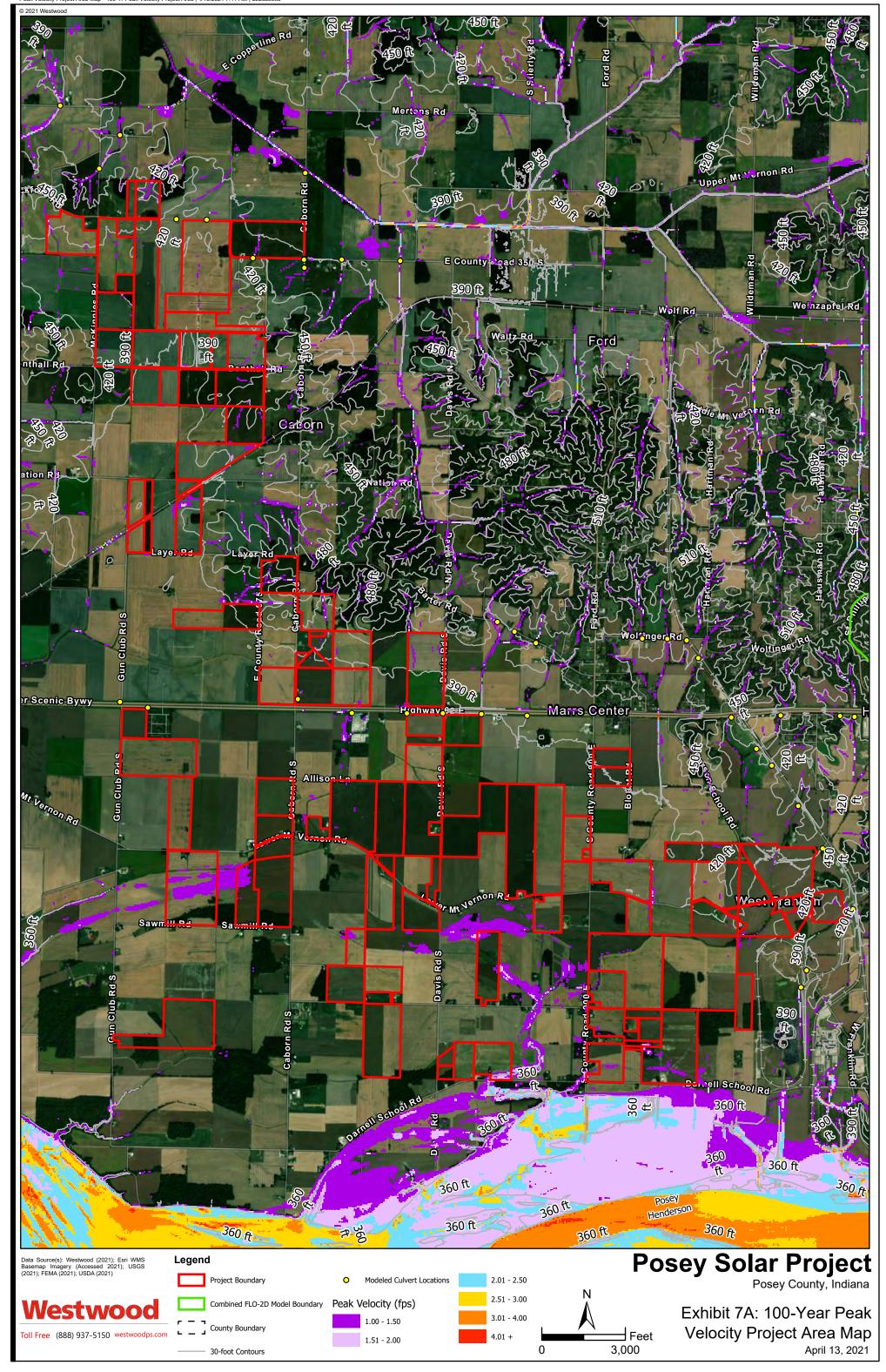
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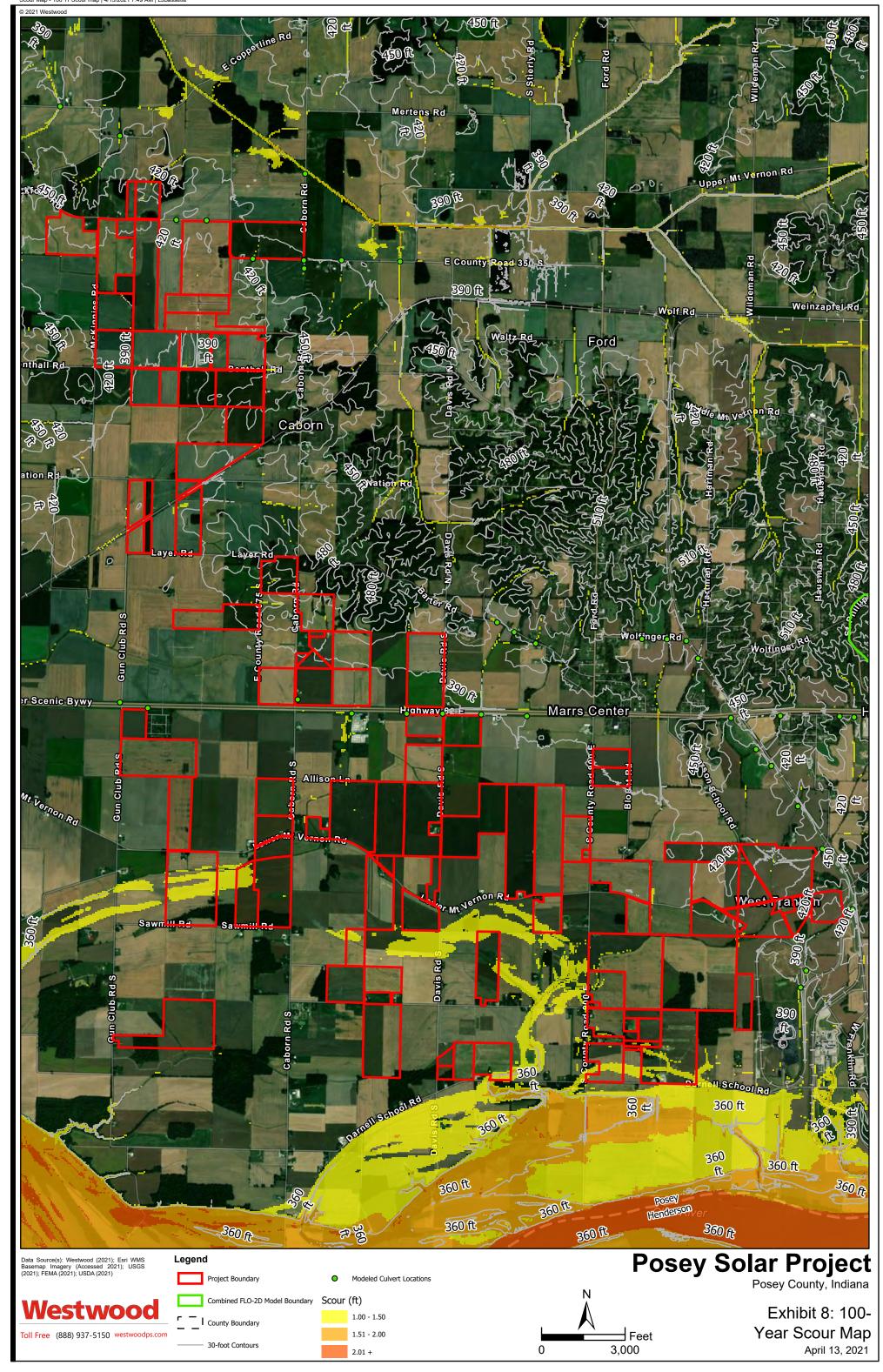








N:\0027286.00\GIS\Hydro Exhibits\2021-04-06 Exhibits 6-8\Posey Solar Project\Posey Solar Project.aprx Peak Velocity Project Area Map - 100 Yr Peak Velocity Project Area | 4/13/2021 7:41 AM | LJBassette



Appendix A Atlas 14 Rainfall Data

Precipitation Frequency Data Server



NOAA Atlas 14, Volume 2, Version 3 Location name: Mount Vernon, Indiana, USA* Latitude: 37.9198°, Longitude: -87.7712° Elevation: 369.98 ft** * source: ESRI Maps ** source: USGS



POINT PRECIPITATION FREQUENCY ESTIMATES

G.M. Bonnin, D. Martin, B. Lin, T. Parzybok, M.Yekta, and D. Riley

NOAA, National Weather Service, Silver Spring, Maryland

PF_tabular | PF_graphical | Maps_&_aerials

PF tabular

PDS	PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches) ¹										
Duration				Average	e recurrence	e interval (y	ears)				
Duration	1	2	5	10	25	50	100	200	500	1000	
5-min	0.395 (0.361-0.434)	0.466 (0.427-0.513)	0.552 (0.504-0.606)	0.622 (0.566-0.680)	0.710 (0.643-0.775)	0.779 (0.702-0.850)	0.847 (0.759-0.922)	0.918 (0.818-1.00)	1.01 (0.896-1.11)	1.08 (0.954-1.18)	
10-min	0.614 (0.561-0.674)	0.728 (0.666-0.801)	0.858 (0.784-0.941)	0.960 (0.873-1.05)	1.09 (0.983-1.19)	1.18 (1.06-1.29)	1.27 (1.14-1.39)	1.37 (1.22-1.49)	1.49 (1.32-1.63)	1.58 (1.39-1.72)	
15-min	0.752 (0.688-0.826)	0.890 (0.814-0.979)	1.05 (0.963-1.16)	1.18 (1.07-1.29)	1.34 (1.21-1.47)	1.46 (1.32-1.59)	1.58 (1.42-1.73)	1.70 (1.52-1.86)	1.86 (1.65-2.03)	1.97 (1.74-2.15)	
30-min	0.995 (0.910-1.09)	1.19 (1.09-1.31)	1.44 (1.32-1.58)	1.64 (1.49-1.79)	1.89 (1.72-2.07)	2.09 (1.88-2.28)	2.29 (2.05-2.49)	2.49 (2.22-2.71)	2.75 (2.44-3.01)	2.96 (2.60-3.23)	
60-min	1.22 (1.11-1.34)	1.46 (1.34-1.61)	1.81 (1.65-1.99)	2.09 (1.90-2.28)	2.46 (2.22-2.68)	2.75 (2.48-3.00)	3.06 (2.74-3.33)	3.37 (3.01-3.68)	3.81 (3.37-4.16)	4.15 (3.65-4.54)	
2-hr	1.47 (1.35-1.61)	1.78 (1.63-1.95)	2.22 (2.04-2.43)	2.57 (2.35-2.81)	3.06 (2.78-3.33)	3.45 (3.13-3.75)	3.86 (3.48-4.19)	4.29 (3.84-4.66)	4.87 (4.34-5.30)	5.34 (4.72-5.81)	
3-hr	1.58 (1.45-1.73)	1.91 (1.75-2.09)	2.39 (2.18-2.61)	2.78 (2.53-3.03)	3.32 (3.01-3.62)	3.76 (3.40-4.10)	4.23 (3.80-4.60)	4.72 (4.22-5.13)	5.42 (4.79-5.89)	5.98 (5.25-6.50)	
6-hr	1.94 (1.78-2.12)	2.33 (2.14-2.56)	2.91 (2.66-3.19)	3.38 (3.09-3.70)	4.05 (3.68-4.42)	4.60 (4.15-5.01)	5.18 (4.66-5.64)	5.80 (5.18-6.31)	6.68 (5.90-7.27)	7.39 (6.47-8.06)	
12-hr	2.29 (2.10-2.49)	2.76 (2.53-3.01)	3.42 (3.14-3.73)	3.97 (3.62-4.32)	4.73 (4.31-5.14)	5.36 (4.86-5.82)	6.02 (5.43-6.53)	6.71 (6.01-7.29)	7.70 (6.83-8.36)	8.49 (7.47-9.23)	
24-hr	2.74 (2.57-2.93)	3.30 (3.09-3.53)	4.10 (3.84-4.39)	4.75 (4.45-5.08)	5.66 (5.28-6.04)	6.40 (5.94-6.82)	7.17 (6.62-7.64)	7.98 (7.33-8.51)	9.11 (8.30-9.74)	10.0 (9.06-10.7)	
2-day	3.25 (3.03-3.49)	3.90 (3.64-4.20)	4.86 (4.53-5.22)	5.65 (5.25-6.06)	6.77 (6.27-7.27)	7.70 (7.10-8.27)	8.69 (7.96-9.34)	9.76 (8.88-10.5)	11.3 (10.2-12.2)	12.5 (11.2-13.6)	
3-day	3.46 (3.23-3.72)	4.15 (3.87-4.46)	5.15 (4.80-5.54)	5.98 (5.56-6.43)	7.18 (6.65-7.72)	8.19 (7.55-8.80)	9.26 (8.49-9.97)	10.4 (9.49-11.2)	12.1 (10.9-13.1)	13.5 (12.0-14.6)	
4-day	3.67 (3.43-3.95)	4.39 (4.10-4.73)	5.44 (5.07-5.86)	6.32 (5.88-6.80)	7.60 (7.04-8.18)	8.67 (8.00-9.34)	9.83 (9.02-10.6)	11.1 (10.1-12.0)	12.9 (11.6-14.0)	14.4 (12.9-15.7)	
7-day	4.26 (3.97-4.59)	5.10 (4.75-5.51)	6.31 (5.87-6.81)	7.31 (6.79-7.89)	8.77 (8.10-9.46)	9.99 (9.19-10.8)	11.3 (10.3-12.2)	12.7 (11.5-13.7)	14.7 (13.2-16.0)	16.4 (14.6-17.9)	
10-day	4.80 (4.46-5.21)	5.75 (5.35-6.24)	7.10 (6.60-7.71)	8.22 (7.62-8.92)	9.84 (9.07-10.7)	11.2 (10.3-12.1)	12.6 (11.5-13.7)	14.1 (12.8-15.4)	16.3 (14.6-17.8)	18.2 (16.1-19.8)	
20-day	6.62 (6.24-7.05)	7.86 (7.40-8.37)	9.42 (8.86-10.0)	10.7 (10.0-11.4)	12.4 (11.6-13.2)	13.8 (12.9-14.7)	15.2 (14.1-16.2)	16.7 (15.4-17.8)	18.7 (17.1-20.0)	20.2 (18.4-21.7)	
30-day	8.17 (7.72-8.65)	9.64 (9.12-10.2)	11.4 (10.8-12.1)	12.8 (12.1-13.5)	14.7 (13.8-15.6)	16.2 (15.2-17.2)	17.7 (16.5-18.8)	19.2 (17.9-20.5)	21.3 (19.7-22.7)	22.9 (21.0-24.5)	
45-day	10.2 (9.72-10.8)	12.0 (11.4-12.7)	14.0 (13.3-14.8)	15.6 (14.8-16.5)	17.8 (16.8-18.8)	19.5 (18.4-20.5)	21.2 (19.9-22.3)	22.8 (21.4-24.2)	25.1 (23.3-26.6)	26.8 (24.8-28.5)	
60-day	12.2 (11.6-12.8)	14.3 (13.6-15.1)	16.6 (15.8-17.5)	18.4 (17.5-19.4)	20.7 (19.7-21.9)	22.5 (21.3-23.8)	24.3 (22.9-25.6)	26.0 (24.4-27.5)	28.2 (26.4-29.9)	29.9 (27.8-31.8)	

¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

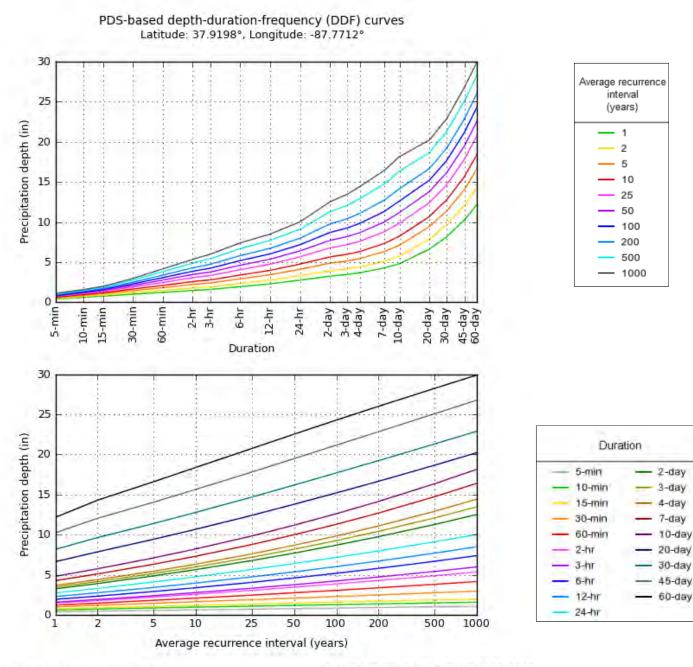
Numbers in parenthesis are PF estimates at low er and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the low er bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

Please refer to NOAA Atlas 14 document for more information.

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Precipitation Frequency Data Server

PF graphical



NOAA Atlas 14, Volume 2, Version 3

Created (GMT): Tue Jun 23 20:44:11 2020

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Maps & aerials

Small scale terrain

Precipitation Frequency Data Server



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US Department of Commerce National Oceanic and Atmospheric Administration National Weather Service National Water Center 1325 East West Highway Silver Spring, MD 20910 Questions?: <u>HDSC.Questions@noaa.gov</u>

Disclaimer

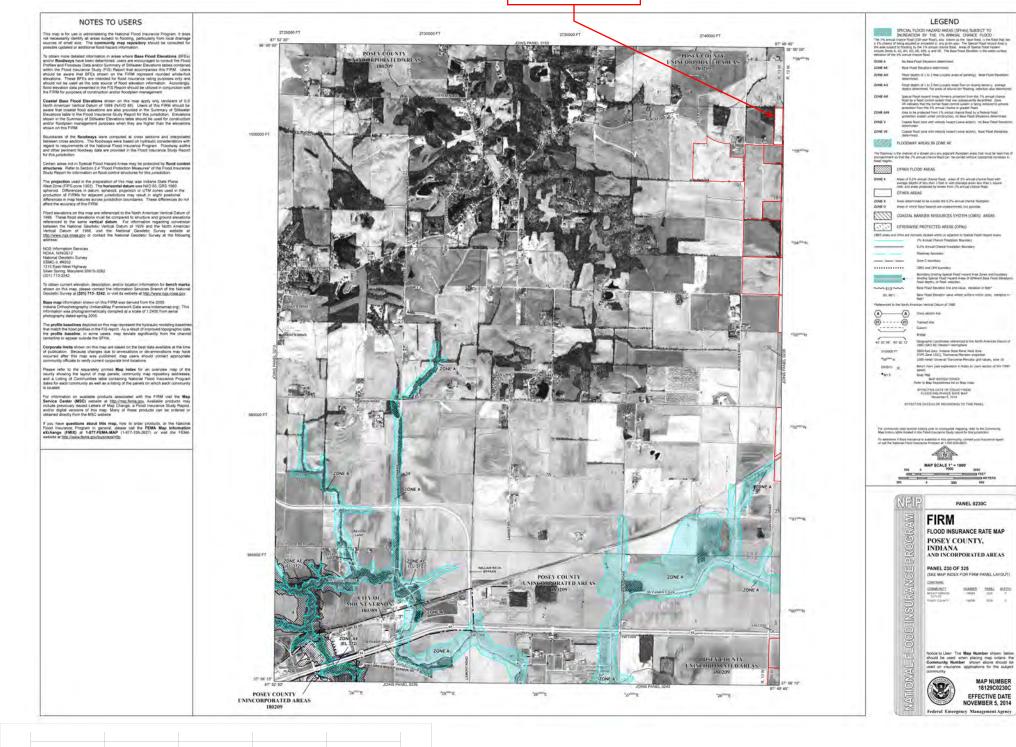
Appendix B Curve Number Table

Table 1. Standard Curve Numbers

					Curve Numb Soil Type*		
Class	Value	Classification Description [NLCD 2006]	A	В	с	D	w
er	11	Open Water - areas of open water, generally with less than 25% cover of vegetation or soil.	98	3 98	98	98	3 1
Water	12	Perennial Ice/Snow - areas characterized by a perennial cover of ice and/or snow, generally greater than 25% of total cover.	98	98	98	98	3 1
		Developed, Open Space - areas with a mixture of some constructed materials, but mostly vegetation in the form of lawn grasses. Impervious surfaces account for less than 20% of total cover. These areas most commonly include large-lot single-family housing units, parks, golf courses, and vegetation planted in developed settings for recreation, erosion control, or aesthetic purposes.	46	65	5 77	, 82	2 10
Developed		Developed, Low Intensity - areas with a mixture of constructed materials and vegetation. Impervious surfaces account for 20% to 49% percent of total cover. These areas most commonly include single-family housing units.	61	. 75	83	8 87	, 10
Deve	23	Developed, Medium Intensity – areas with a mixture of constructed materials and vegetation. Impervious surfaces account for 50% to 79% of the total cover. These areas most commonly include single-family housing units.	77	, 85	90	95	5 10
	24	Developed High Intensity -highly developed areas where people reside or work in high numbers. Examples include apartment complexes, row houses and commercial/industrial. Impervious surfaces account for 80% to 100% of the total cover.	89	92	. 94	95	5 10
Barren		Barren Land (Rock/Sand/Clay) - areas of bedrock, desert pavement, scarps, talus, slides, volcanic material, glacial debris, sand dunes, strip mines, gravel pits and other accumulations of earthen material. Generally, vegetation accounts for less than 15% of total cover.	77	7 86	91	. 94	10
	41	Deciduous Forest - areas dominated by trees generally greater than 5 meters tall, and greater than 20% of total vegetation cover. More than 75% of the tree species shed foliage simultaneously in response to seasonal change.	43				
Forest	42	Evergreen Forest - areas dominated by trees generally greater than 5 meters tall, and greater than 20% of total vegetation cover. More than 75% of the tree species maintain their leaves all year. Canopy is never without green foliage.	43	55	5 70) 77	, 10
	43	Mixed Forest - areas dominated by trees generally greater than 5 meters tall, and greater than 20% of total vegetation	43) 77	· 10
land		cover. Neither deciduous nor everareen species are greater than 75% of total tree cover. Dwarf Scrub - Alaska only areas dominated by shrubs less than 20 centimeters tall with shrub canopy typically greater than 20% of total vegetation. This type is often co-associated with grasses, sedges, herbs, and non-vascular vegetation.	43				
Shrubland	52	Shrub/Scrub - areas dominated by shrubs; less than 5 meters tall with shrub canopy typically greater than 20% of total vegetation. This class includes true shrubs, young trees in an early successional stage or trees stunted from environmental conditions.	43	48	65	5 73	3 10
SL	71	Grassland/Herbaceous - areas dominated by gramanoid or herbaceous vegetation, generally greater than 80% of total vegetation. These areas are not subject to intensive management such as tilling, but can be utilized for grazing.	43	58	5 71	. 78	3 10
Herbaceous	72	Sedge/Herbaceous - Alaska only areas dominated by sedges and forbs, generally greater than 80% of total vegetation. This type can occur with significant other grasses or other grass like plants, and includes sedge tundra, and sedge tussock tundra.	43	58	5 71	. 78	3 10
Her	73	Lichens - Alaska only areas dominated by fruticose or foliose lichens generally greater than 80% of total vegetation.	43	48	65	73	3 1
	74	Moss - Alaska only areas dominated by mosses, generally greater than 80% of total vegetation.	43	48	65	5 73	8 1
'Culti d		Pasture/Hay – areas of grasses, legumes, or grass-legume mixtures planted for livestock grazing or the production of seed or hay crops, typically on a perennial cycle. Pasture/hay vegetation accounts for greater than 20% of total vegetation.	43	58	71	. 78	3 1
Planted/Culti vated		Cultivated Crops – areas used for the production of annual crops, such as corn, soybeans, vegetables, tobacco, and cotton, and also perennial woody crops such as orchards and vineyards. Crop vegetation accounts for greater than 20% of total vegetation. This class also includes all land being actively tilled	67	78	85	89) 1
μ		Small Grains	63	3 75	83	8 87	' 1
Wetlan ds		Woody Wetlands - areas where forest or shrubland vegetation accounts for greater than 20% of vegetative cover and the soil or substrate is periodically saturated with or covered with water.	45	66	5 77	' 83	s 1
Me Me		Emergent Herbaceous Wetlands - Areas where perennial herbaceous vegetation accounts for greater than 80% of vegetative cover and the soil or substrate is periodically saturated with or covered with water.	45	66	5 77	83	3 1

*A/D, B/D and C/D soils lumped as D soils, W denotes water **Curve Numbers for NLCD Codes 41-81 have been increased from 30 to 43 as many of these areas are partially grazed Woods-grass combination.

Appendix C FEMA FIRM Panels



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2.5 mi





This map is for use in administering the National Picod Insurance Program. It do not necessarily identify all areas subject to flooding, particularly from lood drama sources of small size. The community map repository should be consulted to possible updated or additional flood hazard information.

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Coastal Base Flood Elevations shown on this map apply only landesid North American Vertical Datum of 1988 (HAVD 88). Users of this FRM show many that coastal flood elevations are also provided in the Summary of Still Elevations table in the Pool Insurance Study Pepon for this junktictory. Ele-teron is the Summary of Stillard Chardren base shown in the Study Pepon Statement of Bellevations Chardren base shown in the Statement of Stillard Chardren based and the Statement of Stillard Chardren based and show the Statement of Stillard Chardren based and show the Statement of Stillard Chardren based and show the Statement of Stillard Statement Chardren based and show the Statement of Stillard Statement of Stillard Statement of Stillard Statement of Stillard Statement of Statement Chardren based and show the Statement of Stillard Statement of Stillard Statement of State ary of Stillwater Elevation magement purposes who

Boundaries of the **Boodways** were computed at cross sections and interpolated between cross sections. The Boodways were based on trydinalic considerations with regard to requirements of the distance Property Produce worths and other periment floodway date provided in the Plood Insurance Study Report.

Certain areas not in Special Flood Hagard Annai may be protected by **flood o** structures. Rafe to Section 2.4 "Social Protection Measures" of the Flood his Study Resonance for information on flood control attractures for this unaddrition.

The projection used in the preparation of this map was indices State Plane West 20m PTPS zone 1520; The heatsettiel allow was AUD 35, CMS 1550 or protection of PTPM heat agreement president may ensure in sight posteriori differences in map features across periodictor boundaries. These differences do which the acouncy of the PTPM.

ood elevations on this map are refer Flood enveloping of this map are methodiad to the North American Vertical Datum of 1988. These flood envelopes must be compared to include and ground elevation informatical to the same vertical datam. For information regarding convenies vertical Datum of 1988, set the National Geodetic Survey at the followin address: elevation of 1988, set the National Geodetic Survey at the followin address:

NGS information Services NGAA, N/NGS12 National Geodetic Survey National Geodetic Survey SSMC-3, #9202 1315 East-West Highway Silver Spring, Maryland 20910-3282 Iver Spring, Ma (01) 713-3242

obtain current elevation, description, and/or location information for bench man own on this map, please contact the information Services Branch of the Natio extents Survey at (301) 713-3242, or visit its website at <u>http://www.ndp.inform.pd</u>

Base map information shown on the FINM was derived from the 2000 indexa Orthophotography informatilg Finnewerk Data swee inducerance org. Information was prodogrammeticably compiled at a scale of 1.2400 from early hydrography dated spring 2005.

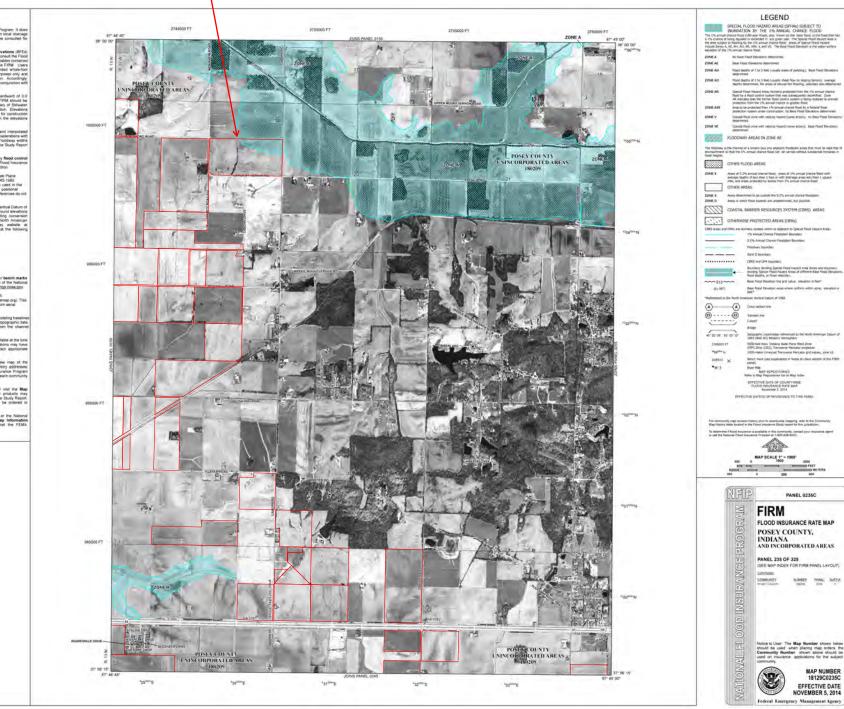
The profile baselines depicted on the map represent the hydraulic modeling baselines has match the flood profiles in the FIS report. As a result of imponvet topographic data, the profile baseline, in some cases, may deviate significantly from the channel vertiening or appear outside the SFH4.

Corporate limits shown on this map are based on the best data available of publication. Because changes due to annexistions or de-interaction pooured after this map was published map users should contact community officials to writh yourest corporate milliolations. lable at the tin a may have

Please refer to the separately printed Map Index for an overview map of the county showing the layout of map genesis, community map repository addresses and a Listing of Communities table containing National Flood Insurance Pregnan dates for each constrainty as well as a listing of the panels on which each contraunty for the contract of the second contract of the panels on which each contraunty and the second contract of the se

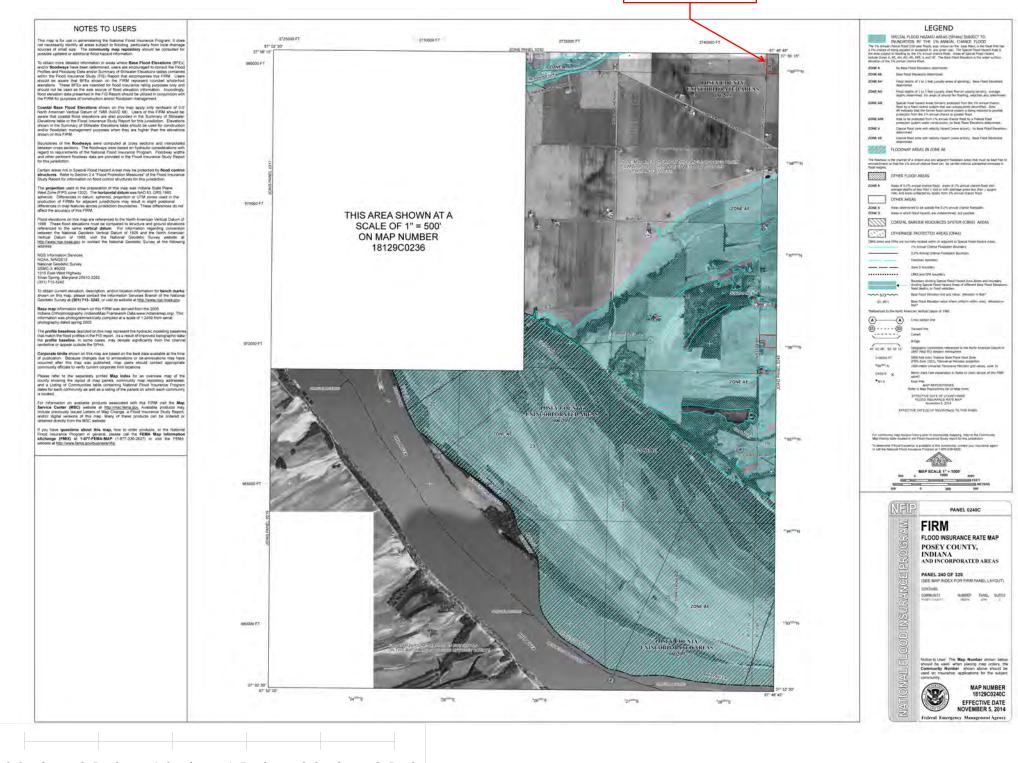
For information on available products askocated with this FIRM viait the Map Service Center (MSC) wetsites all <u>the importence too</u> Available products may include previously asked Letters of Map Change, a Flood traumos duud Report, and/or digital versions of this map Mary of these products can be ordered to obtained directly from the MSC setable.

If you have questions about this map, how to order products, or the National Flood Insurance Program in peneral, please call the FEMA Map Information exchange (FMN) in 1-677-68M.AMAP (1-677-336-2627) or vali the FEMA website of <u>http://www.fema.aov/businessinfip</u>

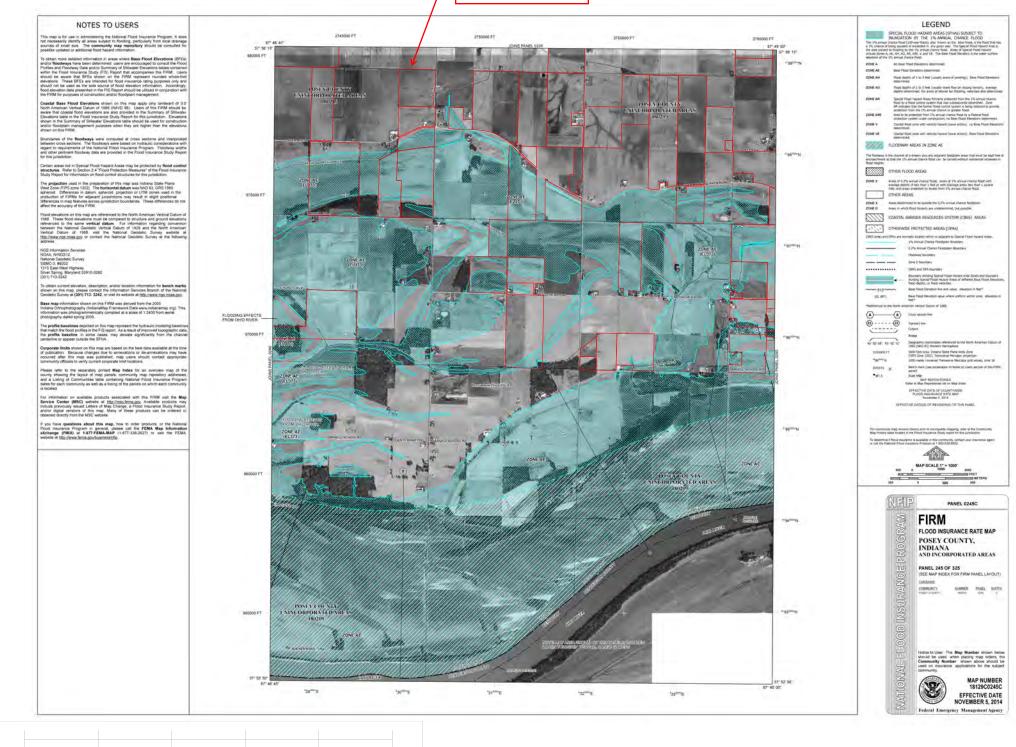


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MAP NUMBER

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> MAP SCALE 1" = 1000"

FIRM FLOOD INSURANCE RATE MAP POSEY COUNTY, INDIANA AND INCORPORATED AREAS PANEL 265 OF 325 ISEE MAP INDEX FOR FIRM PANEL LAYOUTI

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This map is for use in administering the National Flood Insurance Program. It does not necessarily identity all amise subject to flooding, particularly from local disinage sources of small size. The **community map repository** should be consulted for possible updated or additional flood hazard information.

To obtain more detailed information in areas where Base Flood Elevia and/or floodways have been determined, users are encouraced to con ions (BFEs did floodways have been determined, users are monoraged to consult the Floo offers and Floodbay (bian and/or Summary of Silvaket Elbowins tables contained in the Flood Insurance Silvay (FIS) Report that accompanies this FRM. User lack to eaver that IREs alrows and the FRM regression states contained valued on the said as the Said source of the FRM regression of y and add not the used as the said source of flood leveluan information. Accompting of allowation state presented in the FIG Report should be utilised in comparison with FIRM for papers of containcion analysis flood planet.

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Certain areas not in Special Flood Hazard Areas may be protected by flood contro structures. Refer to Section 2.4 "flood Protection Measures" of the Flood Insurance Study Report for information on flood control structures for this junctidiction.

The projection used in the properties of its imply was locate 3 ball time heat 200 MPR 200 MP

Page elevations on this map are inferenced to the florth American Versical Datum of 1988. These flood elevations mult be compared to Shutchy end ground elevations relevanced to the american Versical Sector Totol Totol and the American Versical Datum of 1988, with the National Geodetic Survey webles at <u>http://www.mational.org</u> or contact the National Geodetic Survey at the Klowing adverses.

NGS Information Services NGAA, N/NGS12 National Geodetic Survey SSMC-3, #0202 1315 East-West Highway Silver Spring, Maryland 20910-3282 (301) 713-3242

To obtain current elevation, description, and/or location information for bench marks shown on this map, please contact the information Services Branch of the Nation Geodetic Survey at (301) 713- 3242, or visit its website at http://www.nos.ntosis.gov

Base map information shown on this FIRM was derived from the 2005 indiana Dimophotography indianalities (Firmwork Data www.indianame.org) information was photogrammetricatly compiled at a scale of 12400 from senial thetography dated spring 2005.

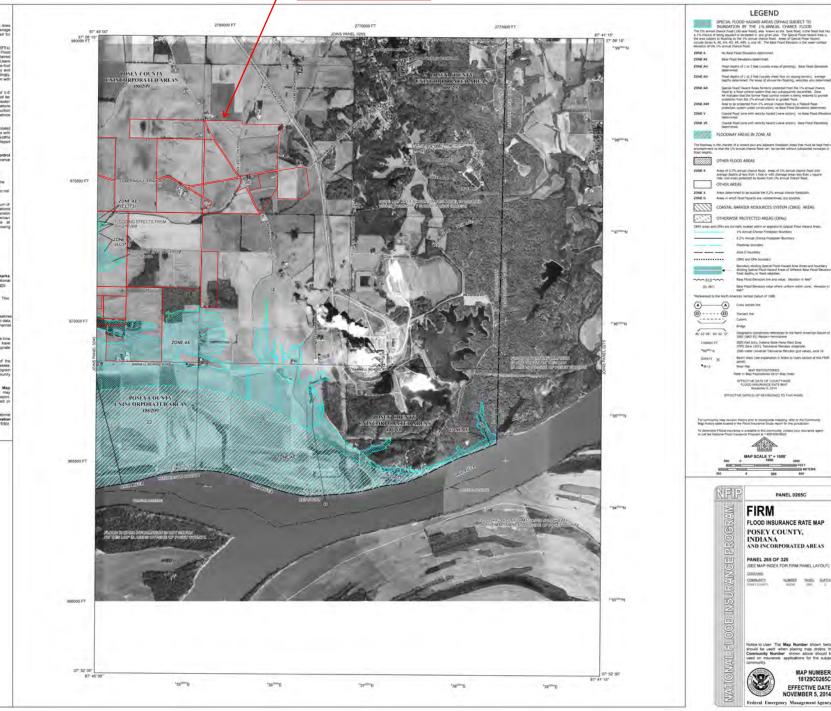
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For information on available products associated with the FIRM with the Maps Service Center (MSC) website at <u>Hostings Service</u>, available products may include previously issued Letters of Map Change, a Hood Issuance Study Report, and/or dipala versions of this map. Many of these products can be ordered or obtained directly front the MSC website

If you have questions about this map, how to other products, or the 1 Food insurance Program in general, please call the FEMA Map Info exchange (FMIX) at 1-877-FEMA-MAP (1-877-336-2627) or visit the unitariat at for lower long or product and the second s



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Appendix D FEMA FIS Report Excerpts

This Revision

Hydrologic analyses were carried out to establish peak discharge-frequency relationships for the flooding source studied by detailed methods affecting Posey County. Table 6 contains a summary of peak discharges for the 10-, 2-, 1-, and 0.2- percent annual chance floods, where applicable, for each flooding source studied in detail in Posey County. Peak discharges in the table were provided by the U.S. Army Corps of Engineers, Louisville District.

Table 6. Summary of Discharges

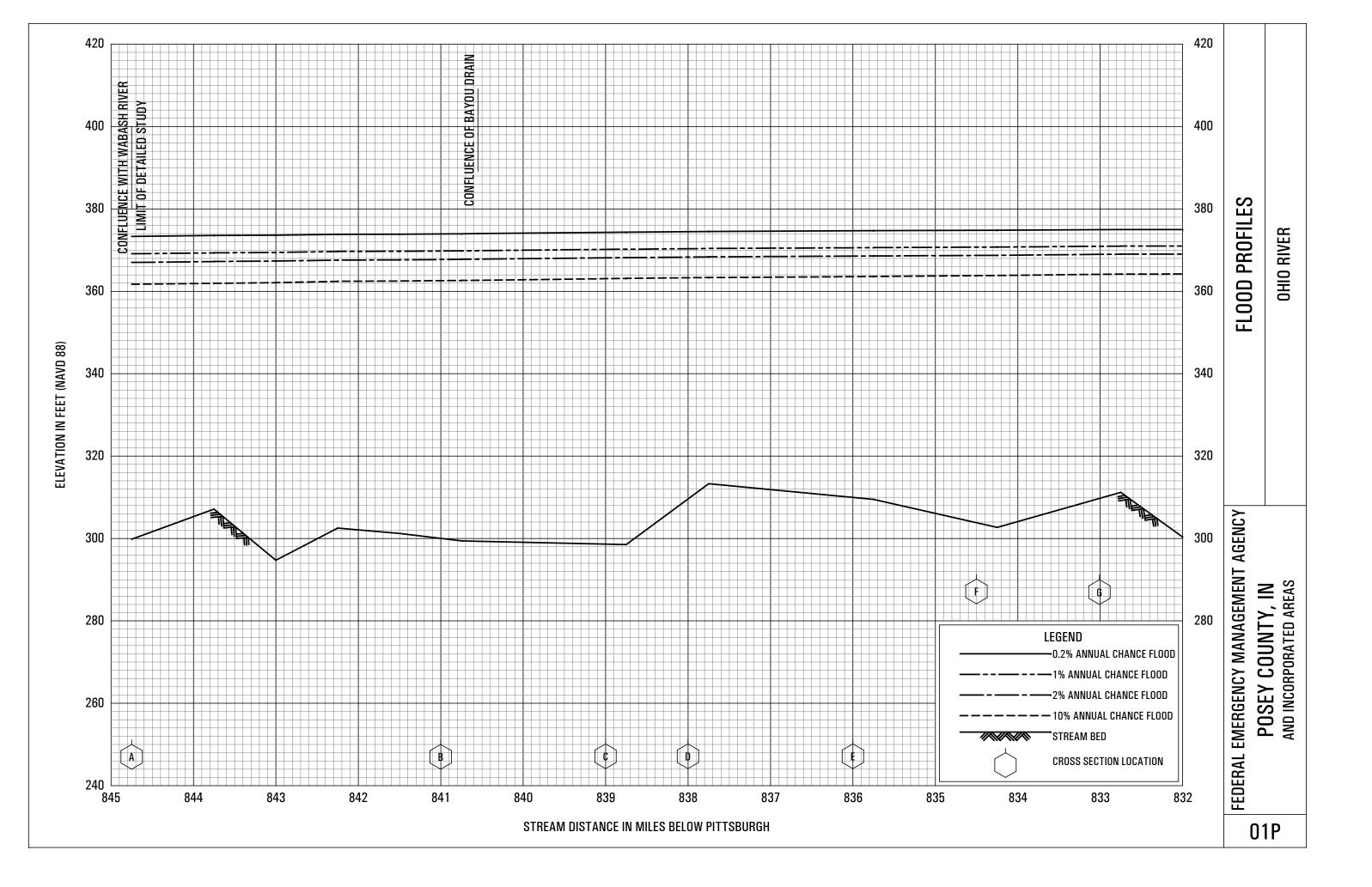
			Peak Di	scharge (CFS)	
		10%	2%	1%	0.2%
Flooding Source	Drainage Area	Annual	Annual	Annual	Annual
And Location	(Square Miles)	<u>Chance</u>	<u>Chance</u>	<u>Chance</u>	<u>Chance</u>
OHIO RIVER					
At Evansville Gage, IN	107,000	708,000	853,000	<mark>920,000</mark>	1,080,000

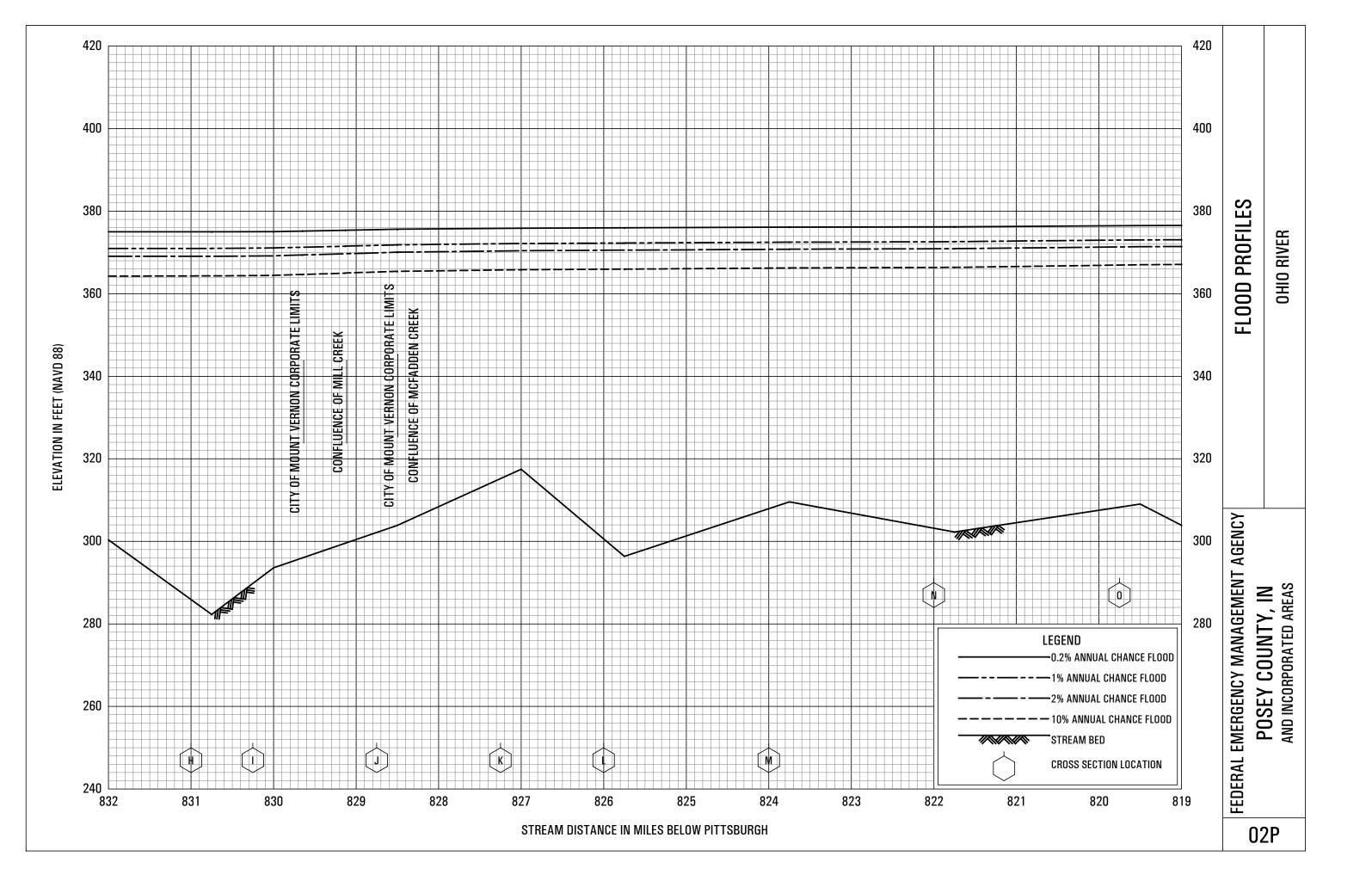
The hydrologic analyses for the Ohio River were performed by the U.S. Army Corps of Engineers. Frequency discharge data for the entire length of the Ohio River are available based on an analysis conducted by the Ohio River Division of the Corps of Engineers in Cincinnati, Ohio. Natural discharge-frequency curves for the Ohio River were developed in accordance with methods presented in papers by Leo R. Beard, <u>Statistical Methods in Hydrology</u>. Modified discharge-frequency curves, for the Ohio River, resulted from routing twelve representative floods for the Ohio River modified by an upstream Corps of Engineers reservoir system. That system included reservoirs completed or near completion in 1976 and is considered current in 2002. Data were plotted opposite original flood data on a grid containing a referenced flow reduction of natural flow and a new best-fit curve drawn. Total reductions were read from the new curve at selected natural flow frequencies, and subtracted from natural flows at those frequencies to obtain new modified-flow values.

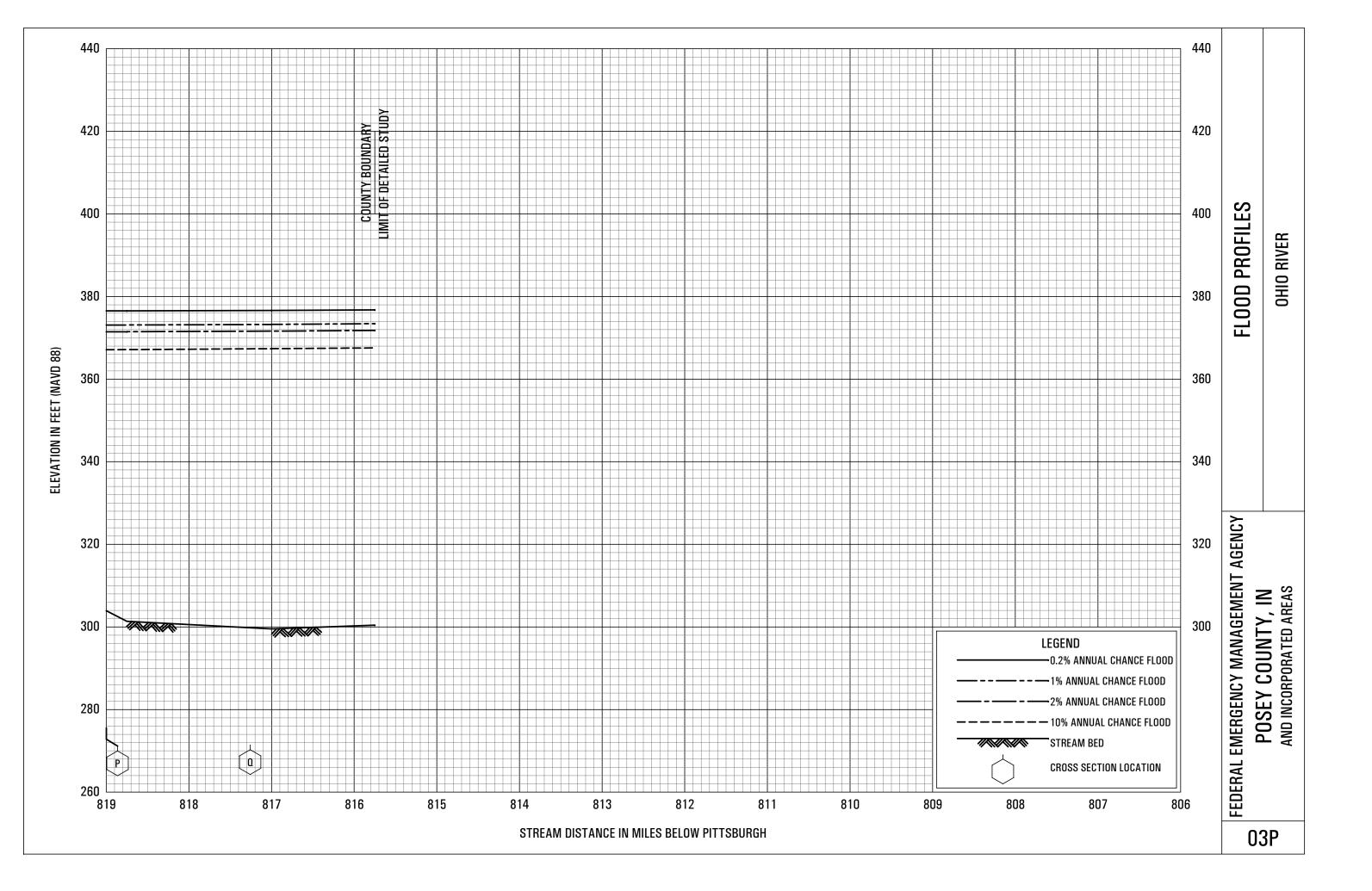
Standard and accepted hydrologic methods were used to develop discharge data on the study streams in Posey County. These data were coordinated with the Indiana Department of Natural Resources, the Natural Resources Conservation Service (formally the Soil Conservation Service), the U. S. Geological Survey and the Louisville District of the U. S. Army Corps of Engineers, through a Memorandum Of Understanding dated May 6, 1976. Discharge curves for the 10%, 2%, 1%, and 0.2% annual chance floods were developed for each study stream using several different procedures and compared for consistency.

3.2 Hydraulic Analyses

Analyses of the hydraulic characteristics of flooding from the sources studied were carried out to provide estimates of the elevations of floods of the selected recurrence intervals. Users should be aware that flood elevations shown on the Flood Insurance







Appendix E USGS StreamStats Station Report



StreamStats Data-Collection Station Report

USGS Station Number 03378550 Station Name BIG CREEK NEAR WADESVILLE, IN

Click here to link to available data on NWIS-Web for this site.

Descriptive Information

Station Type	Streamgage, continuous record
Location	Lat 38°04'59.2", long 87°46'10.7" referenced to North American Datum of 1983, in SW 1/4 SW 1/4 sec.16, T.5 S., R.12 W.,
	Posey County, IN, Hydrologic Unit 05120113, on left bank at downstream side of bridge on State Highway 66, 0.6 mi northwest of Blairsville, 0.8 mi upstream from County Road 250 North, and 1.6 mi southeast of Wadesville.
Gage	Water-stage recorder. Datum of gage is 369.70 feet above the North American Vertical Datum of 1988.
Regulation and Diversions	None known.
Regulated?	Unknown
Period of Record	
Remarks	
Latitude (degrees NAD83)	38.08282118
Longitude (degrees NAD83)	-87.7694692
Hydrologic unit code	05120113
County	129-Posey
HCDN2009	No

Citation

Physical Characteristics

Characteristic Name	Value	Units	Citation Number
Descriptive Information			
Datum_of_Latitude_Longitude	NAD83	dimensionless	<u>30</u>
District_Code	18	dimensionless	<u>30</u>
Precipitation Statistics			
24_Hour_2_Year_Precipitation	3.3000	inches	<u>31</u>
Mean_Annual_Precipitation	42.000	inches	<u>31</u>
Temperature Statistics			
Mean_Min_January_Temperature	21.000	degrees F	<u>31</u>
Topographical Characteristics			
Elevation_of_10_and_85_points	445.000	feet	<u>31</u>
Mean_Basin_Elevation	445.000	feet	<u>31</u>
Land Cover Characteristics			
Percent_Forest	11.400	percent	<u>31</u>
Percent_Storage	0.6100	percent	<u>31</u>
Stream Channel Properties			
Main_Channel_Length	18.600	miles	<u>31</u>
Stream_Slope_10_and_85_Method	3.8000	feet per mi	<u>31</u>
Basin Dimensional Characteristics			
Contributing_Drainage_Area	104.000	square miles	<u>31</u>
Drainage_Area	104	square miles	<u>290</u>
r			

Streamflow Statistics

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StreamStats Data-Collection Station Report

Statistic Name	Value	Units	Number	Preferred? Record	percent	log-10	Interval	Interval	Date
Peak-Flow Statistics									
10_percent_AEP_flood	7470	cubic feet per second	<u>62</u>	Y					
4_percent_AEP_flood	8780	cubic feet per second	<u>62</u>	Y					
2_percent_AEP_flood	9740	cubic feet per second	<u>62</u>	Y					
1_percent_AEP_flood	10700	cubic feet per second	<u>62</u>	Y					
0_5_percent_AEP_flood	11600	cubic feet per second	<u>62</u>	Y					
0_2_percent_AEP_flood	12800	cubic feet per second	<u>62</u>	Y					
Systematic_peak_years Flood-Volume Statistics	38	years	<u>62</u>	Y					
1_Day_2_Year_Maximum	3070.00	cubic feet per second	<u>31</u>	Y					
1_Day_20_Year_Maximum	5570.00	cubic feet per second	<u>31</u>	Y					
1_Day_5_Year_Maximum	4170.00	cubic feet per second	<u>31</u>	Y					
1_Day_10_Year_Maximum	4850.00	cubic feet per second	<u>31</u>	Y					
1_Day_25_Year_Maximum	5700.00	cubic feet per second	<u>31</u>	Y					
1_Day_50_Year_Maximum	6500.00	cubic feet per second	<u>31</u>	Y					
3_Day_2_Year_Maximum	1960.00	cubic feet per second	<u>31</u>	Y					
3_Day_5_Year_Maximum	2840.00	cubic feet per second	<u>31</u>	Y					
3_Day_10_Year_Maximum	3400.00	cubic feet per second	<u>31</u>	Y					
3_Day_20_Year_Maximum	4050.00	cubic feet per second	<u>31</u>	Y					
3_Day_25_Year_Maximum	4200.00	cubic feet per second	<u>31</u>	Y					
3_Day_50_Year_Maximum	4900.00	cubic feet per second	<u>31</u>	Y					
3_Day_100_Year_Maximum	5580.00	cubic feet per second	<u>31</u>	Y					
7_Day_2_Year_Maximum	1180.00	cubic feet per second	<u>31</u>	Y					
7_Day_5_Year_Maximum	1630.00	cubic feet per second	<u>31</u>	Y					
7_Day_10_Year_Maximum	1930.00	cubic feet per second	<u>31</u>	Y					
7_Day_20_Year_Maximum	2240.00	cubic feet per second	<u>31</u>	Y					
7_Day_25_Year_Maximum	2300.00	cubic feet per second	<u>31</u>	Y					
7_Day_50_Year_Maximum	2630.00	cubic feet per second	<u>31</u>	Y					
7_Day_100_Year_Maximum	2950.00	cubic feet per second	<u>31</u>	Y					
15_Day_2_Year_Maximum	667.000	cubic feet per second	<u>31</u>	Y					
15_Day_10_Year_Maximum	1160.00	cubic feet per second	<u>31</u>	Y					
15_Day_20_Year_Maximum	1380.00	cubic feet per second	<u>31</u>	Y					

4	1/5/2021		Stream	Stats Data	-Collectio	n Station Report	
	15_Day_25_Year_Maximum	1420.00	cubic feet per second	<u>31</u>	Y		
	15_Day_50_Year_Maximum	1630.00	cubic feet per second	<u>31</u>	Y		
	15_Day_100_Year_Maximum	1840.00	cubic feet per second	<u>31</u>	Y		
	30_Day_2_Year_Maximum	450.000	cubic feet per second	<u>31</u>	Y		
	30_Day_10_Year_Maximum	646.000	cubic feet per second	<u>31</u>	Y		
	30_Day_25_Year_Maximum	745.000	cubic feet per second	<u>31</u>	Y		
	30_Day_50_Year_Maximum	830.000	cubic feet per second	<u>31</u>	Y		
	Low-Flow Statistics						
	1_Day_10_Year_Low_Flow	0	cubic feet per second	<u>290</u>	Y	46	4/1/1966 3/3 [,]
	7_Day_10_Year_Low_Flow	0	cubic feet per second	<u>290</u>	Y	46	4/1/1966 3/3 [.]
	30_Day_10_Year_Low_Flow	0	cubic feet per second	<u>290</u>	Y	46	4/1/1966 3/3 [.]
	Flow-Duration Statistics						
	1_Percent_Duration	2210	cubic feet per second	<u>290</u>	Ν	46	10/1/1965 9/3(
	1_Percent_Duration	2080	cubic feet per second	<u>325</u>	Y	50	10/1/1965 9/3(
	2_Percent_Duration	1270	cubic feet per second	<u>325</u>	Y	50	10/1/1965 9/3(
	2_Percent_Duration	1280	cubic feet per second	<u>290</u>	Ν	46	10/1/1965 9/3(
	3_Percent_Duration	892	cubic feet per second	<u>325</u>	Y	50	10/1/1965 9/3(
	5_Percent_Duration	523	cubic feet per second	<u>290</u>	Ν	46	10/1/1965 9/3(
	5_Percent_Duration	536	cubic feet per second	<u>325</u>	Y	50	10/1/1965 9/3(
	10_Percent_Duration	217	cubic feet per second	<u>290</u>	Ν	46	10/1/1965 9/3(
	10_Percent_Duration	217	cubic feet per second	<u>325</u>	Y	50	10/1/1965 9/3(
	15_Percent_Duration	128	cubic feet per second	<u>325</u>	Y	50	10/1/1965 9/3(
	20_Percent_Duration	90	cubic feet per second	<u>290</u>	Ν	46	10/1/1965 9/3(
	20_Percent_Duration	89	cubic feet per second	<u>325</u>	Y	50	10/1/1965 9/3(
	25_Percent_Duration	66	cubic feet per second	<u>325</u>	Y	50	10/1/1965 9/3(
	30_Percent_Duration	52	cubic feet per second	<u>290</u>	Ν	46	10/1/1965 9/3(
	30_Percent_Duration	51	cubic feet per second	<u>325</u>	Y	50	10/1/1965 9/3(
	35_Percent_Duration	40	cubic feet per second	<u>325</u>	Y	50	10/1/1965 9/3(
	40_Percent_Duration	32	cubic feet per second	<u>290</u>	Ν	46	10/1/1965 9/3(
	40_Percent_Duration	31	cubic feet per second	<u>325</u>	Y	50	10/1/1965 9/3(
	45_Percent_Duration	24	cubic feet per second	<u>325</u>	Y	50	10/1/1965 9/3(
	50_Percent_Duration	18	cubic feet per second	<u>290</u>	Ν	46	10/1/1965 9/3(
	50_Percent_Duration	18	cubic feet per second	<u>325</u>	Y	50	10/1/1965 9/3(

https://streamstatsags.cr.usgs.gov/gagepages/html/03378550.htm

4/5/2021		Stream	nStats Da	ata-Collect	tion Station Report	
55_Percent_Duration	13	cubic feet per second	<u>325</u>	Y	50	10/1/1965 9/3(
60_Percent_Duration	9	cubic feet per second	<u>290</u>	Ν	46	10/1/1965 9/3(
60_Percent_Duration	9.2	cubic feet per second	<u>325</u>	Y	50	10/1/1965 9/3(
65_Percent_Duration	6	cubic feet per second	<u>325</u>	Y	50	10/1/1965 9/3(
70_Percent_Duration	3.9	cubic feet per second	<u>290</u>	Ν	46	10/1/1965 9/3(
70_Percent_Duration	3.9	cubic feet per second	<u>325</u>	Y	50	10/1/1965 9/3(
75_Percent_Duration	2.5	cubic feet per second	<u>325</u>	Y	50	10/1/1965 9/3(
80_Percent_Duration	1.5	cubic feet per second	<u>290</u>	Ν	46	10/1/1965 9/3(
80_Percent_Duration	1.5	cubic feet per second	<u>325</u>	Y	50	10/1/1965 9/3(
85_Percent_Duration	0.84	cubic feet per second	<u>325</u>	Y	50	10/1/1965 9/3(
90_Percent_Duration	0.3	cubic feet per second	<u>290</u>	Ν	46	10/1/1965 9/3(
90_Percent_Duration	0.36	cubic feet per second	<u>325</u>	Y	50	10/1/1965 9/3(
95_Percent_Duration	0.1	cubic feet per second	<u>290</u>	Ν	46	10/1/1965 9/3(
95_Percent_Duration	0.1	cubic feet per second	<u>325</u>	Y	50	10/1/1965 9/3(
97_Percent_Duration	0.02	cubic feet per second	<u>325</u>	Y	50	10/1/1965 9/3(
98_Percent_Duration	0	cubic feet per second	<u>290</u>	Ν	46	10/1/1965 9/3(
98_Percent_Duration	0	cubic feet per second	<u>325</u>	Y	50	10/1/1965 9/3(
99_Percent_Duration	0	cubic feet per second	<u>290</u>	Ν	46	10/1/1965 9/3(
99_Percent_Duration	0	cubic feet per second	<u>325</u>	Y	50	10/1/1965 9/3(
Annual Flow Statistics		3000110				
Mean_Annual_Flow	124	cubic feet per second	<u>325</u>	Y	50	10/1/1965 9/3(
Mean_Annual_Flow	114.000	cubic feet per second	<u>31</u>	Ν		
Daily_flow_years	14.000	years	<u>31</u>	Y		
Stand_Dev_of_Mean_Annual_Flow	49	cubic feet per second	<u>325</u>	Y	50	10/1/1965 9/3(
Maximum_Annual_Mean_Flow	255	cubic feet per second	<u>325</u>	Y	50	10/1/1965 9/3(
Minimum_Annual_Mean_Flow	39	cubic feet per second	<u>325</u>	Y	50	10/1/1965 9/3(
Monthly Flow Statistics						
January_Mean_Flow	145.000	cubic feet per second	<u>31</u>	Y		
February_Mean_Flow	163.000	cubic feet per second	<u>31</u>	Y		
March_Mean_Flow	254.000	cubic feet per second	<u>31</u>	Y		
April_Mean_Flow	220.000	cubic feet per second	<u>31</u>	Y		
May_Mean_Flow	110.000	cubic feet per second	<u>31</u>	Y		
June_Mean_Flow	102.000	cubic feet per second	<u>31</u>	Y		
July_Mean_Flow	78.400	cubic feet per	<u>31</u>	Y		
https://streamstatsags.cr.usgs.gov/gagepa	ges/html/033	78550.htm				4/5

https://streamstatsags.cr.usgs.gov/gagepages/html/03378550.htm

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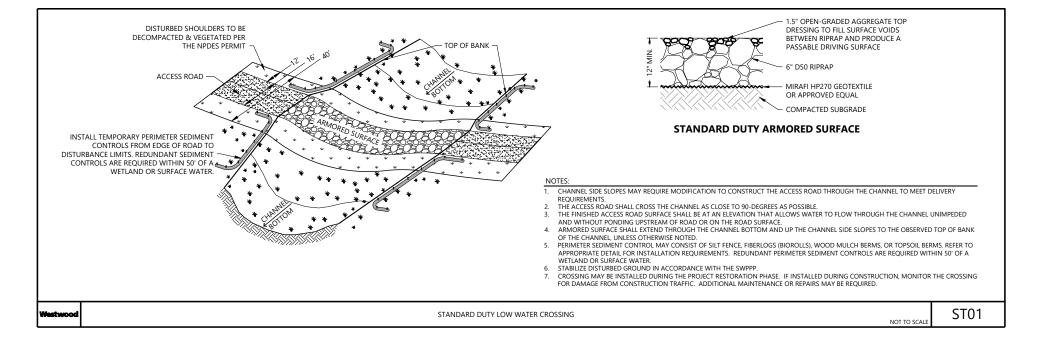
StreamStats Data-Collection Station Report

		second				
August_Mean_Flow	50.800	cubic feet per second	<u>31</u>	Y		
September_Mean_Flow	31.100	cubic feet per second	<u>31</u>	Y		
October_Mean_Flow	23.200	cubic feet per second	<u>31</u>	Y		
November_Mean_Flow	57.700	cubic feet per second	<u>31</u>	Y		
December_Mean_Flow	133.000	cubic feet per second	<u>31</u>	Y		
General Flow Statistics						
Minimum_daily_flow	0	cubic feet per second	<u>325</u>	Y	50	10/1/1965 9/3(
Maximum_daily_flow	10200	cubic feet per second	<u>325</u>	Y	50	10/1/1965 9/3(
Std_Dev_of_daily_flows	457	cubic feet per second	<u>325</u>	Y	50	10/1/1965 9/3(
Harmonic_Mean_Streamflow	0.66	cubic feet per second	<u>325</u>	Y	50	10/1/1965 9/3(
Mean_of_Logs_of_Daily_Values	1.149139	Log base 10	<u>325</u>	Y	50	10/1/1965 9/3(
Std_Dev_of_Logs_of_Daily_Values	1.028524	Log base 10	<u>325</u>	Y	50	10/1/1965 9/3(
Skew_of_Logs_of_Daily_Values	-0.366392	Log base 10	<u>325</u>	Y	50	10/1/1965 9/3(
Non_Zero_Adjusted_Harmonic_Mean_Flow	0.68	cubic feet per second	<u>325</u>	Y	50	10/1/1965 9/3(
Base Flow Statistics						
Number_of_years_to_compute_BFI	38	years	<u>42</u>	Y	38	
Average_BFI_value	0.119	dimensionless	<u>42</u>	Υ	38	
Std_dev_of_annual_BFI_values <i>Probabillity Statistics</i>	0.04	dimensionless	<u>42</u>	Y	38	
Probability_flow_durations_are_zero	0.024751	dimensionless	<u>325</u>	Y	50	10/1/1965 9/3(

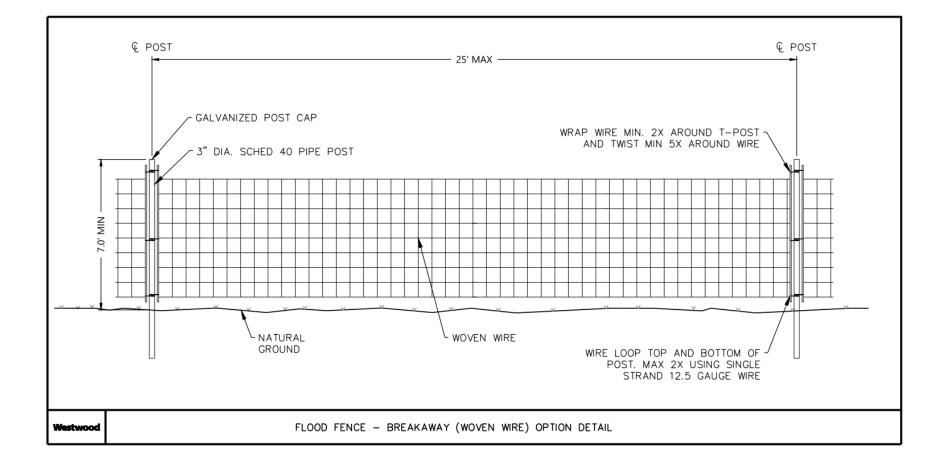
Citations

Citation Number	Citation Name and URL
31	Imported from Basin Characteristics file
42	Wolock, D.M., 2003, Base-flow index grid for the conterminous United States: U.S. Geological Survey Open-File Report 03-263, digital data set
62	<u>Rao, A.R., 2005, Flood-Frequency Relationships for Indiana: Joint Transportation Research Program, Purdue</u> <u>University, FHWA/IN/JTRP-2005/18, 14 p.</u>
290	Fowler, K.K., and Wilson, J.T., 2015, Low-flow characteristics for selected streams in Indiana: U.S. Geological Survey Scientific Investigations Report 2014 95242, 353 p.
325	Granato G.E., Ries, K.G., III, and Steeves, P.A., 2017, Compilation of streamflow statistics calculated from daily mean streamflow data collected during water years 1901 2015 for selected U.S. Geological Survey streamgages: U.S. Geological Survey Open-File Report 2017-1108, 17 p.

Appendix H Standard Duty Low Water Crossing Detail

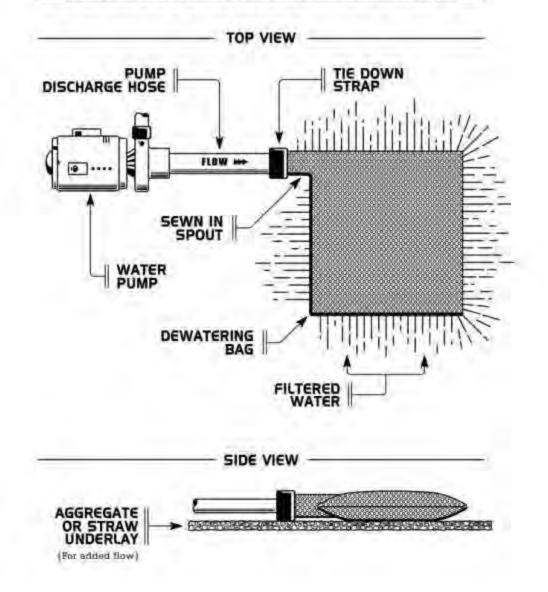


Appendix I Breakaway Fence Detail



Appendix J Dewatering Bag Detail & Specifications





DANDY DEWATERING BAG ™ PUMPED WATER SEDIMENT CONTROL SYSTEM GUIDE SPECIFICATIONS

PRODUCT:

DANDY DEWATERING BAGTM

MANUFACTURER:

Dandy Products Inc. P.O. Box 1980 Westerville, Ohio 43086 Phone: 800-591-2284 Fax: 740-881-2791 E Mail <u>dlc@dandyproducts.com</u> Web <u>www.dandyproducts.com</u>

1.0 **Description:**

1.1 Work covered under this consists of furnishing, installing, maintaining, and removal of the Dandy Dewatering Bag TM The purpose is to control sediment discharge in any dewatering or pumped water application.

2.0 Material:

- 2.1 The Dandy Dewatering Bag[™] shall be a bag sewn of nonwoven fabric **in the U.S.A.** using a double needle machine and a high strength thread.
- 2.2 The Dandy Dewatering Bag[™] shall have a spout opening large enough to accommodate at least a four (4) inch pump discharge hose with an attached strap to tie unit closed.
- 2.3 The Dandy Dewatering Bag[™] Seams shall be a double stitched "J" type seam with an average wide width strength per ASTM D-4884 of 60lb/in for a 8 oz. fabric manufactured in the U.S.A. with the following characteristics:

PROPERTY	TEST METHOD	UNITS	MARV
Grab Tensile Strength	ASTM D 4632	kN (lbs)	0.9 (205)
Grab Tensile Elongation	ASTM D 4632	%	50
Puncture Strength	ASTM D 4833	kN (lbs)	0.58 (130)
Mullen Burst Strength	ASTM D 3786	kPa (psi)	2618 (380)
Trapezoid Tear Strength	ASTM D 4533	kN (lbs)	0.36 (80)
% Open Area	COE - 22125-86	%	N/A
Apparent Opening Size	ASTM D 4751	mm (US Std Sieve)	.0180 (80)

Permittivity	ASTM D 4491	sec ¹	1.2
Permeability	ASTM 4491	cm/sec	0.21
Water Flow Rate	ASTM 4491	l/min/m ² (gal/min/ft ²)	3866 (95)
Ultraviolet Resistance	ASTM D 4355	%	70
Color			Black

3.0 Installation:

- 3.1 Lifting straps (not included) should be placed under the unit to facilitate removal after use.
- 3.2 Unfold Dandy Dewatering BagTM on a stabilized area over dense vegetation, straw, or gravel (if an increased drainage surface is needed) or as detailed in plans.
- 3.3 Insert discharge hose from pump into Dandy Dewatering BagTM a minimum of six (6) inches and tightly secure with attached strap to prevent water from flowing out of the unit without being filtered.

4.0 Maintenance:

- 4.1 Replace the unit when ½ full of sediment or when sediment has reduced the flow rate of the pump discharge to an impractical rate.
- 4.2 Remove and dispose of the sediment in a manner satisfactory to the engineer/inspector or in one of the following ways:
 - A) Remove the unit and sediment from environmentally sensitive areas and waterways. At the approved disposal site, slit the unit; remove the sediment and grade smoothly into the existing topography. Dispose of unit no longer in use at an appropriate recycling or solid waste facility.
 - B) Bury unit on site; remove any visible fabric and seed.

5.0 Method of Measurement:

5.1 The quantity to be paid is for the actual number of Dandy Dewatering Bags TM.

6.0 Basis of Payment:

6.1 The unit price shall include labor, equipment, and materials necessary to install, maintain, and remove the Dandy Dewatering BagTM.

6.2 Payment for the completed work will be made at the contract prices for:

 $\frac{\text{ITEM}}{\text{Dandy Dewatering Bag}^{\text{TM}}} \qquad \frac{\text{UNIT}}{\text{EA}}$

DESCRIPTION Pumped Water Sediment Control Unit (#_____UNITS)